

DESIGN DATA

PROJECT N2007(1-1)1,2&4

Design Speed Maximum Curvature Maximum Gradient Minimum Passing Sight Distance Minimum Stopping Sight Distance	50 mph 4° 6% 1835.93 Ft. 425.00 Ft.
Average Daily Traffic 2015	327 vpd
Future ADT (2035)	868 vpd
R.O.W. Width	See Table

N2007 RIGHT-OF-WAY TABLE

STATION	' TO	STATION	Width (Lt.) Ft.	Width (Lt.) Ft.
0+68.94	to	19+58.69	50.00	
19+60.09	to	28+93.53	75.00	
28+93.53	to	57+82.74	100.00	
0+68.94	to	19+53.05		50.00
19+47.42	to	31+46.09		150.00
31+53.63	to	57+82.74		50.00

LENGTH OF PROJECT

STATION TO STATION	FEET	MILES
B.O.P. Station 0+69.88 B.O.B. Station 24+83.00 E.O.B. Station 30+07.50 E.O.P. Station 57+82.74	2413.12 524.50 2775.24	0.4570 0.0993 0.5256
TOTAL	5712.86	1.0819

88.69+0
PROJ. N2007(1-1)1,2&4
I.D. N0. N00841

Grade, Drain, Aggregate Base, Hot Asphaltic Concrete Pavement, Bridge & Miscellaneous Construction

NAVAJO INDIAN RESERVATION -Route 2011 — ONHIR water pond and well -RIO PUERCO BRIDGE NOTE: See the specifications paragraphs for restrictions on use of existing roads. Routes 2007, 2015, and 2030

have 38 ton weight limits and the existing Puerco River bridge on Route 2007 has a posted weight limit of 10 tons.

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LEGEND

COUNTY LINE		
TOWNSHIP or RANGE LINE		
SECTION LINE		
NATIONAL FOREST LINE		I INSIDE OF LINE
HIGHWAY RIGHT-OF-WAY LINE		
UNFENCED PROPERTY		
SECTION CORNER and 1/4 CORNER	₹	\odot
POWER LINE and POLES	P	PP
TELEPHONE LINE and POLES	T	otot_
POLE GUY and ANCHOR	,	
TRAFFIC SIGN		Ь
GUARD RAIL	•	
DELINIEATION		EXISTING
BARBED WIRE FENCE		TWO WAY
		GATE * * * *
	***	**[[][`***
CATTLE GUARD	PLANNED	EXISTING
CULVERTS		EXISTING
CONCRETE BOX CULVERTS		EXISTING
GROUND LINE - EARTH		
GROUND LINE - ROCK		
EXISTING ROAD		4 & TYPE
SIDE ROAD TURNOUT		EXISTING
TREES and SHRUBS	<u>&&</u> & & &	3 000 00 3 000 00
CHANNEL or DITCH		
DIKE or DITCH BLOCK		
RIP-RAP	<u>0</u> \$%	52086886828
RAILROAD TRACK		+++++++++++++++++++++++++++++++++++++++
GAS LINE	G ———— G ———	G G
IRRIGATION LINE	IRR ——— IRR ——	IRR ——— IRR
WELL		
DWELLING		
SCH00L] V
CHURCH		弄
WINDMILL		X
RIGHT-OF-WAY MONUMENT	PLANNEI	D EXISTNG
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INDIAN SERVICE COUNTY	STATE	FEDERAL
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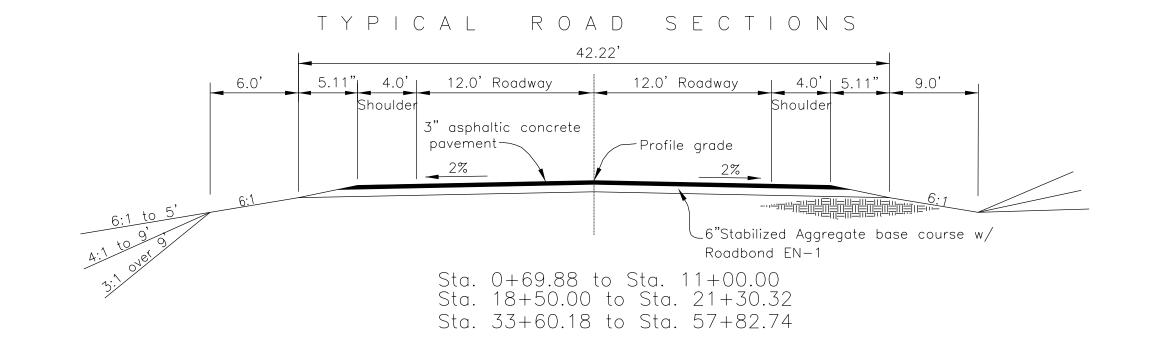
U. S. DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

08/28/2012 DATE AGENCY ROAD ENGINEER 08/28/2012 PLANNING & DESIGN BRANCH CHIEF DATE



REGIONAL DIRECTOR

08/28/2012



Existing ground

Varies

9.0'

New ditch flowline

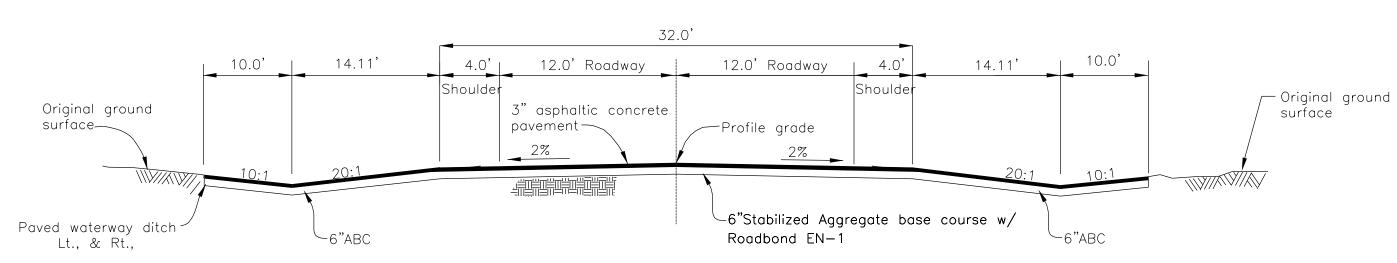
Existing ditch flowline

Sta. 53+50 to Sta. 58+40 Lt. - Grade to drain

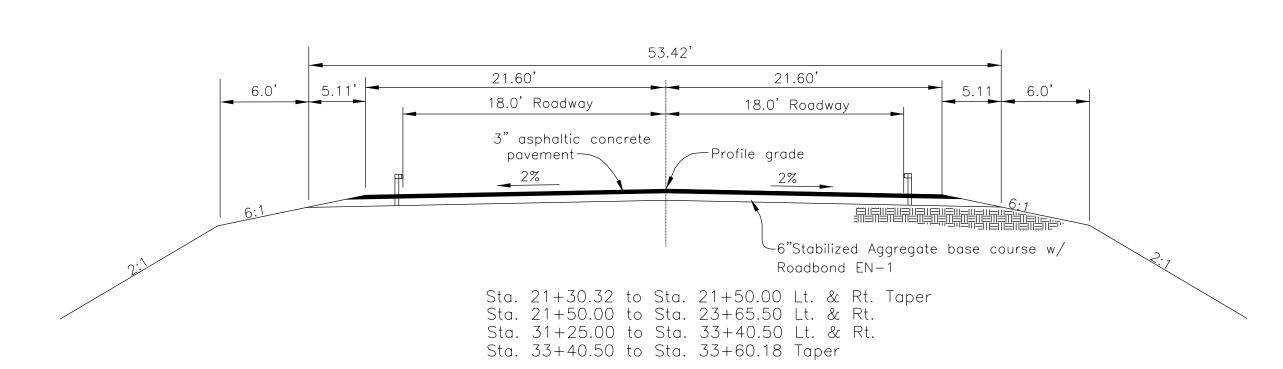
from new ditch flowline to existing ditch flowline.

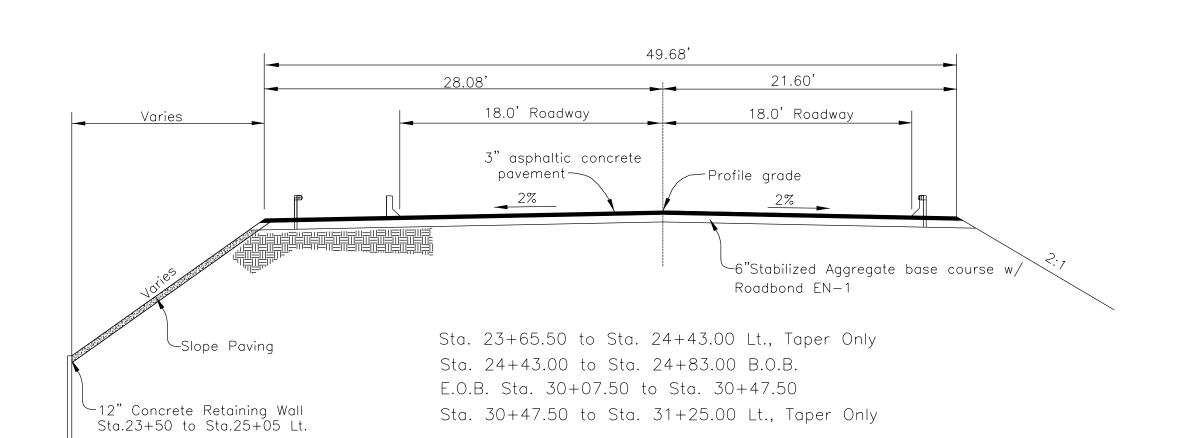
SEE SHEET 11&13 FOR FRONTAGE ROAD

TYPICAL CROSS SECTION.



Sta. 11+00 to Sta. 18+50.00 Rt. Sta. 11+00 to Sta. 17+00.00 Lt. SPECIAL PAVED DITCHES





SPECIAL PAVED DITCH NOTES:

1—Constructed the paved ditches as shown through residential area on right and left.

2—The paved ditch structural section shall match the roadway section (3 inches of asphalt over 6 inches of ABC) for the entire width and length of the ditch. The ABC course to extend 6 inches beyond all free asphalt edges.

3—The paved ditch inslope shall be 20:1 starting at the roadway shoulder NO 6:1 EDGE TAPER TO BE INSTALLED.

4—At driveways the paved backslope shall be lengthen to the right of way line or at PCC driveways, to the existing PCC pavement.
5—The backslope grade shall be adjusted to fit each existing driveway as directed by the COR/AOTR.

6—At roadway turnout at sta.14+50 Lt., carry paved ditch around the radius to the right of way line on the north side of the turnout. On the south side restart the paved ditch at the right of way line

TURNOUT LOCATIONS

LOC.	SIZE	TYPE	REMARKS
Lt.	40' x 34'	А	Reconstruct existing T.O. to match existing concrete at R/W line. Remove existing 24" pipe and replace w/new pipe & end sections
Rt.	14' x 34'	А	Reconstruct existing T.O.—No cattleguard w/type I gate and pipe.
Lt.	16' x 34'	А	Reconstruct existing T.O.—No cattleguard or pipe.
Rt.	16' × 34'	А	Reconstruct existing T.O.—No cattleguard or pipe.
Rt.	24' × 34'	А	Reconstruct existing T.O.—No cattleguard or Pipe.
Lt.	16' x 34'	А	Reconstruct existing T.O.—No cattleguard or Pipe.
Rt.	24' x 34'	А	Reconstruct existing T.O.—to match existing PCC driveway for new Business.
Rt.	24' x 34'	А	Reconstruct existing T.O.—to match existing PCC driveway.
Lt.	24' × 34'	Α	Reconstruct existing T.O.—to newlands office.
Rt.	16' × 34'	А	Reconstruct existing T.O.—to match existing PCC driveway.
Rt.	16' x 34'	А	Reconstruct existing T.O.—to match existing PCC driveway.
Lt.	16' x 34'	А	Reconstruct existing T.O.—No cattleguard or pipe.
Rt.	16' x 34'	А	Reconstruct existing T.O.—to match existing PCC driveway.
Rt.	16' x 34'	А	Reconstruct existing T.O.—to match existing PCC driveway.
Rt.	16' × 34'	А	Reconstruct existing T.O.—to match existing PCC driveway.
Rt.	24' × 34'	А	Reconstruct existing T.O.—Paved 50' from C/L and install new csp
Lt.	24' x 34'	А	Reconstruct existing T.O.—@ 113 ° skew—relocate 3—unit cattlegua
			180' Lt of C/L on old 66 Route, pass the proposed BNSF Service Road Intersection. Pave Turnout to new cattleguard location match existing service road and turnout @ 100'.
	Lt. Rt. Rt. Rt. Lt. Rt. Rt. Lt. Rt. Rt. Lt. Rt. Rt. Rt. Rt. Rt. Rt. Rt. Rt. Rt. R	Lt. 40' × 34' Rt. 14' × 34' Lt. 16' × 34' Rt. 24' × 34' Lt. 16' × 34' Rt. 24' × 34' Rt. 24' × 34' Rt. 16' × 34' Rt. 24' × 34'	Lt. 40' x 34' A Rt. 14' x 34' A Lt. 16' x 34' A Rt. 16' x 34' A Lt. 16' x 34' A Rt. 24' x 34' A Rt. 24' x 34' A Rt. 16' x 34' A Rt. 24' x 34' A

SEQUENCING NOTES:

1—The Contractor shall be required to break up all of the existing asphaltic concrete pavement structure in accordance with Section 204.09(b) of FP-03, including all existing paved turnouts. The Contractor has the option to utilized cold milling machine or other construction method to break up the existing pavement structure.

2-After processing the existing pavement, the contractor shall build up the subgrade with the material along with additional borrow with moisture and density control per Section 204 of FP-03.

3—At all new and reconstructed turnouts, and mainline, the contractor shall furnish and place 6" of new aggregate base course material on the finished and approved subgrade. The new placed ABC shall be treated with Roadbond EN—1.

4—The contractor shall then place 3" (2—1.5" lifts) of hot asphalt concrete pavement on the mainline, and all turnouts.

5—The Contractor shall provide for a safe and relatively dust free driving surface throughout the reconstruction process for all school, general traveling public, and local traffic use day and night. The contractor's traffic control and construction sequencing plan shall include these requirements and be submitted for review and approval prior to any ground disturbing activities taking place.

SPECIAL ROADWAY TYPICAL AND SPECIAL DITCH LOCATION

Station	To Station		Roadway Width to Hinge		 	
31011011	10 .	31011011	Lt.	Rt.	Kemarks	
6+60.00	То	11+00.00	16.00 ft.	16.00 ft.	Special Ditch Grade	
10+90.00	То	11+00.00	16.00 ft.	16.00 ft.	Special Ditch Grade	
11+00.00	То	17+00.00		16.00 ft.	Special Paved Ditch, Rt.	
11+00.00	То	17+00.00	16.00 ft.		Special Paved Ditch, Lt.	
20+30.32	То	21+50.00	Varies	Varies	19.68 ft. Taper Lt. & Rt.	
21+50.00	То	23+65.50	21.60 ft.	21.60 ft.	Guardrail Location:	
23+65.50	То	24+43.00	21.60 ft.	21.60 ft.	77.50 ft. of Taper with Guardrail, Left Side.	
24+43.00	То	24+83.00	27.08 ft.	21.60 ft.	1:11/2 at Fill Slope Location, Lt.	
24+83.00	То	30+07.50			Bridge Location.	
30+07.50	То	30+47.50	27.08 ft.	21.60 ft.	2:1 at Fill Slope Location, Lt. & Rt.	
30+47.50	То	31+25.00	27.08 ft.	21.60 ft.	77.50 ft. of Taper with Guardrail, Left Side.	
31+25.00	То	33+40.50	21.60 ft.	21.60 ft.	Guardrail Location,	
33+40.50	То	33+60.18			19.68 ft. Taper Lt. and Rt.	

REVISED: 08-15-2016

TYPICAL CROSS SECTION

UNITED STATES

DEPARTMENT OF THE INTERIOR

BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

DRAWN BY: Gerald.Hood DATE: 5/7/2009	
DESIGNED BY: NRDOT DATE:5/7/2009	
REVISED: 08/15/2016 BY: Peterson.Yazzie	
ANNOTATION SCALE: Full Size 1=1	
FILENAME: Sht.2_Typical Sections Sheet.dgn	

BASIS ESTIMATED QUANTITIES DESCRIPTION ITEM NO. GRADE APPLICATION FRONTAGE ROAD UNITS 30101-2000 Untreated Aggregate Base Course 140 lbs/ft 6" Mainline, — 4" Turnout 6" Service Road BNSF 30413-1000 Aggregate Stabilization with Roadbond EN-140201-0500 Hot Asphalt Concrete Pavement Class "B' 3" Mainline, — 2" Turnout 2" Service Road BNSF 150 lbs/ft° 40502-0800 Asphalt Cement 6% by Total Weight HACP PG58-28 0.9806 L/kg 0.30 gal/sq. yd. top of ABC 41101-5000 Asphalt Prime Coat 2.53 gal/ton

GENERAL NOTES:

- 1. ALL WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR CONSTRUCTION OF ROADS AND BRIDGES ON FEDERAL HIGHWAY PROJECTS (FP-03 ENGLISH UNITS), AND THE SUPPLEMENTAL SPECIFICATIONS FOR THIS PROJECT.
- 2. ALL PERMANENT AND TEMPORARY ROADSIDE SIGNS, AND PAVEMENT MARKINGS SHALL BE PLACED IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) FOR STREETS AND HIGHWAYS (LATEST EDITION) AND IN ACCORDANCE WITH THE DETAILS ON THESE PLANS. PLACEMENT OF "STOP" BAR, PERMANENT TRAFFIC SIGNS AND PAVEMENT MARKINGS SHALL BE FIELD ADJUSTED AS DIRECTED BY THE COR/AOTR, AT NO ADDITIONAL COST TO THE GOVERNMENT.
- THE TEMPORARY TRAFFIC CONTROL DETAILS SHOWN REFLECTS GENERAL REQUIREMENTS FOR THIS PROJECT. THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND SUBMITTING A TRAFFIC CONTROL PLAN IN ACCORDANCE WITH THESE DETAILS, TAKING INTO ACCOUNT THE CONTRACTOR'S CONSTRUCTION SEQUENCING PLAN, MUTCD, AND THE SUPPLEMENTAL SPECIFICATIONS FOR SECTION 635.—TEMPORARY TRAFFIC CONTROL. THE CONTRACTOR SHALL ALSO SUBMIT A COPY OF HIS TRAFFIC CONTROL PLAN, RELATED TO I—40 FRONTAGE ROAD AND RAILROAD CROSSING, TO THE ARIZONA DEPARTMENT AND TRANSPORTATION (DIANA ARMIJO (928)524—5455), AND BURLINGTON NORTHERN & SANTA FE (BNSF) (2)—WEEKS PRIOR TO START OF CONSTRUCTION.
- 4. THE DESIGN FEATURES INCLUDING HORIZONTAL AND VERTICAL ALIGNMENTS, TYPICAL SECTIONS, AND OTHER DESIGN DETAILS SHOWN SHALL NOT BE ALTERED OR MODIFIED IN ANYWAY DURING CONSTRUCTION WITHOUT THE EXPRESSED WRITTEN DIRECTION AND WRITTEN APPROVAL OF THE NAVAJO REGION OFFICE—DIVISION OF TRANSPORTATION (NRDOT) DIVISION MANAGER THROUGH THE AWARDING OFFICIAL (A0), UNLESS OTHERWISE NOTED IN THESE PLANS OR SPECIFICATIONS. DRAINAGE STRUCTURES AND TURNOUTS SHALL BE INSTALLED AS SHOWN WITH ONLY MINOR CORRECTIONS IN LOCATION, SKEW, AND/OR INVERT ELEVATIONS AS NEEDED TO FIT FIELD CONDITIONS. TURNOUTS MAY NOT BE SHIFTED MORE THAN 5.0 METERS FROM THE LOCATIONS SHOWN ON THE PLANS WITHOUT THE WRITTEN APPROVAL OF THE NRDOT DIVISION MANAGER THROUGH THE AWARDING OFFICIAL.
- 5. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY AND EXPENSE FOR DISPOSAL OF TRASH AND/OR CONSTRUCTION DEBRIS IN ACCORDANCE WITH SECTIONS 107 AND 203 OF THE FP-03 AS WELL AS ANY AND ALL PERMIT REQUIREMENTS. THIS WORK SHALL BE INCIDENTAL OBLIGATIONS OF THE CONTRACTOR.
- 6. THE BIDDER SHALL READ AND MAKE CAREFUL EXAMINATION OF THE PLANS, SPECIFICATIONS, QUANTITIES, MATERIAL, SURVEYING REQUIREMENTS, AND VISIT THE SITE OF THE PROPOSED CONSTRUCTION TO BECOME FAMILIAR WITH THE SITE CONDITIONS AND LIMITATIONS BEFORE MAKING A PROPOSAL. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY AND ALL ERRORS RESULTING FROM THE FAILURE TO MAKE SUCH AN EXAMINATION. ANY INFORMATION DERIVED FROM THE MAPS, PLANS, SPECIFICATIONS, PROFILES, DRAWINGS OR THE ENGINEER, SHALL NOT RELIEVE THE CONTRACTOR FROM ANY RISK OR FROM FULFILLING THE TERMS OF THE CONTRACT. THERE ARE SEVERAL AREAS WITH LIMITED WORKING ROOM WITHIN THE PROJECT RIGHT—OF—WAY, AND/OR WITH EXISTING FEATURES WITHIN OR NEAR THE PROJECT RIGHT—OF—WAY, THAT WILL REQUIRE 'SPECIAL' CONSTRUCTION PROCEDURES.
- 7. THE CONTRACTOR IS REQUIRED TO SUBMIT A REVISED PIPE LIST TO THE NRDOT, PLANNING & DESIGN BRANCH CHIEF THROUGH THE AOTR/COR, BASED ON THE FIELD STAKING IN ACCORDANCE WITH SECTION 152 OF THE CONTRACT SUPPLEMENTAL SPECIFICATION. THE APPROVAL OF ANY AND ALL REVISED PIPE LISTS WITH ACCOMPANYING DRAWINGS IS RENDERED AS A SERVICE ONLY AND IS NOT CONSIDERED A GUARANTEE OF MEASUREMENTS, QUANTITIES, INSTALLATION PROCEDURES, AND/OR DIMENSIONS, NOR SHALL IT BE CONSIDERED AS RELIEVING THE CONTRACTOR FROM COMPLYING WITH THE CONTRACT SPECIFICATIONS AND DESIGN PLANS. THE CONTRACTOR IS HEREBY NOTIFIED THAT UNDER NO CIRCUMSTANCE SHALL ANY DRAINAGE STRUCTURE(S) BE INSTALLED BELOW THE NATURAL FLOW LINE OF THE WASH, CHANNEL, ARROYO, OR DITCH LINE.
- 8. NO WORK SHALL BE PERFORMED OR GROUND DISTURBED OUTSIDE OF THE DESIGNATED CONSTRUCTION LIMITS IN ACCORDANCE WITH SECTION 107 OF THE FP-03 WITHOUT WRITTEN APPROVAL BY THE NRDOT DIVISION MANAGER UNLESS OTHERWISE SHOWN AND LABELED ON THESE PLANS AS "CONSTRUCTION ZONE". IN NO CASE SHALL ANY WORK BE PERFORMED OUTSIDE THE DESIGNATED RIGHTS-OF-WAY LIMITS WITHOUT WRITTEN APPROVAL FROM THE NRDOT DIVISION MANAGER, UNLESS OTHERWISE SHOWN AND CALLED OUT ON THESE PLANS AS "CONSTRUCTION ZONE". THE CONSTRUCTION LIMIT IS THE CATCH POINT EARTHWORK LIMIT PLUS 3.0 METERS, (OR AS DIRECTED BY COR/AOTR) NOT TO EXCEED THE RIGHT-OF-ENTRY LIMITS.
- 9. THE DETAILS SHOWN ON THE STORM WATER POLLUTION AND EROSION/SEDIMENT CONTROL DETAILS ARE GENERAL REQUIREMENTS TO BE USED BY THE CONTRACTOR IN PREPARING A STORM WATER POLLUTION PREVENTION PLAN ALONG WITH THE REQUIREMENTS IN SECTION 157 OF THE SUPPLEMENTAL SPECIFICATION AND SPECIAL CONTRACT REQUIREMENTS. THE SWPPP IS REQUIRED AT THE DRAINAGE PIPE REPLACEMENT LOCATIONS, ACCESS ROAD TO RAILROAD TRACKS & RIO PUERCO RIVER, ANYWHERE WHERE THERE IS GROUND DISTRUBING ACTIVITIES, AND MATERAILS STOCKPILES. THE CONTRACTOR IS REQUIRED TO SUBMIT COURTESY COPY OF THE APPROVED SWPPP TO THE ARIZONA DEPARTMENT ENVIRONMENTAL QUALITY (ADEQ) OFFICE (602) 771-4245.NICOLE CORONADO @ nm1@azdeq.gov
- 10. THE QUANTITIES SHOWN ARE FOR ESTIMATING PURPOSES ONLY AND TO COMPARE AND CANVAS BIDS. ACTUAL PAY QUANTITIES WILL BE DETERMINED IN THE FIELD FOR AUTHORIZED CHANGES THAT AFFECT THE QUANTITIES. ANY OVER-RUN OR UNDER-RUN OF QUANTITIES SHALL BE SUBJECT TO FAR 52.211-18, VARIATION IN ESTIMATED QUANTITY.
- 11. ALL TURNOUT/DRIVEWAYS, AS CALLED FOR ON THESE PLANS, SHALL EITHER BE CONSTRUCTED, REBUILT, RESHAPED AND/OR REMOVED UP TO THE RIGHT-OF-WAY LIMITS. ALL TURNOUTS SHALL BE PAVED TO THE CATTLEGUARD, THEN FROM THE BACK OF CATTLEGUARD TO THE R/W LINE, PLACE AGGREGATE BASE FOR ALL 14.0' WIDE TURNOUTS; PLACE AGGREGATE AND HOT ASPHALTIC CONCRETE FOR TURNOUTS WIDER THAN 14.0' TO MATCH THE STRUCTURAL SECTION. REQUIRED GRADING, SHAPING, AND EARTH COMPACTION OUTSIDE OF THE RIGHT-OF-WAY, TO CONNECT NEW TURNOUTS TO THE EXISTING ROADWAY/DRIVEWAY (AS SHOWN ON THE PLANS OR AS DIRECTED BY THE AOTR/COR) SHALL BE INCIDENTAL TO BID ITEM 20102-0000. ANY REQUIRED AGGREGATE BASE AND/OR ASPHALT MATERIAL SHALL BE PAID FOR UNDER THE APPROPRIATE BID ITEMS FOR THIS WORK AS SHOWN IN THE BID SCHEDULE.
- 12. THE CONTRACTOR SHALL BE REQUIRED TO OBLITERATE ALL EXISTING ABANDONED TURNOUTS AND ROADWAY WITHIN THE RIGHT-OF-WAY LIMITS, AND ANY EXISTING TURNOUTS/ROADWAY OUTSIDE OF THE RIGHT-OF-WAY THAT ARE DESIGNATED ON THE PLANS FOR OBLITERATION. OBLITERATION SHALL BE AS PER FP-03, METHOD 2. SCARIFICATION SHALL BE TO A DEPTH OF 12-INCH THE SCARIFIED SURFACE SHALL BE LEFT ROUGH, WITH 4-INCH TO 12-INCH HIGH RIDGES PERPENDICULAR TO THE EXISTING ROAD CENTERLINE. ROADWAY OBLITERATION INCLUDES GRADING DRAINAGE CHANNELS ACROSS THE OLD ROADBED, TO RE-ESTABLISH NATURAL DRAINAGE CHANNELS AND/OR TO OPEN CHANNELS FOR THE NEWLY INSTALLED (IN NEW ROADWAY) DRAINAGE STRUCTURES. THIS WORK TO BE INCIDENTAL WORK UNDER BID ITEM 20304-1000. PERMANENT SEEDING AND STRAW MULCHING SHALL BE APPLIED TO ALL AREAS WITHIN THE CONSTRUCTION LIMITS. SEEDING AND MULCHING TO BE PAID UNDER ITEM 62510-1000.

- 13. STRUCTURAL EXCAVATION AND BEDDING/BACKFILL OF ALL DRAINAGE STRUCTURES SHALL BE CONSIDERED INCIDENTAL TO THE INSTALLATION OF STRUCTURES. BEDDING AND BACKFILL MATERIAL SHALL MEET ALL REQUIREMENTS OF FP-03, SECTIONS 209 AND 704. APPROVED EXCESS EXCAVATION MATERIAL MAY BE USED TO REBUILD TURNOUTS, EARTHEN DITCH BLOCKS, AND/OR PLACED ALONG ROADWAY SHOULDERS AS EMBANKMENT IN AREAS ADJACENT TO THE REMOVAL AND AS DIRECTED BY THE COR/AOTR.
- 14. ALL FURROW AND DRAINAGE DITCHES SHALL BE STAKED AND GRADED TO DRAIN UP TO THE RIGHT-OF-WAY LIMITS. EARTHEN DITCH BLOCKS, DIKES AND DITCHES SHALL BE CONSTRUCTED AS SHOWN ON THESE PLANS AND/OR ADDED AT LOCATIONS DESIGNATED BY THE COR/AOTR. ALL DITCH BLOCKS, DIKES AND FURROW DITCHES SHALL BE PAID FOR UNDER THE APPROPRIATE BID ITEMS FOR THIS WORK AS SHOWN IN THE BID SCHEDULE. AT ALL DRAINAGE PIPE REPLACEMENTS, INSTALLATIONS, EXTENSIONS, AND IN-PLACE PIPE CLEANING LOCATIONS, THE CONTRACTOR SHALL CLEAN, REGRADE, AND RESHAPE THE INLET AND OUTLET CHANNELS TO THE RIGHT-OF-WAY LINE AS DIRECTED BY THE COR/AOTR.
- 15. IMMEDIATELY PRIOR TO PLACING EMBANKMENT, AGGREGATE BASE AND/OR RECYCLED MATERIAL, THE TOP 6-INCH OF THE ORIGINAL GROUND, OR FINISHED SUBGRADE (INCLUDING TURNOUTS) SHALL BE CHECKED FOR COMPACTION AND GRADE. IF COMPACTION DOES NOT MEET THE MINIMUM SPECIFIED COMPACTION AND TOLERANCE REQUIREMENTS, THE ORIGINAL GROUND AND/OR SUBGRADE SHALL BE RE-WATERED AND/OR SCARIFIED AS NEEDED AND RE-COMPACTED TO THE REQUIRED DENSITY AND TOLERANCE, AT THE CONTRACTOR'S EXPENSE. IN NO CASE SHALL ANY EMBANKMENT OR SURFACING MATERIAL (INCLUDING BASECOURSE) BE PLACED ON FROZEN, MUDDY OR UNSTABLE NATURAL GROUND OR SUBGRADE.
- 16. THE EARTHWORK TABLE SHOWN IS TO ASSIST THE CONTRACTOR IN ESTABLISHING A BID UNDER THE EARTHWORK ITEMS SHOWN IN THE BID SCHEDULE. ANY BORROW MATERIAL CALLED FOR ON THE PLANS SHALL BE TAKEN FROM CONTRACTOR IDENTIFIED SOURCES OUTSIDE THE RIGHT—OF—WAY LIMITS. IT IS THE SOLE RESPONSIBILITY AND EXPENSE OF THE CONTRACTOR TO PROVIDE ANY NECESSARY BORROW MATERIAL FOR THIS PROJECT INCLUDING ALL NECESSARY PERMITS. ALL EXCAVATION, BORROW AND EMBANKMENT MATERIAL SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEMS 20401—0000 AND 20403—0000.
- 17. THE LOCATION OF UTILITIES AS SHOWN IN THESE PLANS ARE APPROXIMATE AND ARE ONLY TO ASSIST THE CONTRACTOR IN COMPLETING THE WORK. THE CONTRACTOR SHALL CONTACT ALL UTILITY OWNERS PRIOR TO STARTING ANY CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL CONTACT THE ARIZONA BLUE STAKES AT 1-800-782-5348, AND NAVAJO TRIBAL UTILITY AUTHORITY (NTUA) AT (928)-729-5 721, PRIOR TO STARTING ANY CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL VERIFY ALL UTILITIES AND THEIR LOCATIONS WITH THE UTILITY OWNERS PRIOR TO CONSTRUCTION. ANY UTILITIES DAMAGED DUE TO NEGLIGENCE OF THE CONTRACTOR SHALL BE RESTORED TO CODE REQUIREMENTS AT THE CONTRACTOR'S EXPENSE.
- THE CONTRACTOR SHALL REMOVE, CLEAN, AND STOCKPILE ALL SALVAGEABLE EXISTING CULVERTS, CATTLE GUARDS AND FENCING MATERIALS, ETC, AS CALLED FOR ON THESE PLANS AND SECTIONS 203 AND 607. ALL SALVAGEABLE MATERIALS, EXCEPT THE BNSF AND COUNTY PROPERTY, SHALL BE STOCKPILED IN A DESIGNATED LOCATION FOR COMMUNITY USE. AOTR SHALL COORDINATE THIS WITH BNSF AND COUNTY PROPERTY REMOVED AS PART OF THIS PROJECT SHALL BE OFFERED TO THE COUNTY AND BNSF. IF THEY ACCEPT, THE MATERIAL SHALL BE HAUL AND STOCKPILED INSIDE THE RAILROAD'S PROPERTY LINE FOR BNSF PICK UP. ANY MATERIALS DETERMINED TO BE UNUSEABLE BY THE COR/AOTR SHALL BE DISPOSED OF BY THE CONTRACTOR IN ACCORDANCE WITH SECTIONS 107, AND 203. THE SALVAGE WORK SHALL BE INCLUDED IN THE APPROPRIATE UNIT PRICE BID ITEMS FOR SECTIONS 203 AND/OR 607
- 19. THE ROADWAY TYPICAL SECTION SHOWN IS THE BASIC TEMPLATE TO WHICH THE PROJECT IS TO BE STAKED AND BUILT. HOWEVER, THERE WILL BE LOCATIONS WHERE, DUE TO EXISTING GROUND CONDITIONS, TURNOUTS, CULVERTS OR OTHER STRUCTURES, ETC., THE SHOWN TYPICAL SLOPES CANNOT BE CONSTRUCTED. IN THIS CASE, THE NRDOT PLANNING & DESIGN BRANCH CHIEF, THROUGH THE COR/AOTR, SHALL BE CONSULTED FOR CHANGES IN THE TYPICAL SECTIONS, DESIGN SLOPES, AND/OR OTHER ADJUSTMENTS BEFORE PROCEEDING WITH THE WORK UNLESS NOTED OTHERWISE ON THE PLANS. THE FINAL CONSTRUCTED ROAD SECTION SHALL BE BASED ON THE GOVERNMENT FURNISHED COMPUTERIZED STAKING REPORT AS ADJUSTED TO FIT FIELD CONDITIONS. THE CONTRACTOR SHALL STAY WITHIN THE LIMITS OF CONSTRUCTION, UNLESS OTHERWISE APPROVED. IN NO CASE SHALL THE CUT AND FILL BACK SLOPES BE BUILT STEEPER THAN THE MAXIMUM ALLOWED IN THE ROADWAY TYPICAL SECTION SHOWN.
- 20. THE CONTRACTOR SHALL SAW CUT (FULL DEPTH) THE EXISTING ASPHALT PAVEMENT (INCLUDING TURNOUTS)WHERE NEW ASPHALT IS TO TIE INTO THE OLD ASPHALT PAVEMENT AT THE LOCATIONS NOTED ON THE PLANS. THE CONTRACTOR SHALL MATCH THE NEW ASPHALTIC CONCRETE PAVEMENT SURFACE TO EXISTING PAVEMENT SECTION AT TIE—IN POINTS AND TO PROVIDE FOR A SMOOTH TRANSITION AS DIRECTED BY THE COR/AOTR. ALL SAWED PAVEMENT EDGES TO RECEIVE ASPHALT TACK COAT. THIS WORK SHALL BE INCIDENTAL TO BID ITEM 40201—0500 AS SHOWN IN THE BID SCHEDULE.
- 21. ANY EXISTING OR NEW ROADSIDE FEATURES OR OTHER IMPROVEMENTS NEGLIGENTLY DAMAGED BY THE CONTRACTOR, DURING CONSTRUCTION, SHALL BE RESTORED/REPLACED IN EQUAL OR BETTER CONDITION AT THE CONTRACTOR'S EXPENSE.
- 22. REMOVAL AND RE-ATTACHMENT OF FENCING REQUIRED TO COMPLETE SPECIFIED WORK AT DRAINAGE STRUCTURES, CATTLE GUARDS, GATES, TURNOUTS, RIPRAP, ETC, SHALL BE CONSIDERED INCIDENTAL TO THE BID ITEMS RELATED TO THE WORK REQUIRING SAID FENCE REMOVAL/RE-ATTACHMENT. FENCING REPAIRS, TEMPORARY FENCING AND/OR REMOVAL AND RE-ATTACHMENT OF FENCING, SHALL BE COMPLETED IN THE SAME WORK DAY SO AS NOT TO ALLOW LIVESTOCK ONTO THE PROJECT. IF WIRE TENSION IS LOST IN THE EXISTING FENCE, THE CONTRACTOR SHALL RE-TIGHTEN THE FENCE AS DIRECTED BY THE COR/AOTR.
- THE CONTRACTOR SHALL REMOVE BIA ROUTE N2007 EXISTING ROADSIDE SIGNS THAT INTERFERE WITH ROAD CONSTRUCTION AND/OR CONTRADICT THE CONTRACTOR'S TEMPORARY TRAFFIC CONTROL PLAN, AT THE START OF THE CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY THE COR/AOTR AT LEAST THREE (3) WORKING DAYS IN ADVANCE OF SUCH SIGN REMOVAL. THESE ROADSIDE SIGNS SHALL BE SALVAGED AND TAKEN TO THE NEW LANDS MAINTENANCE YARD. SIGNS NEEDED FOR SAFETY/INFORMATION SHALL BE TEMPORARILY RESET AS DIRECTED BY THE COR/AOTR. ALL COUNTY ROUTE SIGNS SHALL BE REMOVED AND STOCKPILED, AND NOTIFY THE MAINTENANCE DEPARTMENT AT (928)-688-2928 FOR MATERIAL PICK UP. THIS WORK SHALL BE CONSIDERED AN INCIDENTAL OBLIGATION OF THE CONTRACTOR.

- AREA STATE RESERVATION ROUTE PROJECT NO. SHEET TOTAL SHEETS

 NAVAJO ARIZONA NAVAJO N2007 N2007(1-1)1,2&4 3 63
- 24. GRADE AND SHAPE THE SHOULDER AND DITCHES (AS DIRECTED BY COR/AOTR) FROM THE SUBGRADE HINGE POINTS TO AND INCLUDING THE EXISTING DITCH LINE AREAS FOR THE CONSTRUCTION OF RIPRAP DITCH LININGS, SLOPE PROTECTION, AND RUNDOWNS. THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE RIPRAP ITEMS SHOWN IN THE BID SCHEDULE.
- 25. AT MAJOR DRAINAGE STRUCTURES AND LIVESTOCK PASS LOCATIONS THAT SPECIFY REPLACING OR INSTALLING WING FENCES, THE CONTRACTOR SHALL TIE WING FENCES TO THE EXISTING STRUCTURES IN ACCORDANCE WITH THE DETAILS ON SHEET 28 OF 63. IF NO CORNER FENCE POST/BRACE/STRAIN EXISTS AT TIE—IN TO RIGHT—OF—WAY FENCE, THE CONTRACTOR SHALL INSTALL A STRAIN POST ASSEMBLY AS PER PLAN SHEET 28 OF 63. ANY EXISTING CATTLE PASS CLOSURES ARE TO BE REMOVED. THIS WORK TO BE INCIDENTAL TO BID ITEM 61921—1000, AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- ALL RIGHT-OF-WAY REFERENCE MARKERS SHALL BE LABELED IN THE ENGLISH UNITS OF MEASURE. ALL EXISTING AND NEW BRASS CAPS SHALL BE STAMPED WITH BOTH ALIGNMENT STATIONING AND ELEVATIONS IN ENGLISH, UNLESS OTHERWISE NOTED UNDER SECTION 152 OF THE SUPPLEMENTAL SPECIFICATIONS. ANY EXISTING R/W MONUMENTS AND BRASS CAPS MISSING SHALL BE RE-SURVEYED IN TO THEIR ORIGINAL POSITION AND LABELED AND STAMPED ACCORDINGLY. ALL EXISTING REFERENCE MARKERS SHALL BE SAND BLASTED, CLEAN, AND REPAINTED WITH ENGLISH STATIONS. ANY MISSING OR DAMAGED MARKERS SHALL BE RE-SURVEYED AND REPLACED. THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEMS 62101-0000 AND 62102-0000.
- 27. DUE TO NARROW RIGHT-OF-WAY WIDTHS, THE COMPLETE TURNOUT RADIUS CAN NOT BE CONSTRUCTED AT NUMEROUS TURNOUTS. AT THESE LOCATIONS THE CONTRACTOR SHALL CONSTRUCT THE PLANNED RADIUS, BUT END THE RADIUS AT THE RIGHT-OF-WAY LIMIT. THE ACTUAL TURNOUT WIDTH (AT THE RIGHT-OF-WAY LINE) WILL BE WIDER THAN THE WIDTH GIVEN ON THE PLANS.
- 28. THE GEO-TECHNICAL REPORT FOR THIS PROJECT SHALL BE PROVIDED UPON WRITTEN REQUEST FROM THE CONTRACTOR THRU COR/AOTR.
- 29. ROADWAY ENDAREA AND PIPE CROSS SECTION DRAWINGS WILL BE PROVIDED IN EITHER HARD COPY OR ELECTRONIC FORMAT UPON WRITTEN REQUEST FROM THE CONTRACTOR THRU CO/AO.
- 30. ANY EXISTING MAIL BOXES, ADVERTISING BILLBOARDS, OR HOUSE ADDRESS SIGNS LOCATED ALONG THE ROADWAY PRISM SHALL BE REMOVED AND RE-INSTALLED OUTSIDE OF THE RIGHT-OF-WAY LIMIT OR AS DIRECTED BY THE COR/AOTR. THE CONTRACTOR SHALL NOTIFY THE US-POSTAL SERVICE AND ATTEMPT TO CONTACT ALL AFFECTED RESIDENTS TEN (10) WORKING DAYS PRIOR TO RESETTING MAIL BOX (ES). THIS WORK SHALL BE INCIDENTAL TO BID ITEM 20304-1000.
- 31. AT THE COMPLETION OF THE CONSTRUCTION, THE CONTRACTOR SHALL INSPECT THE INTERIOR OF ALL NEWLY INSTALLED OR EXTENDED/CLEANED CULVERTS, CATTLEGUARDS, AND/OR OTHER EXISTING DRAINAGE STRUCTURES. THESE STRUCTURES SHALL BE MAINTAINED IN A CLEAN CONDITION, FREE OF SILT AND OTHER DEBRIS UNTIL FINAL ACCEPTANCE OF THE PROJECT. THIS WORK SHALL BE CONSIDERED AN INCIDENTAL OBLIGATIONS OF THE CONTRACTOR UNDER THE APPROPRIATE BID ITEMS, FOR SECTIONS 602, 603, 607, AND 619.
- THERE ARE NUMBER OF LOCATIONS WHERE RIPRAP, CHANNEL FLOWLINE GRADING, TURNOUTS, ETC., WILL REQUIRE WORK AND IMPROVEMENTS PLACED THROUGH AND BEYOND THE RIGHT-OF-WAY FENCING LOCATIONS. IN THESE LOCATIONS, THE RIGHT-OF-WAY FENCING SHALL BE ADJUSTED (POST SPACING, VERTICAL ALIGNMENT, POST INSTALLATIONS THROUGH RIPRAP, RIGHT-OF-WAY MONUMENT/MARKER ADJUSTMENT, ETC.) AS DIRECTED BY THE COR/AOTR. THIS WORK TO BE INCIDENTAL TO BID ITEM 61901-1000, 62101-0000, AND 62102-0000, AND NO ADDITIONAL PAYMENT WILL BE MADE.
- 33. IT IS EXPECTED A REVISED/FINAL RIGHTS-OF-WAY GRANT OF EASEMENT BE DEVELOPED DURING THE CONSTRUCTION OF THE N2007 PROJECT. THE CONTRACTOR SHALL NOT SURVEY FOR OR INSTALL R.O.W. MONUMENTS AND MARKERS OR FENCINGS UNTIL EXPRESSLY APPROVED BY THE NRDOT DIVISION MANAGER. RIGHT-OF-WAY FENCING CAN BE PLACED AT ALL ARCHAEOLOGICAL SITES IF SPECIFIED ON THE PLANS.

REVISED: 08-15-2016

UNITED STATES
DEPARTMENT OF THE INTERIOR
BUREAU OF INDIAN AFFAIRS

GENERAL NOTES

NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

DRAWN BY: Peterson.Yazzie DATE: 7/16/2011

DESIGNED BY: NRDOT DATE: 7/16/2011
REVISED: 8/15/2016 BY: Peterson.Yazzie

ANNOTATION SCALE: Full Size 1=1

TLENAME: Sht.3_General Notes_ 012813.dgr



TEM NO.	DESCRIPTION	N2007-QUANTITY F	rontage/Access Rd.	RIO PUERCO Bridge	UNIT
901-0000	Miscellaneous and Extra Work Under Section 109.02(m)	All Req'd.	All Req'd.	All Req'd.	L.S.
101-0000	Mobilization	All Req'd.	All Req'd.	All Req'd.	L.S.
201-0000	Construction Survey and Staking	All Req'd.	All Req'd.	All Req'd.	L.S.
301-0020	Contractor Quality Control	14,000.00	7,000.00	7,000.00	Man-
708-1000	Temporary Erosion Control	All Req'd.	All Req'd.	All Req'd.	L.S.
714-0000	Temporary Straw Mulching	12.40	5.00	0.00	Ac.
102-0000	Clearing and Grubbing	All Req'd.	All Req'd.	All Req'd.	L.S.
304-1000	Removal of Structures and Obstructions	All Req'd.	All Req'd.	All Req'd.	L.S.
401-0000	Roadway Excavation	5,866.99	719.64	0.00	C.Y.
403-0000	Borrow Excavation *	104,309.80	0.00	9,868.00	C.Y.
410-2000	Furrow Ditches and Ditch Blocks	500.00	0.00	0.00	L.F.
125-2000	Channel Reshaping, 3 ft. wide	0.00	369.00	0.00	L.F.
501-0000	Development of Water Supply	4.82	0.03	0.00	M-G
101-2000	Placed Riprap Class 2	38.84	27.00	0.00	C.Y.
12-3000	Articulated Concrete Block Revetment Type CC20	0.00	0.00	2,846.00	S.Y.
302-1000	Gabions, Galvanized Coated, Class 2	0.00	0.00	721.00	C.Y.
01-2000	Aggregate Base Course, Grading Special	8,084.18	710.44	0.00	Ton
113-1000	Aggregate Base Course, Stabilization, Roadbond, EN-1	21,431.00	0.00	0.00	S.Y.
201-0500	Hot Asphalt Concrete Pavement, Class B, Grade B, Type III Smoothnes	3,896.04	266.25	0.00	Ton
502-0800	Asphalt Cement Grade PG 58—28	233.71	16.13	0.00	Ton
01-5000	Prime Coat, Grade PEP	29.58	2.88	0.00	Ton
201-0200	Structural Concrete, Class A(AE)	0.00	0.00	1,888.00	C.Y.
01-2000	Precast Prestressed Structural Member, BT-72, 130'-5' long girder	0.00	0.00	12.00	Ea.
01-2010	Precast Prestressed Structural Member, BT-72, 129'-4' long girder	0.00	0.00	12.00	Ea.
1000	Reinforcing Steel, Grade 60	0.00	0.00	186,034.00	Lb.
01-2000	Epoxy Coated Reinforcing Steel, Grade 60	0.00	0.00	236,909.00	Lb.
01-0600	Drilled Shalfs, 4'-0" diameter	0.00	0.00	507.00	L.F.
01-0800	Drilled Shalfs, 5'-0" diameter	0.00	0.00	407.00	L.F.
201-0810	24" Corrugated Steel Pipe Culvert	178.00	0.00	0.00	L.F.
01-0910	36" Corrugated Steel Pipe Culvert	212.00	0.00	0.00	L.F.
202-0510	28" Span x 20" Rise, Corrugated Steel Pipe Arch	0.00	142.00	0.00	L.F.
02-0610	35" Span x 24" Rise, Corrugated Steel Pipe Arch	70.00	0.00	0.00	L.F.
10-0810	End Section for 24" CSPC	5.00	0.00	0.00	Ea.
10-1010	End Section for 36" CSPC	2.00	0.00	0.00	Ea.
11-0910	End Section for 28" Span, 20" Rise CSPA	0.00	6.00	0.00	Ea.
211-1010	End Section for 35" Span, 24" Rise CSPA	1.00	0.00	0.00	Ea.
701-1000	Removing, Cleaning, and Stockpiling Salvageable Culverts	448.00	0.00	0.00	L.F.
801-0500	Paved Waterway, Type V, 2" Thick	0.00	1,514.30	0.00	S.F.
701-1250	Guardrail System: SGR04b, Type PDE02 w/ SKT-350 End Treatment	1,178.00	0.00	0.00	L.F.
07-0000	Structural Transition Railing : Thrie Beam Transition at Bridge	0.00	0.00	75.00	L.F.
711-5000	Impact Crash Attenuator, QUADGUARD or Approved equal	0.00	0.00	2.00	Ea.
301-1000	Concrete Barrier	40.00	0.00	0.00	L.F.
901-1000	Fence, 5 Strand Barbed Wire	5,900.00	0.00	0.00	L.F.
01-1300	Fence, Chain Link, Bridge, Pedestrain, Curved 6'-6" Height	0.00	0.00	552.00	L.F.
01-1800	Fence, Chain Link, Bridge, Pedestrain, Stright 5'-0" Height	0.00	0.00	552.00	L.F.
01-2100	Fence, Chain Link 7 ft. High, with 2—24 ft. Swinging Gates (maint. ya	d 0.00	150.00	0.00	L.F.
01-3400	Temporary Safety Fence, HDPE—Type	200.00	0.00	0.00	L.F.
02-0010	16 ft. wide Turnout w/ out gate	9.00	0.00	0.00	Ea.
02-0020	24 ft. wide Turnout w/ out gate	7.00	0.00	0.00	Ea.
	14 ft. wide Turnout w/ Type I Gate Only	2.00	1.00	0.00	Ea.
	Gate, Type III, 20 Ft. Width	0.00	1.00	0.00	Ea.
	18 ft. Type III Lockable Closure Gate	1.00	0.00	0.00	Ea.
03-0810		1.00	0.00	0.00	Ea.
03-1100	Cattleguard, 3 Unit, with Type II Gate	1.00	0.00	0.00	Ea.
03-1210	Cattleguard, 4 Unit, without Gate	2.00	0.00	0.00	Ea.
21-0000	Remove and Reset fence	675.00	0.00	0.00	L.F.
01-0000	Right-of-Way Monument	21.00	0.00	0.00	Ea.
02-0000	Reference Marker	21.00	0.00	0.00	Ea.
10-1000	Seeding, Dry Method	8.50	0.00	0.00	Ac.
01-1100	Erosion Control Matting, Type IV	2,723.00	0.00	0.00	S.Y.
02-0003	Sign Installation, 1 Post and Hardware: 2.75 lb/ft.	38.00	0.00	0.00	S.F.
02-0010	Sign Installation, 2 Posts and Hardware: 2.00 lb/ft.	41.65	0.00	0.00	S.F.
08-2000	Object Markers, Flexible, Type 2	6.00	0.00	0.00	Ea.
08-3000	Object Markers, Type 3, w/ 1— Post and Hardware, 2.00 lb/ft.	0.00	0.00	4.00	Ea.
09-0010	Delineators, Flexible, Type "1a"	10.00	0.00	0.00	Ea.
09-0020	Delineators, Flexible, Type "1b"	5.00	0.00	0.00	Ea.
18-1000	Mile Markers, w/ 1 Post and Hardware; 2.00 lb/ft.	4.00	0.00	0.00	Ea.
-01-1510	Pavement Markings, Type "H", Solid Yellow	2,383.00	522.62	0.00	Ed
		2,383.00 9,650.00			
01-1520	Pavement Markings, Type "H", Solid White	·	0.00	0.00	L.F.
01-1610	Pavement Markings, Type "H", Broken Yellow	6,661.00	0.00	0.00	L.F.
01-0000	Temporary Traffic Control	All Req'd.	All Req'd.	0.00	L.S.
02-3000	Temporary Traffic Control, Raised Pavement Markers, Yellow	700.00	0.00	0.00	Ea.
09 – 1000	Flaggers	8,000.00	8,000.00	0.00	Man

ITEM	61921-00	000 —	REMOVE	AND	RESET	FENCE	@	675 I	L.F.	
STATIC	NOITATE OT NO		I REMARKS							

STATION TO STATION	LOCATION	REMARKS
20+50 to 23+75	Lt.	Remove/reset 325' barbed wire fence for BNSF service road
26+65.00	Lt. & Rt.	Remove 350' existing barbed wire fence, as necessary for construction activities. Reset fence under new bridge & across the removed bridge location. Tie reset fence onto existing fence.

ITEM 61901-2100 - CHAIN LINK FENCE w/2-12' SWINGING GATE

STATION TO STATION	LOCATION	DESCRIPTION	
23+40.00 to 24+90.00	Left	150' Install new chain link fence 2-12' swinging gates	
		TOTAL:	299 L.F.

ITEM	62901-1	100 -	EROSION	CONTROL	MATTING	F, TYPE IV

TILIWI OZJOT I	<u> </u>	1001014	CONTROL	INIT I I I I I A C	9 □
STATION	LOCATION	HEIGHT	LENGTH	2:1Factor	QUANTITY (S.Y.)
21+30 to 23+50	Lt.	17.18 Avg.	220	0.7071	296.86
21+30 to 24+68	Rt.	23.09 Avg.		0.7071	613.17
30+15 to 33+60	Lt.	33.46 Avg.		0.7071	906.35
21+30 to 24+68	Rt.	33.46 Avg.		0.7071	906.35
				TOTAL:	2,722.73 S.Y.

REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	NAVAJO	N2007	N2007(1-1)1,2&4	4	63

Description:	Location:	Offset:	New aggregate base course (ton)	Aggregate stabilization with RoadBond	HACP (ton)	Asphalt Cement PG58—28 (ton)	Asphalt Prime coat (ton)	Remark:
0+69.88 - 21+30.32	mainline	centerline	2,814.20	8,149.00	1,309.00	78.50	9.70	regular roadway section.
21+30.32 - 21+50.00	mainline	centerline	30.80	90.00	14.60	0.90	0.10	tapered guardrail section.
21+50.00 - 23+65.50	mainline	centerline	379.30	1,122.00	182.40	10.90	1.30	constant guardrail section.
23+65.50 - 24+43.00	mainline	centerline	132.30	411.00	68.60	4.10	0.50	tapered guardrail section on left side.
24+43.00 - 24+83.00	mainline	centerline	72.90	227.00	37.90	2.30	0.30	constant guardrail section to Beginning of Bridge.
30+07.50 - 30+47.50	mainline	centerline	72.90	227.00	37.90	2.30	0.30	Ending of Bridge to constant guardrail section.
30+47.50 - 31+25.00	mainline	centerline	132.30	411.00	68.60	4.10	0.50	tapered guardrail section on left side.
31+25.00 - 33+40.50	mainline	centerline	379.30	1,122.00	182.40	10.90	1.30	constant guardrail section.
33+40.50 - 33+60.18	mainline	centerline	30.80	90.00	14.60	0.90	0.10	tapered guardrail section.
33+60.18 - 57+82.74	mainline	centerline	3,308.80	9,582.00	1,539.10	92.30	11.40	regular roadway section.
turnout	3+40	left	64.00	0.00	39.00	2.37	0.36	40 ft wide x 34 ft driveway to business.
turnout with type 1 gate	8+27	right	32.33	0.00	19.17	1.15	0.18	14 ft wide x 34 ft turnout.
turnout	11+41	left	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	11+71	right	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	12+30	right	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout.
turnout	12+65	left	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	13+34	right	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout.
turnout	14+40	right	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout.
turnout	14+50	left	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout.
turnout	15+16	right	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	15+85	right	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	15+85	left	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	16+55	right	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	17+25	left	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout.
turnout	17+25	right	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	17+94	right	32.33	0.00	19.17	1.15	0.18	16 ft wide x 34 ft turnout.
turnout	19+75	right	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout to old HWY666
turnout	19+75	left	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout to BNSF service road.
turnout with type 2 gate.	37+00	right	42.91	0.00	26.28	1.58	0.24	24 ft wide x 34 ft turnout with 3-unit cattleguard.
, , , , , , , , , , , , , , , , , , ,		subtotal:	8,084.18	21,431.00	3,896.04	233.71	29.58	
0+17.38 - 5+75.00	Frontage	centerline	444.60	0.00	143.90	8.60	1.60	Frontage and Access Road
5+75.00 - 8+32.00	Access	centerline	150.60	0.00	46.90	2.80	0.50	Frontage and Access Road
turnout	1+35.00	left	42.91	0.00	26.28	1.58	0.24	24 ft wide x 80 ft
turnout with type 1 gate	4+80.00		32.33	0.00	19.17	1.15	0.18	14 ft wide x 10 ft
turnout	5+30.00		20.00	0.00	15.00	1.00	0.18	24 ft wide x 10 ft to BNSF service building.
turnout	6+35.62		20.00	0.00	15.00	1.00	0.18	24 ft wide x 20 ft to BNSF track.
		subtotal:	710.44	0.00	266.25	16.13	2.88	
		grand total:	8,794.62	21,431.00	4,162.29	249.84	32.46	

ITEM 61903-1210 - 4 UNIT CATTLEGUARD NO GATE STATION LOCATION DESCRIPTION 19+25.0 C/L Install new 4 unit cattle guard

57+82.7 C/L E.O.P. Install new 4 unit cattle guard

<u>ITEM 20304-1000 - REMOVAL OF STRUCTURE & OBSTRUCTIONS</u>

STATION	LOCATION	REMARKS
3+55.00	Left.	Remove and salvage metal post & wing fences
3+33.00	Leit.	at T.O. Set fence material out side R/W line.
8+27.00	Right	Remove existing cattleguard and foundation
19+25.00	Centerline	Remove existing 4-unit cattleguard & tie-in
19+23.00	Centernie	fencing back to R/W lines.
20+40.71	3.76 Rt.	Remove existing sign
20+30.00 to 24+90.00	Left	Remove meandering 465' barbed wire fence
23+80.00 to 24+90.00	Left	Remove 110' temporary chainlink fence segments
25+05.00	Right	Remove ±60' of BNSF barbed wire fencing
25+95.00	Right	Remove ±100' of BNSF barbed wire fencing
26+50.00 to 30+00.00	Left	Remove existing bridge SEE NOTE #15 on SHEET B-1
31+60.00	Rt. & Lt.	Remove ±400' of BNSF property barbed wire fencing
24+15.00	Left	Remove 2'x3' Steel Metal Grate & 2-14"x ±160' pipe
		DETOUR — FRONTAGE ROAD
5+35.00	Left.	Remove 10'x12' concrete slab at West Entrance
J+33.00	Leit.	of Service Building off Frontage Road
25+40+	Left	Relocate existing drainage pipes 18' Left under new Detour Road.

ITEM 20401-0000 - EARTHWORK QUANTITIES

11211 20101 000		11 O I (I (Q O)	111111	
STATION - STATION	CUT (yd ³)	FILL (yd ³)	BORROW (yd ³)	WASTE (yd ³)
0+69.88 to 21+60.15	4,281.25	4,281.25	0	0
21+60.15 to 24+83.0	0	32,790.48	0	0
	**** EXCEPTION	N - BRIDGE *	***	
30+07.50 to 57+82.7	1,585.74	73,105.05	71,519.32	0
Sub-total:	5,866.99	110,176.78	104,309.80	0
**	** MAINTENANC	E/DETOUR ROA	D ****	
0+17.38 to 01+32.5	41.16	41.15	0	0
01+32.52 to 8+32	678.48	154.97	0	523.50
Sub-total:	719.63	196.13	0	523.50
TOTAL:	6,586.63	110,372.91	104,309.80	0

* 20% Shrinkage Factor applied

ITEM 61902-0010 - 16' TURNOUT NO GATE -9 REQ'D.

STATION	LOCATION	DESCRIPTION
11+41.00	Lt.	Reconstruct 16' turnout
11+71.00	Rt.	Reconstruct 16' turnout
12+65.00	Lt.	Reconstruct 16' turnout
15+16.00	Rt.	Reconstruct 16' turnout to match existing concrete driveway
15+85.00	Lt.	Reconstruct 16' Turnout
15+85.00	Rt.	Reconstruct 16' turnout to match existing concrete driveway
16+55.00	Rt.	Reconstruct 16' turnout to match existing concrete driveway
17+25.00	Rt.	Reconstruct 16' turnout to match existing concrete driveway
17+94.00		Reconstruct 16' turnout to match existing concrete driveway
		ý

ITEM 61902-0020 - 24' TURNOUT NO GATE - 7 REQ'D.

STATION	LOCATION	DESCRIPTION
12+30.00	Rt.	Reconstruct 24' turnout
13+66.00	Rt.	Reconstrurt 24' turnout to match existing concrete driveway
14+40.00	Rt.	Reconstruct 24' turnout to match existing concrete driveway
14+50.00	Lt.	Reconstruct 24' turnout to New Lands Office
17+25.00	Lt.	Reconstruct 24' turnout
19+75.00	Rt.	Reconstruct 24' turnout
19+75.00	Lt.	Reconstruct 24' turnout @ 113 degree skew
MAINTE	NANCE RO	DAD — 24' TURNOUT NO GATE
1+35.00*	Lt.	Reconstruct 24' turnout to Old Hwy. 66
5+30.00*	Lt.	Reconstruct 48' turnout to BNSF Service Building
6+35.62*	Lt.	Reconstruct 24' turnout to existing bridge
4. D	/ .	

*Reconstruct maint./detour road turnouts with gravel only

ITEM 60701-1000 - REMOVING, CLEANING, STOCKPILING SALVAGEABLE CSPC STATION | SIZE | REMARKS

31711011	LOOKITOIT	J12L	ILLMAKKS
3+55.00	Turnout Left		To be removed/salvage
6+60.00	C/L	$1-24" \times 70'$	To be removed/salvage
8+27.00	Turnout-Rt.	1-18" x 50'	To be removed/salvage
24+15.00	10'+Lt.	2-14" x 160'	Concrete inlet and under ground drainage
31+75.00	300' Lt./exst'g road	2-24" x 30'	Removed & salvage
34+50.00	130' Rt./exst'g road	1-24" x 30'	To be removed/salvage
47+40.00	150' Lt./exst'g road	$1-24" \times 50'$	To be removed/salvage
50+30.00	C/L	1-24" x 80'	To be removed/salvage

TOTAL: 448 Feet To be removed/salvage

ITEM 61902	2-1300 - 1	4' TURNOUT w/TYPE 1 GATE
STATION	LOCATION	DESCRIPTION
8+27.00	Rt.	Reconstruct 14' Turnout
MAINTENAN	CE ROAD -	14' Turnout w/Type 1 Gate
4+80.00	Lt.	Frontage Road

ITEM 61901-1000 - BARBED WIRE FENCING STATION TO STATION LOCATION REMARKS

STATION TO STATION	LOCATION	INCMANNS
19+25 - 20+10	Lt.	100' Tie to Cattleguard
20+30-21+52	Lt.	120' Install to Cattleguard & tie to Guardrail
31+60.00	Lt. & Rt.	400' BNSF property fence
31+60-58+00	Lt. & Rt.	5,280 L.F. R/W fence
	TOTAL ·	5 900 I F

61701-5000; GUARDRAIL SYSTEM, SGR-04b, TYPE PDE WITH STK 350 TERMINAL SECTION

STATION	1 TO	STATION	LOCATION	LENGTH (ft)	Remarks
21+50.00	То	24+44.583	Right	294.58	Includes STK 350, Connect to guardrail transition railing
21+50.00	То	24+44.583	Left	294.58	Includes STK 350, Tapered guardrail
30+45.917	То	33+40.50	Right	294.58	Includes STK 350, Straight section guardrail
30+45.917	То	33+40.50	Left	294.58	Includes STK 350, Tapered guardrail
			TOTAL:	1.178.33	

ITEM 61903-1100 - 24' TURNOUT w/3 UNIT CATTLEGUARD & TYPE II GATE | STATION | LOCATION | DESCRIPTION | 37+00.00 | Rt. | Construct 24' turnout w/3 unit cattleguard & Type II Gate

ITEM 61903-0810 - 40' Turnout w/No Gate

STATION LOCATION DESCRIPTION

3+40.00 Lt. Construct 40' turnout w/No Gate

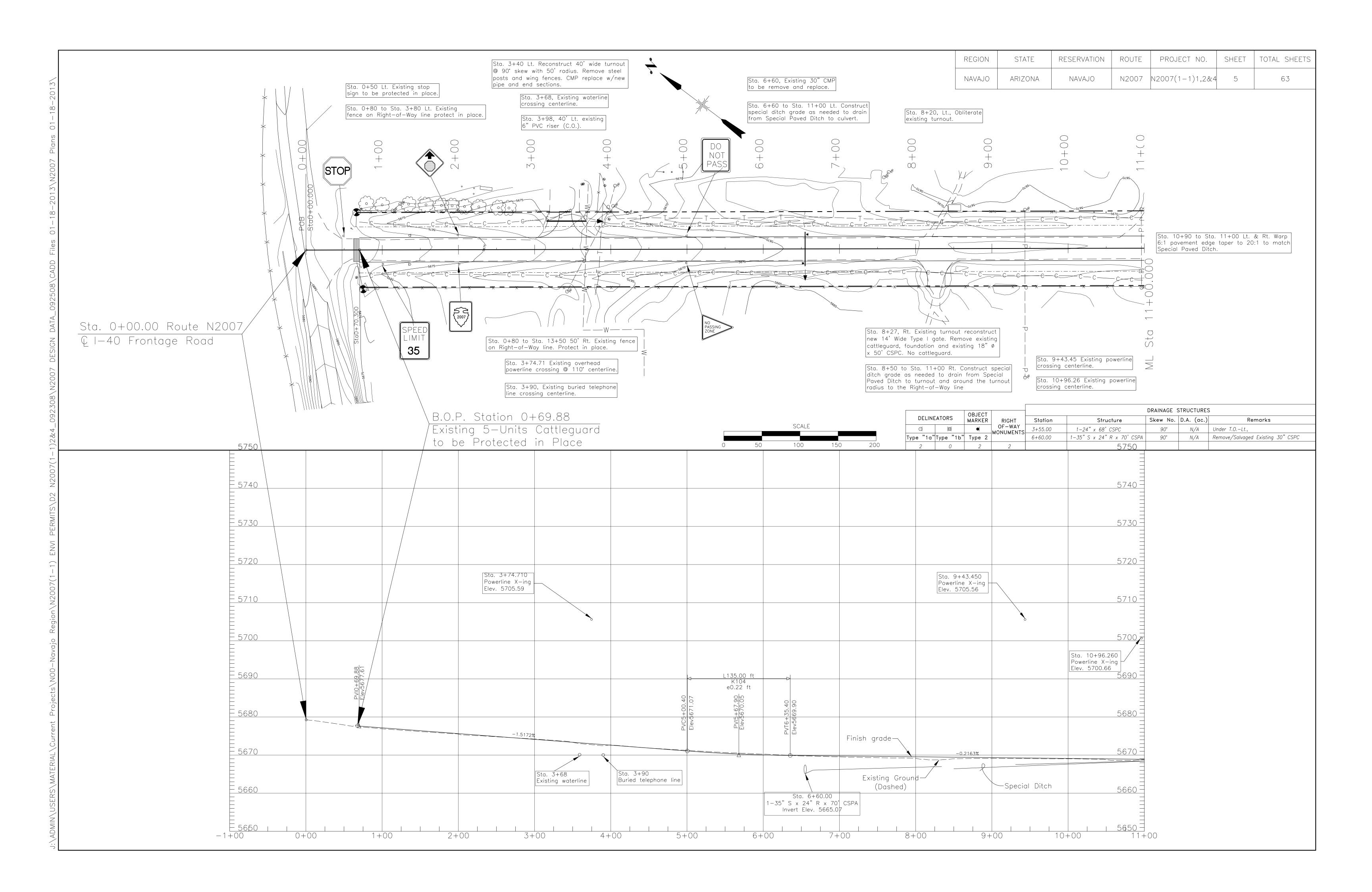
REVISED ON 8/16/2016

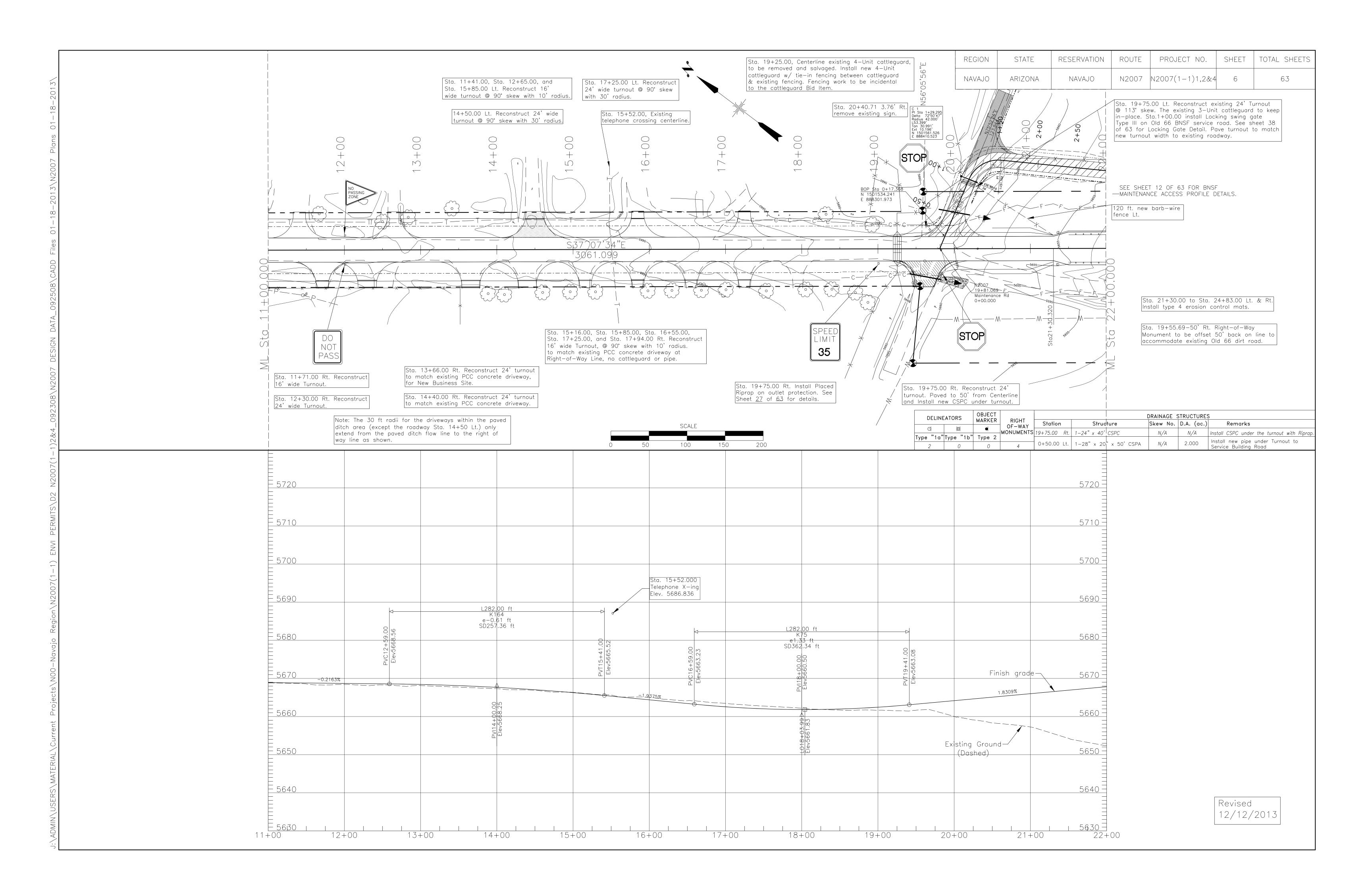
UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

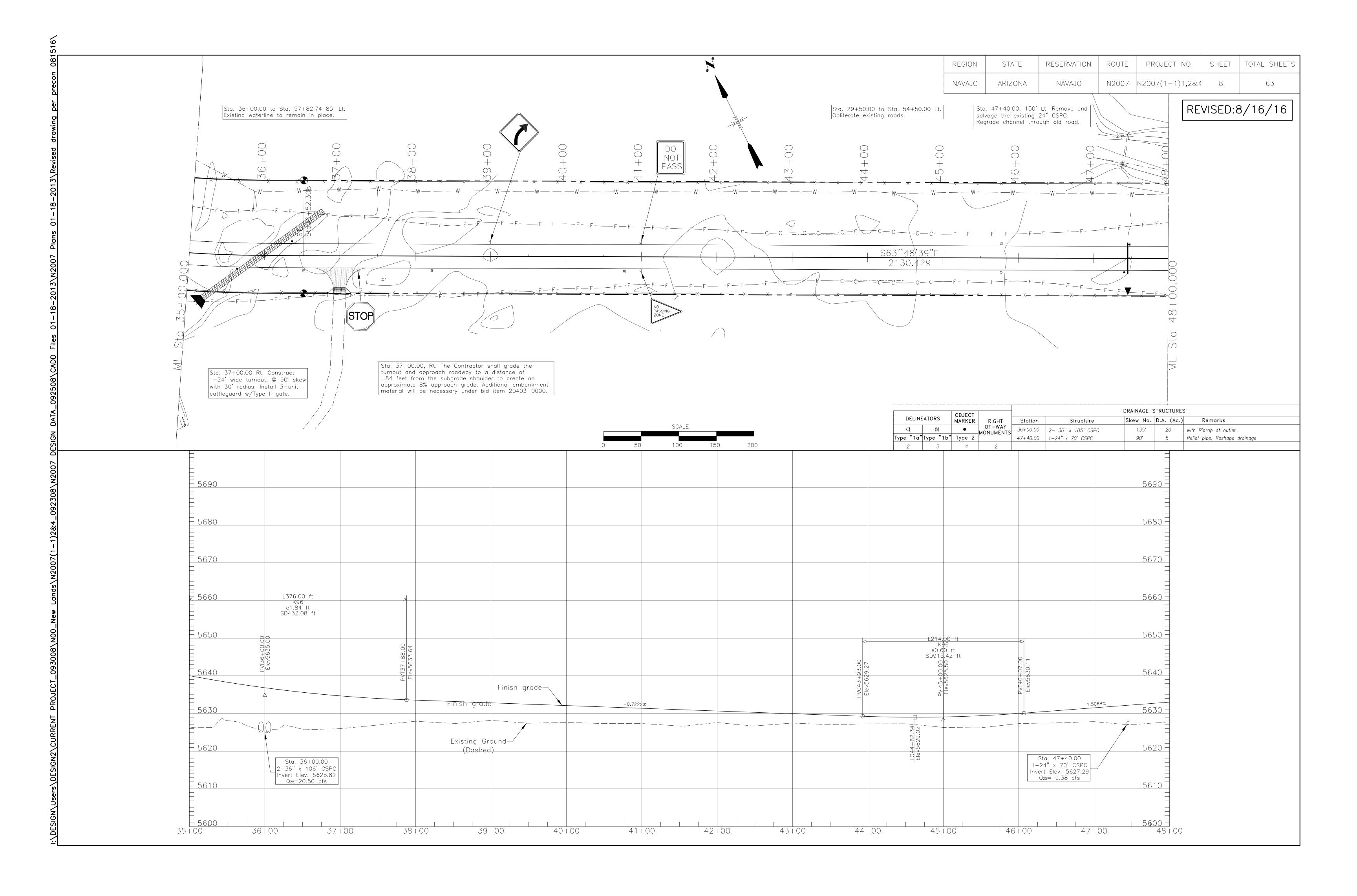
ESTIMATED QUANTITIES

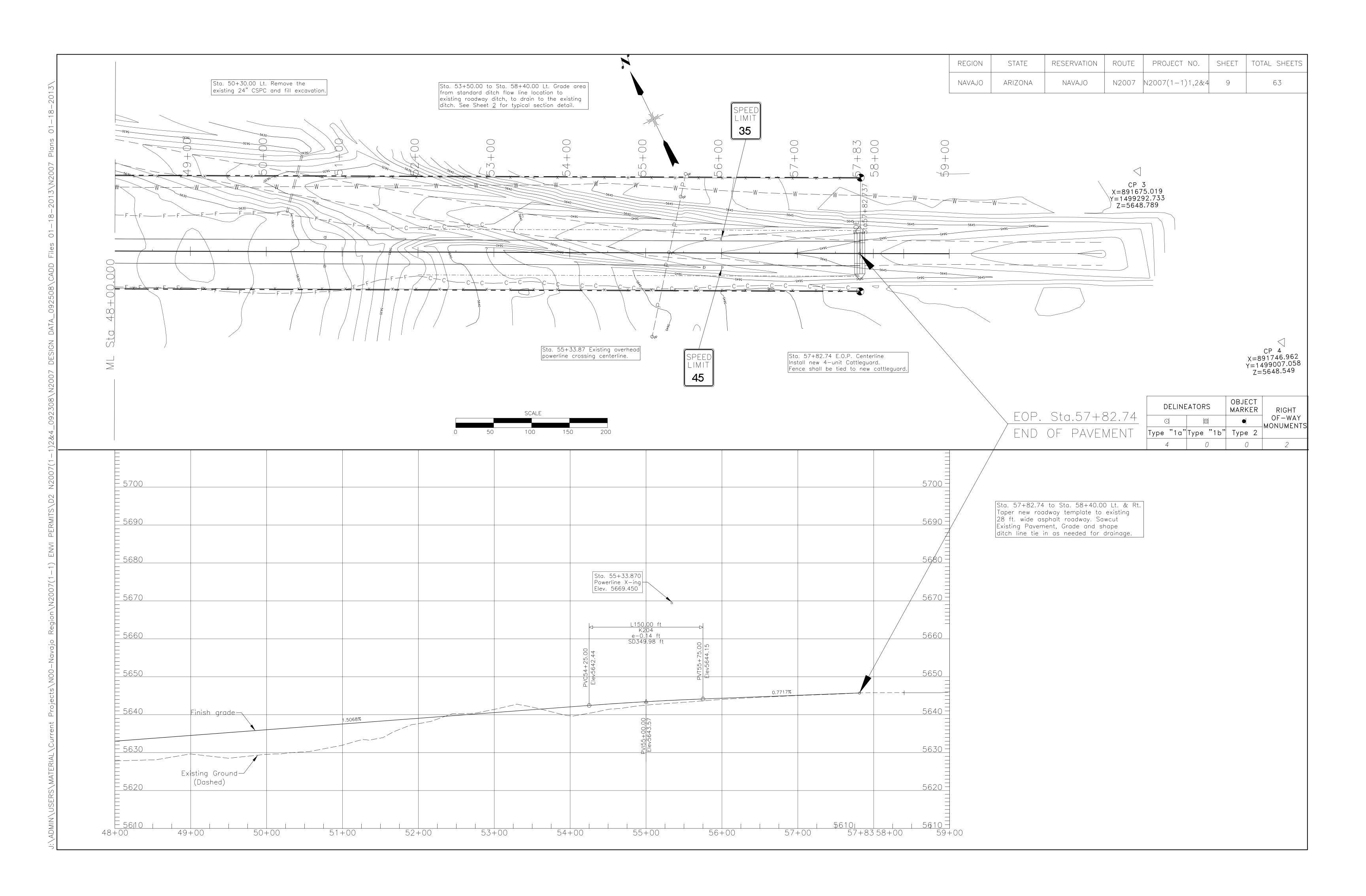
DRAWN BY: Peterson. Yazzie DATE: 1/28/2013 DATE: 1/28/2013 DESIGNED BY: NRDOT REVISED: 8/16/2016 BY: Gerald.Hood ANNOTATION SCALE: Full Size 1=1FILENAME: Sht.4_Est. Quantities.dgn











REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
OLAVAN	ARIZONA	OLAVAN	N2007	N2007(1-1)1,2&4	10	63

N2007 HORIZONTAL ALIGNMENT TABLE REPORT *

Point	Station	Direction	Style:	Northing	Easting	Radius	Length	Delta /	Rotation
Туре			·					Theta	Direction
POB	00+00.000		Tangent	1,503,109.49	887,089.42				
TS	30+61.099	S37°07'34"E	Tangent	1,500,668.85	888,937.01				
TS	30+61.099	S37°07'34"E	Clothoid	1,500,668.85	888,937.01				
SPI	32+01.220		Clothoid	1,500,557.13	889,021.59		210.00	7°21'00"	Left
SC	32+71.099		Clothoid	1,500,507.11	889,070.71				
SC	32+71.099		Arc	1,500,507.11	889,070.71				
PI	33+57.017		Arc	1,500,445.80	889,130.90	818.51	171.21	11°59'05"	Left
CC			Arc	1,501,080.57	889,654.75				
CS	34+42.308	S63°48'39"E	Arc	1,500,398.33	889,202.52				
CS	34+42.308		Clothoid	1,500,398.33	889,202.52				
SPI	35+12.418		Clothoid	1,500,359.59	889,260.95		210.00	7°21'00"	Left
ST	36+52.308		Clothoid	1,500,297.75	889,386.69				
ST	36+52.308	S63°48'39"E	Tangent	1,500,297.75	889,386.69				
POE	57+82.737		Tangent	1,499,357.52	891,298.41				

* SPC-NAD83-AZ-E CSF: 1.0003293134 (0.9996708050) U.S. SURVEY FOOT

N2007 FRONTAGE ROAD ALIGNMENT *

DESCRIPT	STATION	DIRECTION	NORTHING (ft)	EASTING(ft)
POB	0+00.000	N 75°52'26" E	1,501,529.971	888,285.138
PC	0+98.303	N 75°52'26" E	1,501,553.962	888,380.469
PI	1+29.295	S 31°16'47" E	1,501,561.526	888,410.523
PT	1+51.702	S 31°16'47" E	1,501,535.039	888,426.614
PC	5+96.511	S 31°16'47" E	1,501,154.886	888,657.566
PI	6+25.054	S 55°52'03" W	1,501,130.493	888,672.385
PT	6+42.141	S 55°52'03" W	1,501,114.478	888,648.760
POE	9+13.599	S 55°52'03" W	1,500,962.160	888,424.063

N2007 CONTROL POINTS *

POINTS DESCRIPTION	STATION	LOCATION OFFSET (FT.)	NORTHING	EASTING	ELEVATION
102	33+56.704	25.012	1500429.978	889117.844	5,627.110
113	33+56.704	25.012	1500843.914	888977.977	5,626.370
122	26+38.067	-194.521	1501123.547	888836.777	5,632.681
129	29+32.510	-203.417	1500894.154	889021.587	5,631.985
808	31+63.893	-73.288	1500633.547	889056.937	5,627.260
CP1	31+52.056	-88.679	1500651.986	889061.887	5,627.424
CP2		50.209	1503016.643	887096.738	5,676.494
CP3			1499292.733	891675.019	5,648.789
CP4			1499007.058	891746.962	5,648.549

** UTILITY CROSSING INFORMATION

STATION	DESCRIPTION	LOCATION	DEPTH	HEIGHT	SKEW	OWNER	REMARKS
1+10.00	Power pole with anchor	50' Left	_	_	_	Navajo Tribal Utility Authority	To Remain in Place
3+68.00	Water Line	CL	3 ft.	_	90°	Navajo Tribal Utility Authority	To Remain in place
3+74.71	Power Line	CL	_	20 ft.	110°	Navajo Tribal Utility Authority	To Remain in Place
3+90.00	Telephone Line	CL	_	_	_	Unknown	To Remain in Place
3+95.00	Power pole	47' Left	_	_	_	Navajo Tribal Utility Authority	To Remain in Place
3+98.00	6-inch PVC Riser	40' Left	_	_	_	Navajo Tribal Utility Authority	To Remain in Place
9+43.45	Power Line	CL	_	20 ft.	90°	Navajo Tribal Utility Authority	To Remain in Place
10+96.26	Power Line	CL	_	20 ft.	90°	Navajo Tribal Utility Authority	To Remain in Place
15+52.00	Telephone Line	CL	_	_	_	Casing may be required	To be relocate by owner
25+00 to 32+00	Water line	Lt. & Rt.	Varies	_	_	Navajo Tribal Utility Authority	To be relocate by owner
25+20.00	Signal control panel and battery box	30', left	_	_	_	BNSF	To Remain In Place
33+40 - 34+40	Buried Telephone Line	Lt. & Rt.	_	_	60°	Table Top Telephone Co.	To Remain In Place
35+50 to 57+83	Water Line	85' Left	3 ft.	_	_	Navajo Tribal Utility Authority	To Remain In Place
55+31.00	Power Line	CL	_	20 ft.	100°	Navajo Tribal Utility Authority	To Remain In Place

** Remarks related to work by owners is work to be required by B.I.A. for Construction of Roadway and Structures. Actual work required / performed by utility owner(s) may vary.

REVISED ON 12/12/2013

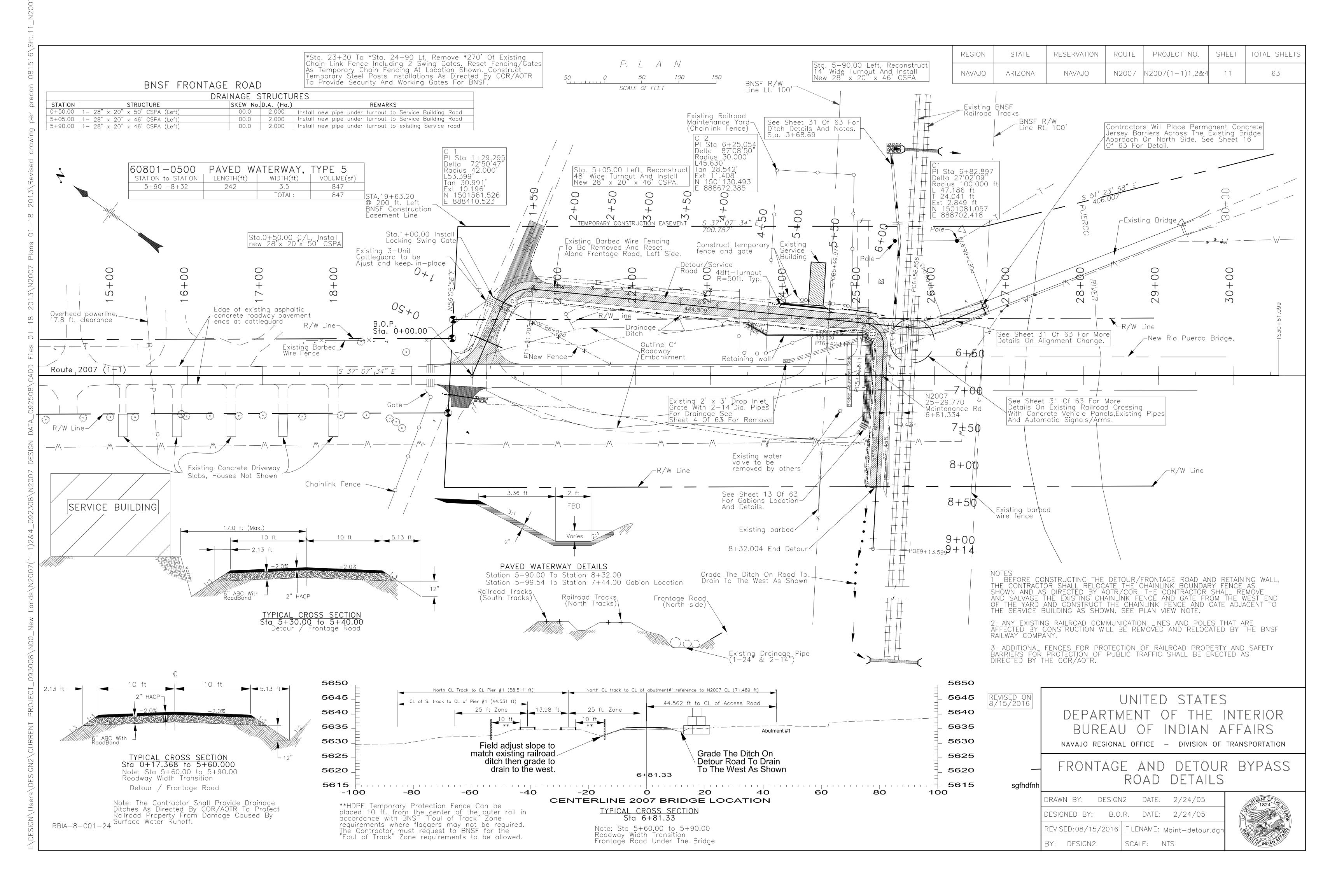
UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

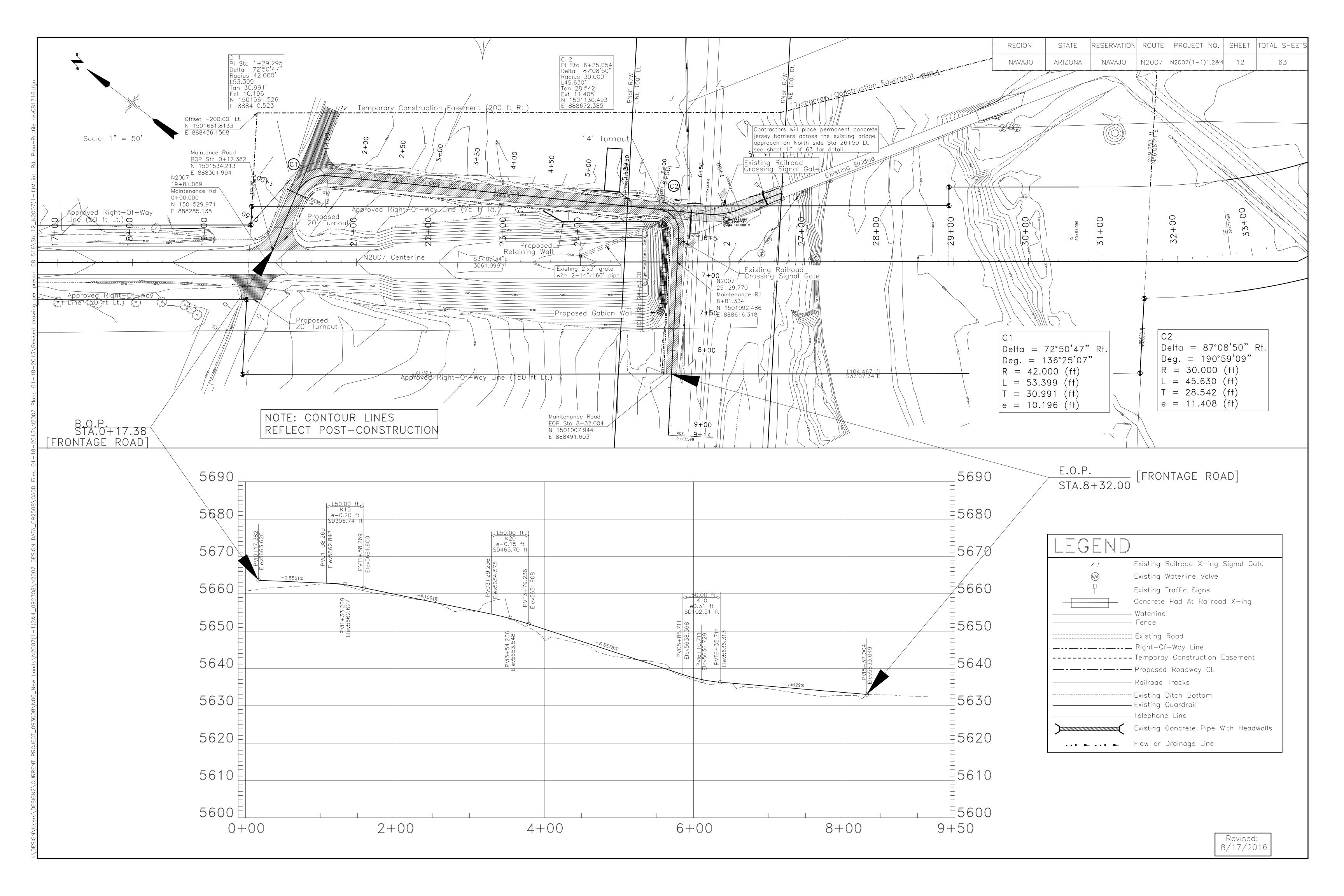
ALIGNMENTENT TABLE AND CONTROL POINTS AND UTILITY CROSSING

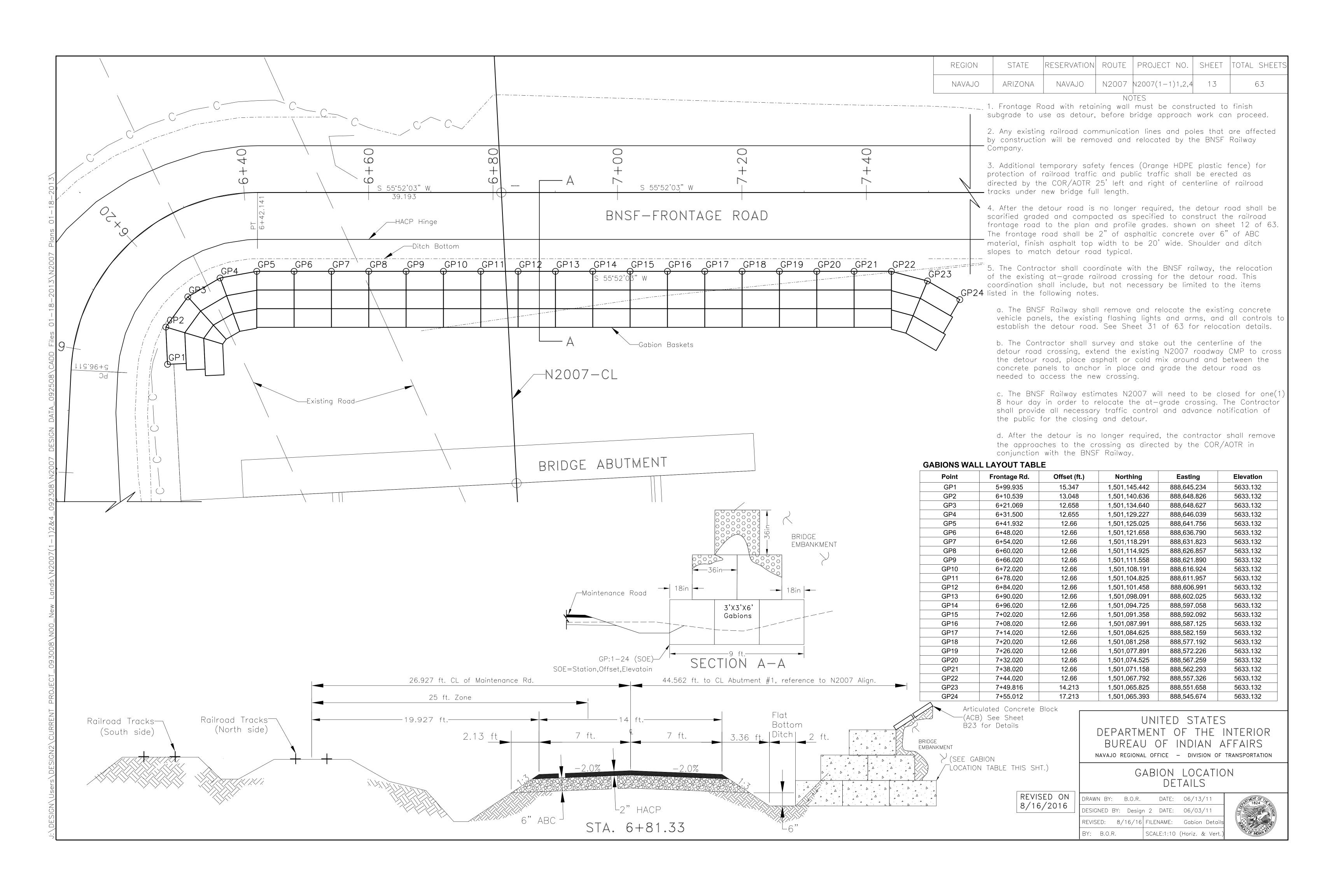
DRAWN BY: Peterson.Yazzie DATE: 5/3/2010 DESIGNED BY: NRDOT DATE: 5/3/2010 REVISED: 1/25/2013 BY: Peterson.Yazzie ANNOTATION SCALE: 1:10

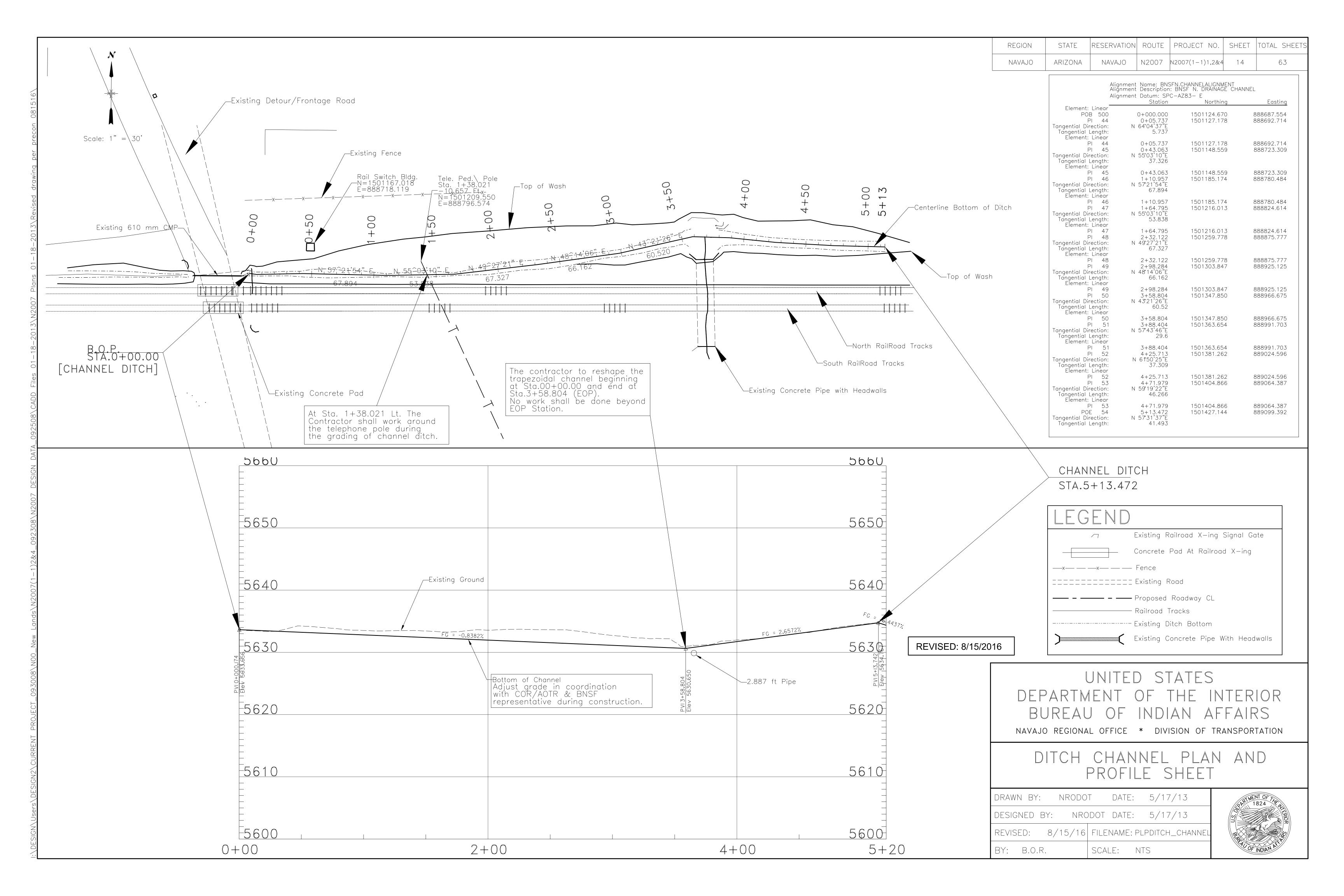
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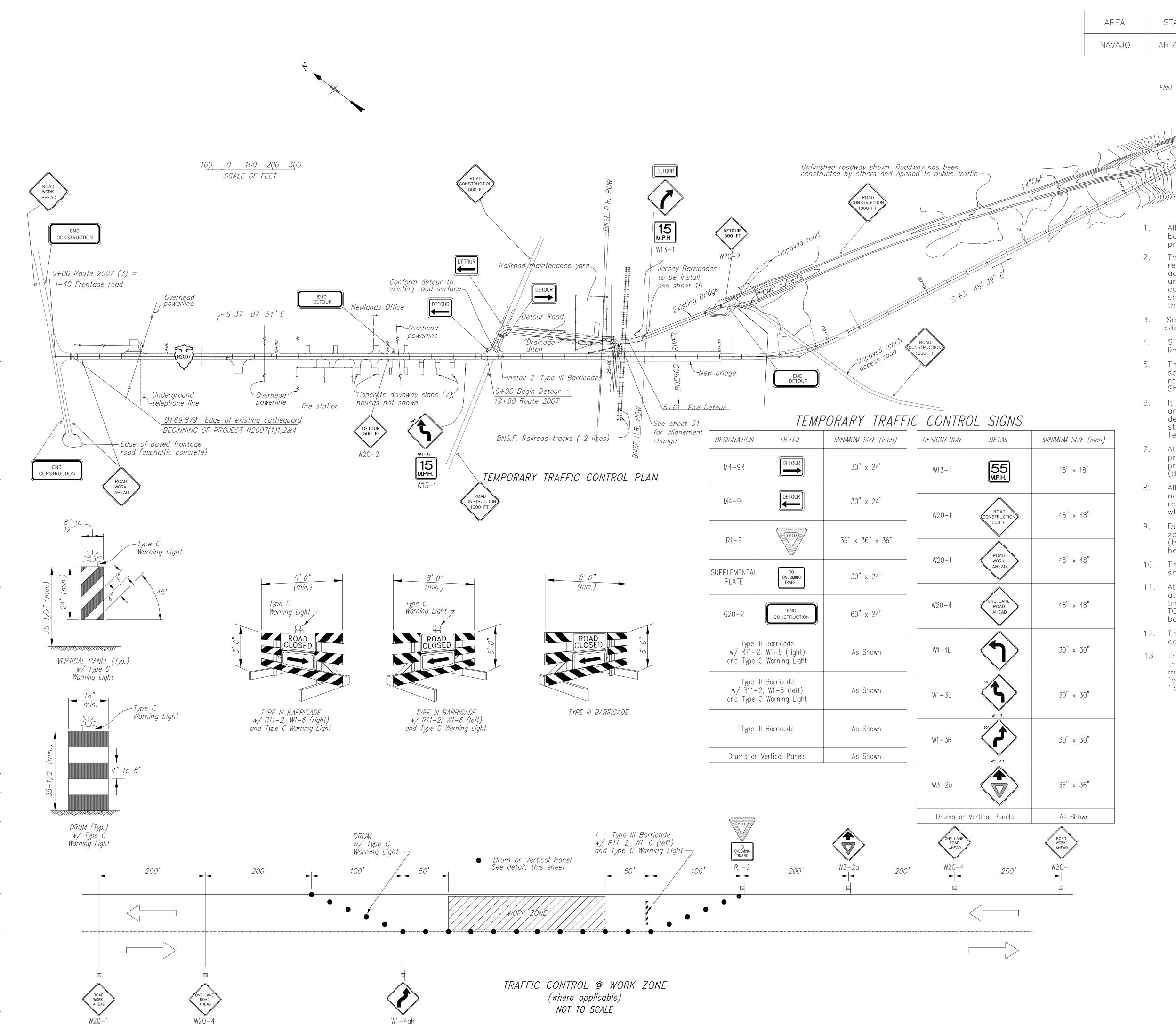












AREA STATE RESERVATION ROUTE PROJECT NO. SHEET TOTAL SHEETS

NAVAJO ARIZONA NAVAJO N2007 N2007(1-1)1,2&4 15 63

57+82.74 Route 2007 (3)=

Existing roadway

END OF PROJECT N2007(1)1,2&4

Install 2-Type III Barrocade

GENERAL NOTES

- All Traffic Control Devices shall be in accordance with the MUTCD (Latest Edition and amendments) and the supplemental specifications for this project.
- The Traffic Control details shown are only a guide. The Contractor is solely responsible for preparing and implementing a Traffic Control Plan (TCP) in accordance with these details, Section 635 of FP-03, and the MUTCD under Contract Item 63501-0000. Any additional traffic control devices called for on the Contractor's TCP will not be measured for payment but shall be considered incidental to the bid items for traffic control shown in the bid schedule.
- 3. See General Notes on 3 of 63 and the supplemental specifications for additional TCP requirements.
- I. Signs W20−1, W20−1a, G20−1, and G20−2a shall be placed at the project limits and remain in place through the duration of the project.
- 5. The Contractor shall construct the frontage/detour road to the typical section on Plan Sheet 11 to subgrade only. After the road is no longer required, as detour, the Contractor shall complete the construction as Plan Sheets 11 and 12.
- 6. It is the responsibility of the Contractor to maintain the existing roadway and/or detour in a drivable condition during construction. The cost of any detour roads (including all detour related earthwork, temporary drainage structure and maintenance) shall be considered incidental to the Temporary Traffic Control bid items.
- 7. At the end of each working day, it will bethe Contractor responsibility to provide a driving surface free of obstructions. Access to all adjoining properties and BA/County system routes shall bemaintained at all times (day and night).
- 8. All traffic control devices (except at detour road locations and as noted in note #4 above) such as construction signs, drums, barricades,etc., shall be removed to a location at least 32 feet from the edge of the shoulder when construction is not in progress.
- 9. During construction operations, traffic shall be moved through the work zone using pilot cars (as required). Applicable signs and other items (two—way radio contact) related to the pilot cars and traffic control shall be considered incidental obligations of the Contractor.
- 10. The Contractor has the option to either use drums or vertical panels, but shall not use a combination of both. No traffic cones are allowed.
- 11. At locations where new road construction intersects existing roadways and at tie—ins with existing roads at the endof the new construction "Special traffic control" procedures shall be included as needed in the Contractor's TCP. These include, but are not necessarily limited to the following: Type III barricades with "Road Closed/ detour sign", flaggers, drums, etc.
- 12. The Traffic Control on this project shall be coordinated with the project construction schedule. The Contractor's TCP shall reflect this coordination.
- 13. The Contractor may install temporary HDPE plastic fence 15 feet out from the outside rail of the railroad tracks so that all work, equipment, and materials are kept outside these limits. This fence installation shall be paid for under bit item 61901—3400. This may reduce the need for railroad flaggers.

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

TEMPORARY TRAFFIC CONTROL

DRAWN BY: Gerald.Hood DATE: 5/7/2009

DESIGNED BY: NRDOT DATE: 5/7/2009

REVISED: 05/20/2013 BY: Gerald.Hood

ANNOTATION SCALE: Full Size 1=1

FILENAME: Sht.15_Traffic Control Details.dgn



	PERMANENT ROADSIDE SIGNS									
STATION	LOC.	SIZE DETAIL NO.	DESCRIPTION	SIGN PANEL SIZE INCH	AREA OF SIGN ft ²	NO. Of POSTS	POST WEIGHT Ib/ft	TOTAL PANELS		
19+68.00 19+86.00	Lt. Rt.	R1-1 R1-1	STOP	30" × 30"	6.25	2 2	2.00	2		
1+00.00 19+00.00 56+00.00	Rt. Rt. Lt.	R2-1(35)	SPEED LIMIT 35	24" × 30"	5.0	1 1 1	2.75	3		
56+00.00	Rt.	R2-1(45)	SPEED LIMIT 45	24" × 30"	5.0	1	2.75	1		
2+00.00	Rt.	M – 1	2007	18" × 24"	3.0	1	2.75	1		
39+00.00	Lt.	W1-2R		30" × 30"	6.25	2	2.00	1		
5+00.00 12+00.00 41+00.00	Lt. Rt. Lt.	M – 1	DO NOT PASS	24" × 30"	5.0	1 1 1	2.75	3		
5+00.00 12+00.00 41+00.00	Rt. Lt. Rt.	R4-1	NO PASSING ZONE	48" × 48" × 36"	5.55	2 2 2	2.00	3		
2+00.00	Lt.	W3-1a		30" × 30"	6.25	2	2.00	1		

63302-0003 Sign Installation, 1 Post & Hardware: 2.75 lbs/ft.___ 38.00 sq.ft.

63302-0010 Sign Installation, 2 Post & Hardware: 2.00 lbs/ft.___ 41.65 sq.ft.

63401-161	0 P	AVEMENT	MARKINGS:	SOLID YEL	LOW
STATION	STATION TO STATION			DECRIPTION	LENGTH (Ft.)
0+69.879	То	5+00.00	Center-Right	Solid Yellow	430.12
0+69.879	То	5+00.00	CenterL-Left	Solid Yellow	430.12
12+00.00	То	41+00.00	Center-Right	Solid Yellow	2,900.00
12+00.00	То	41+00.00	Center-Left	Solid Yellow	2,900.00
				TOTAL:	6,660.24

63401-1520 PAVEMENT MARKINGS: SOLID WHITE

STATION TO STATION	LOCATION	DECRIPTION	LENGTH (Ft.)
0+69.879 To 57+82.74	Right	Solid White	5,712.86
Minus (1) 14' T.O. @ 68	,		-68.00
Minus (6) 16' T.O. @ 70	,		-420.00
Minus (5) 24' T.O. @ 11		-590.00	
		SUB-TOTAL:	4,634.86
0+69.879 To 57+82.74	Left	Solid White	5,712.86
Minus (1) 40' T.O. @ 13	4'		-134.00
Minus (3) 16' T.O. @ 70),		-210.00
Minus (3) 24' T.O. @ 11	8'		-354.00
		SUB-TOTAL:	5,014.86
		GRAND-TOTAL:	9,649.72

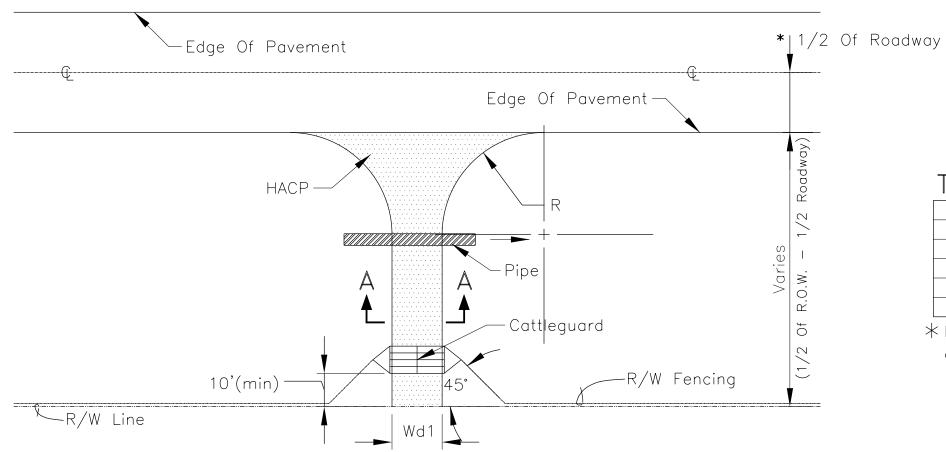
63401-1510 PAVEMENT MARKINGS: BROKEN YELLOW STATION TO STATION LOCATION DESCRIPTION LENGTH (Ft)
 5+00.00
 To
 12+00.00
 Center
 Broken Yellow

 41+00.00
 To
 57+82.74
 Center
 Broken Yellow
 TOTAL: 2,382.74

MAINTENANCE/DETOUR ROAD

63401-1510 PAVEMENT M			MARKINGS:	SOLID YEL	LOW	
STATION TO STATION			LOCATION	LOCATION DESCRIPTION		
0+17.38 To 5+40.00			Center	Solid Yellow	522.62	
				TOTAL:	522.62	

63502-3000 TTC, RAISED PAVEMENT MARKINGS: 63502-3000 TTC, RAISED PAVEMENT MARKERS @ 700 TOTAL ★ See Typical Section Detail For Roadway Width



TYPE "A" TURNOUT 4-Unit 6-Unit *Radius 10', for housing street/ driveway approaches.

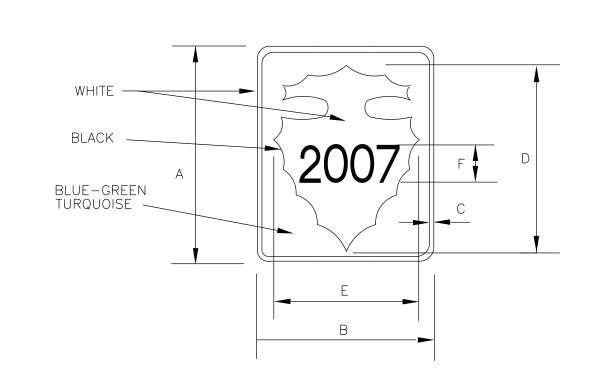
REGION

OLAVAIO

STATE

ARIZONA

OLAVAIO



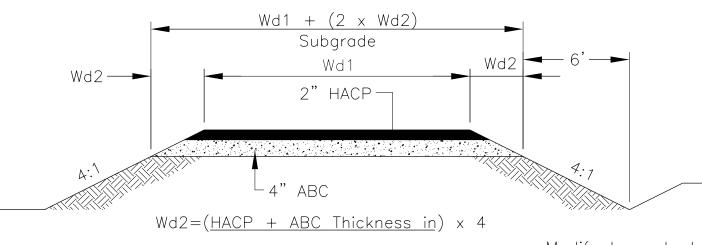
RESERVATION ROUTE PROJECT NO. SHEET TOTAL SHEETS

63

16

N2007 N2007(1-1)1,2&4

TYPICAL TYPE "A" TURNOUT Special width turnouts at Sta.3+55 Lt. and Sta.13+34 Rt., to be constructed similar. Radii to be 30'.



SECTION A-A

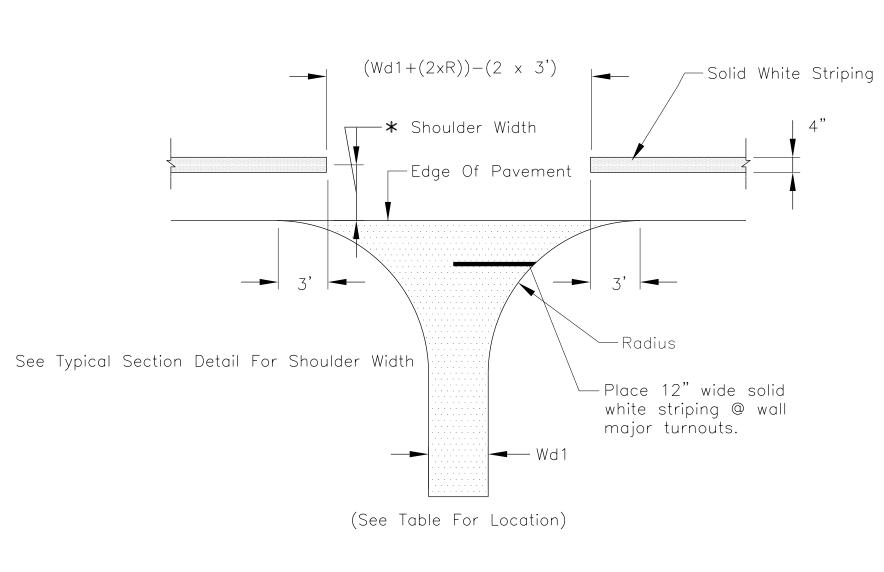
Modify turnout shoulder/ditch grading to fit existing drainage and ground conditions, if directed by the C.O.R./AOTR

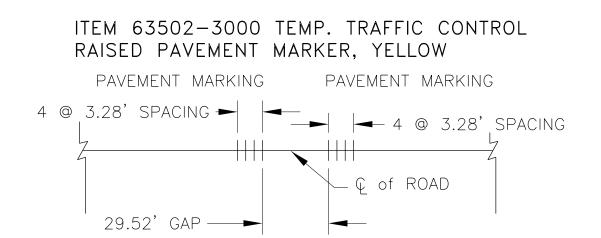
SIGN		DIME	NSION	inches		F = NUM	1ERALS			
	А	В	С	D	E	DIGITS IN ROUTE	1	2	3	4
MIN.	24"	18"	1/2"	19-1/2"	13-1/2"	SIZE	14 1/2"	12"	9"	8"

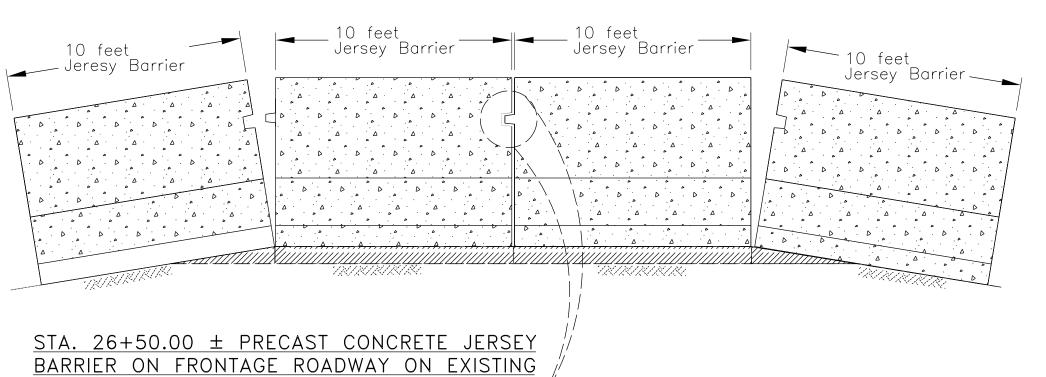
Roadway Centerline	Broken Yellow St	riping —	4"
10'	30'	10'	
TYPICAL PAVEN			YELLOW"
	(See Table For Lo	ocation)	

BRIDGE APPROACH

STOP SIGN and S	TOP BAR IC	Y (m)=		
Radius of turnout (ft)	X (ft)	y' (ft)	y' + LO (ft)	Length of Stop Line
10.00	5.00	5.00	7.00	1/2 Roadway width + Y
20.00	10.00	10.00	8.50	1/2 Roadway width + Y
30.00	15.00	15.00	10.00	1/2 Roadway width + Y
40.00	20.00	20.00	11.00	1/2 Roadway width + Y
50.00	25.00	25.00	12.50	1/2 Roadway width + Y







0.25" gap for ___expansion and contraction

PRECAST JOINT DETAIL

(a). The Precast Jersey Barrier shall be install on the North side of the existing bridge approach as shown and location determined by AOTR.

(b). The Precast Jersey Barrier construction shall meet the requirements of Safety-Shape Barrier (SGM11a-b) in the AASHTO-AGC-ARTBA Joint Committee Task Force 13 Report.

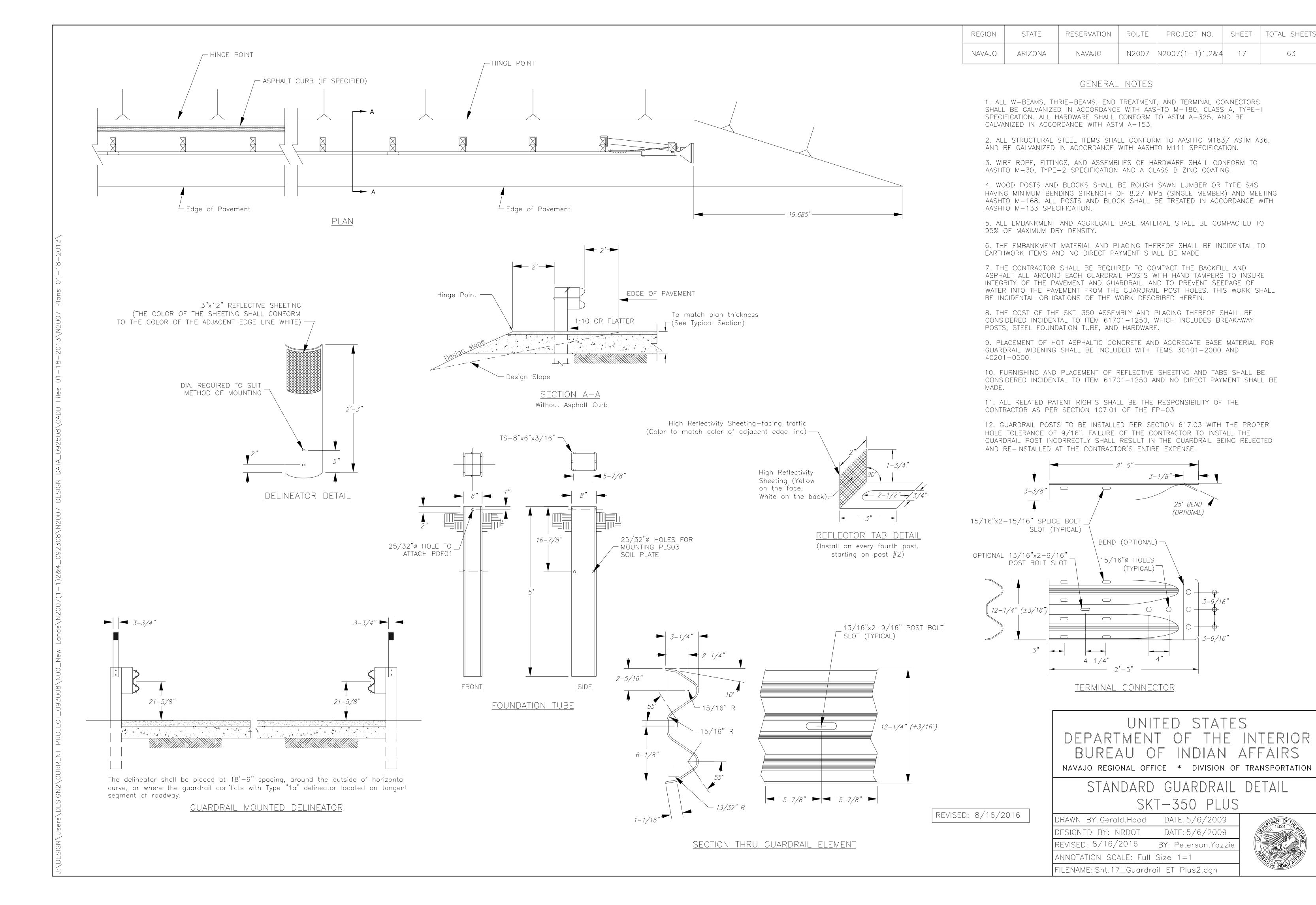


SIDE VIEW

PERMANENT PAVEMENT MARKINGS ROADSIDE SIGNS & TURNOUT DETAIL

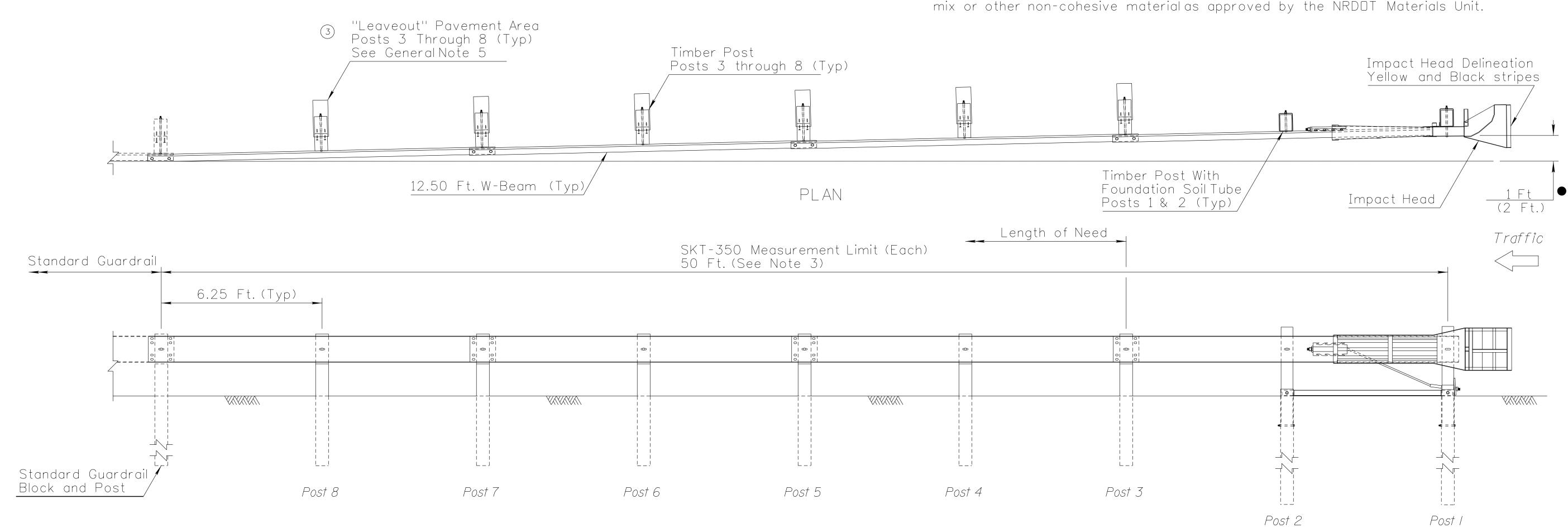
DRAWN BY: Gerald. Hood	DATE: 5/7/2009
DESIGNED BY: NRDOT	DATE:5/7/2009
REVISED: 6/4/2013	BY: Gerald.Hood
ANNOTATION SCALE: Full	Size 1=1
FILENAME: Sht.16_PermSi	gn & T.O Details.dgn





	REGION	STATE RES	SERVATION ROUTE	PROJECT	SHEET	TOTAL SHEETS
	NAVAJO	AZ N	NAVAJO N2007	N2007(1-1)1,2&4	4 18a	63
● FOR ELEVATIONS ABOVE 4002 (Ft.), USE THE VALUES IN PARENTHESES	CENIED AL NOTES					

- GENERAL NOTES
- 1. This detailis for roadway layout only.
- 2. The SKT-350 shall be installed in accordance with the manufacturer's specifications and current approved drawings including all details, hardware, hardware quantities, and other information as shown in these plans.
- 3. The 50 Ft. W-Beam length shall consist of four 12.50 Ft. sections, the end section being a proprietary split rail.
- 4. See specifications and other drawings and details in these plans.
- 5. "Leaveouts" in asphaltic concrete shallbe provided in the AC pavement around the guardrail posts at the locations and dimensions specified on the Road Systems Inc. approved drawing (SKT-350 Sht 3 of 3) shown in these plans. "Leaveout" material shall consist of a 1-sack grout mix or other non-cohesive material as approved by the NRDOT Materials Unit.



ELEVATION

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

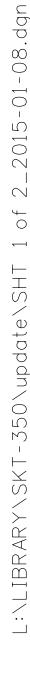
GUARDRAIL END TREATMENT

SKT-350 LAYOUT; SHEET 1 of 3

REVISED ON 4/21/2015



File Name: SKT-350 Sht 1 of 3_2015-02-13



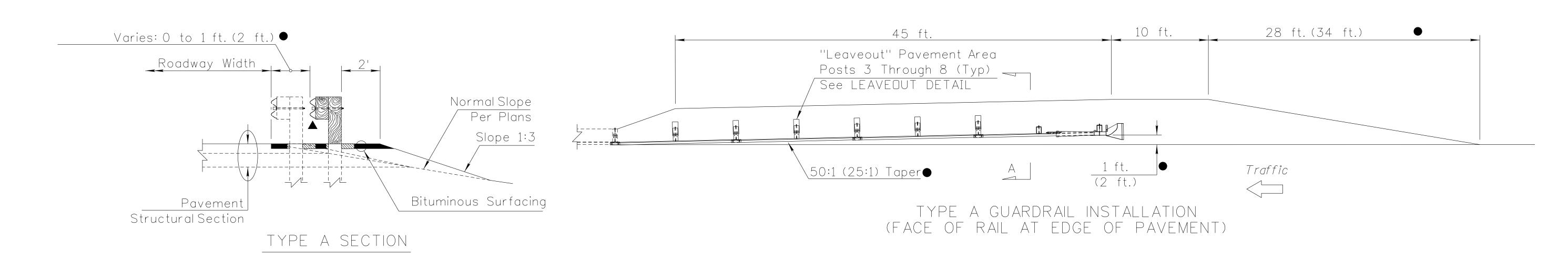
REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
NAVAJO	AZ	NAVAJO	N2007	N2007(1-1)1,2&4	18b	63

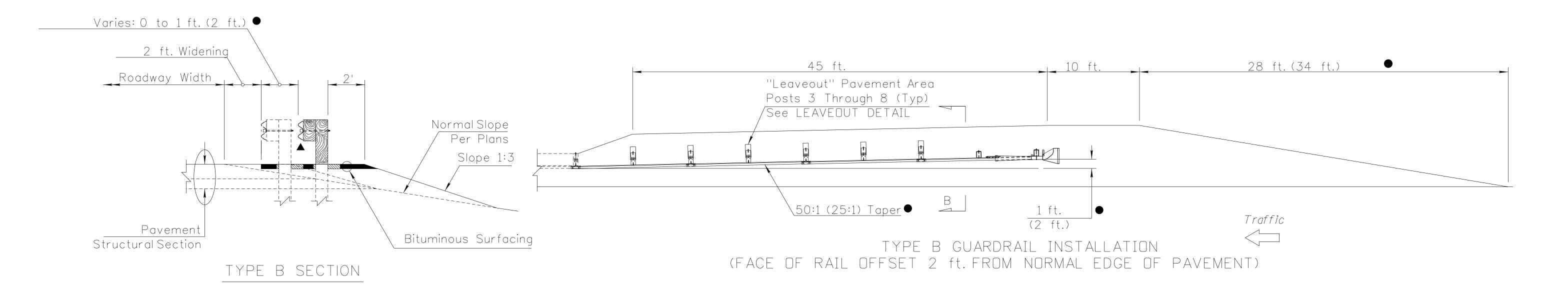
• FOR ELEVATIONS ABOVE 4002 (Ft.), USE THE VALUES IN PARENTHESES

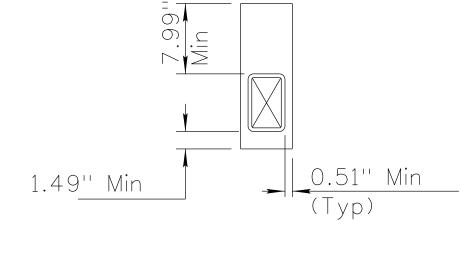
▲ Top of Rail to Roadway Surface = 28"

REVISED ON

4/21/2015







LEAVEOUT DETAIL

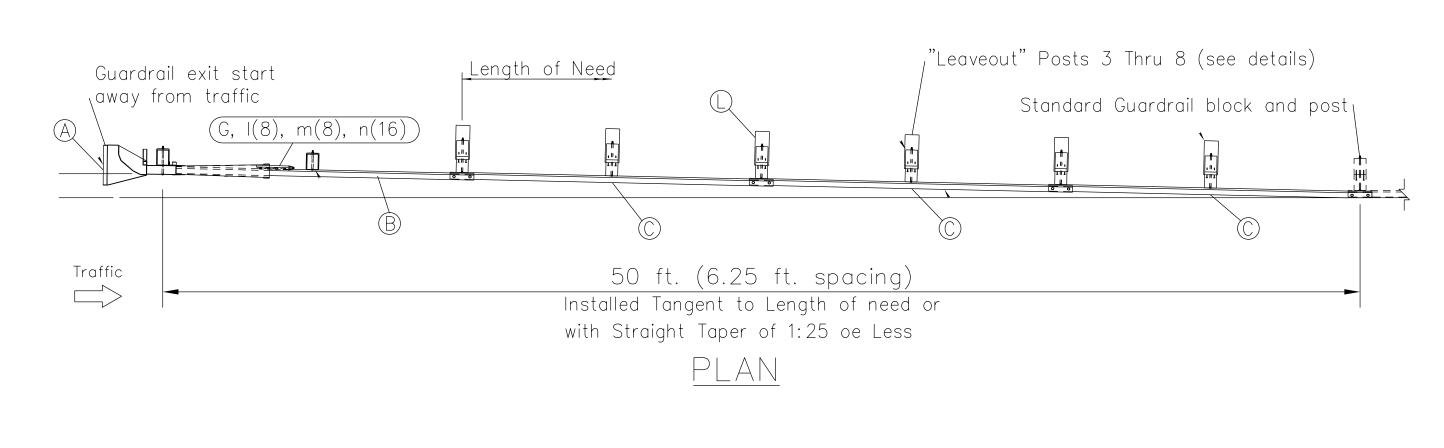
UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

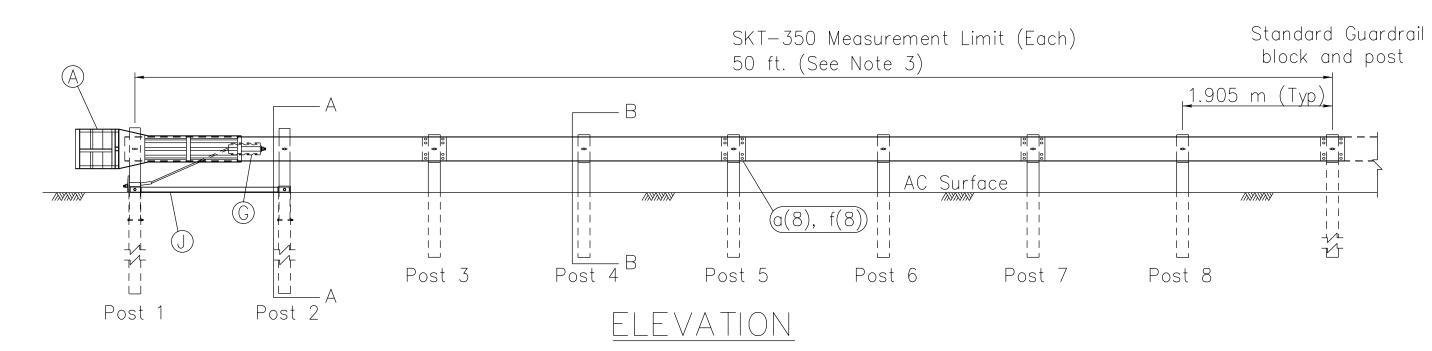
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

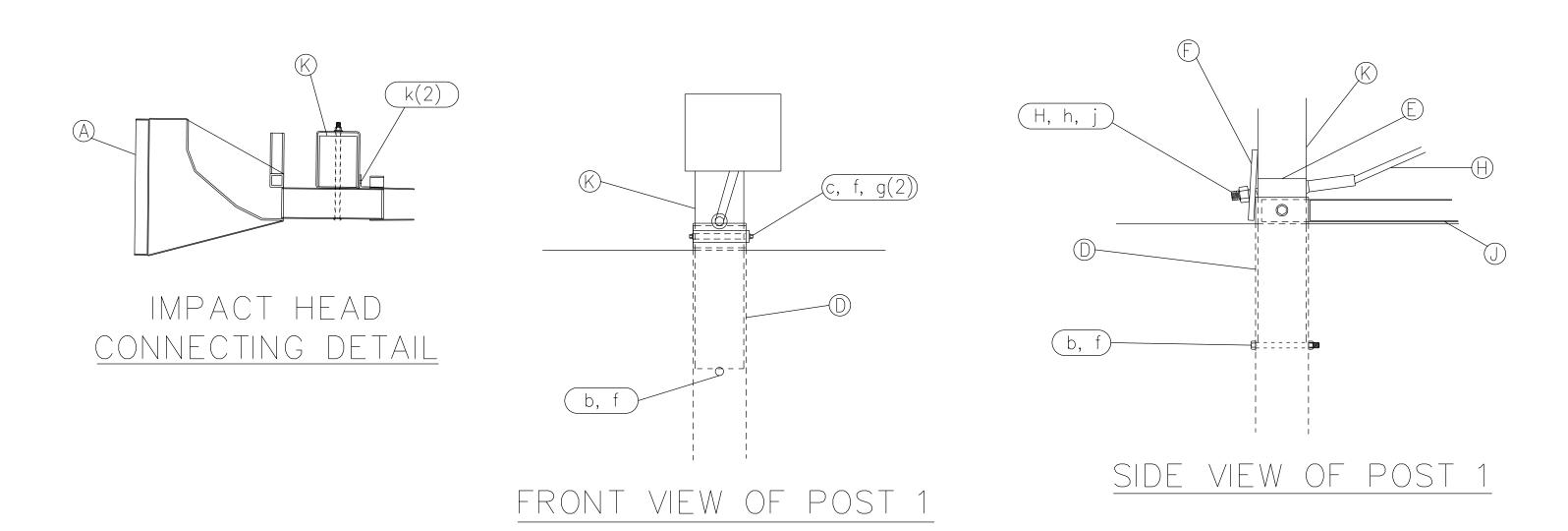
GUARDRAIL END TREATMENT SKT-350 LAYOUT; SHEET 2 of 3

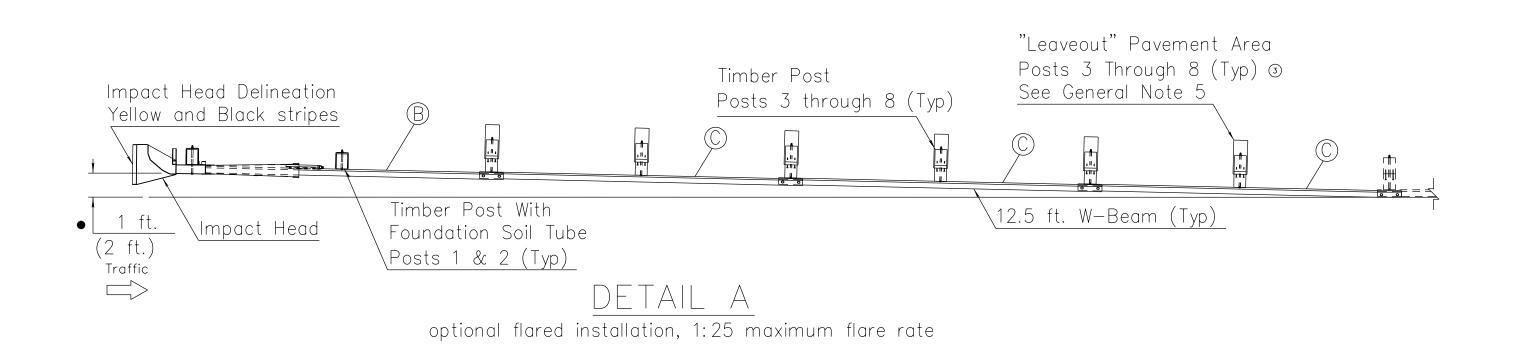
Designed by: BIA NRN-DAT Structural Unit

Designed by. DIA NIN	J-DDT Structuraronit
Drawn by:	Date: 2/13/15
Revised by:	Date:
File Name: SKT-350	Sht 2 OF 3_2015-02-13









GENERAL NOTES 1. Breakaway posts are required with the Sequential Kinking Terminal.

2. All bolts, nuts, cable assemblies, cable anchors and bearing plates shall be galvanized. 3. The SKT can be flared at a rate of 1:25 to prevent the impact head from encroaching on the shoulder. The flare is not required and may be decreased or eliminated fot specific installations. 4. The soil tubes shall not protrude more than 4" above ground (measured along a 4.92 ft. chord). Site grading may be necessary to meet this requirement. 5. The soil tubes may be driven with an approved driving head. Soil tubes should not be driven with the post in the tube. If the tubes are placed in drilled holes, the backfill material must be satisfactorily compacted to prevent settlement. 6. When rock is encountered during excavation, a 1 ft. Dia. post hole, 20" into rock may be used if approved by the engineer. Granular material will be placed in the bottom of the hole approx. 2.51" deep to provide drainage. The soil tubes will be field cut to length, placed in the hole and backfilled with adequately compacted material excavated from the hole.

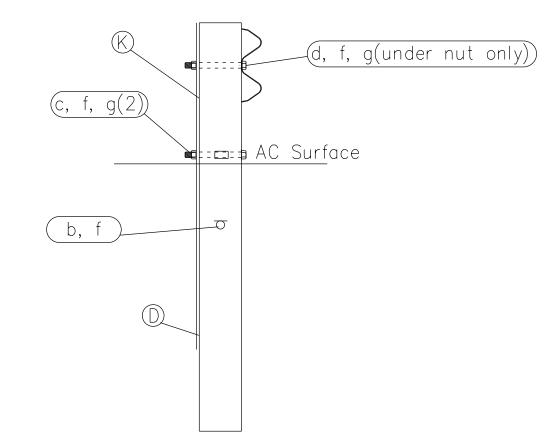
7. The breakaway cable assembly must be taut. A locking device, (vice-grips or channel-lock pliers) should be used to prevent the cable from twisting when tightening nuts.

8. A special site evaluation should bee considered prior to using the SKT where there is less than 25 ft. between the outlet side of the SKT and any adjacent driving lane.

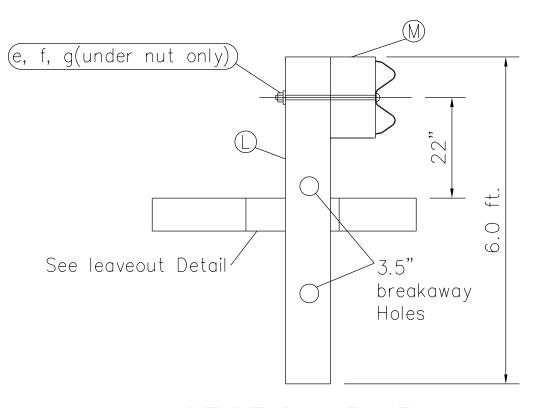
9. The wood blockouts should be "toe-nailed" to the wood posts to prevent them from turning when the wood shrinks.

10. Guardrail splice shall be overlapped in the direction of adjacent traffic.

11. Bill of materials and some of the details herein were provided by Road Systems Inc.



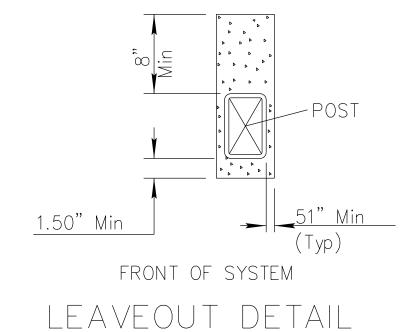
SECTION A-A



SECTION B-B typical at Post 3 thru 8

REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
NAVAJO	AZ	NAVAJO	N2007	N2007(1-1)1,2&4	18c	63

Code	QTY.	BILL OF MATERIALS	ITEM#
А	1	IMPACT HEAD	S3000
В	1	W-BEAM GUARDRAIL END SECTION, 12 GA., 12.5 ft.	FS1303
С	3	W-BEAM GUARDRAIL, 12 ga., 12.5 ft RAIL ELEMENTS	G1203
D	2	FOUNDATION SOIL TUBE, 6" x 8" x 6'	E731
E	1	PIPE SLEEVE	E740
F	1	BEARING PLATE, 8" x 8" x 0.63"	E750
G	1	CABLE ANCHOR BOX	S760
Н	1	BCT CABLE ANCHOR ASSEMBLY	E770
J	1	GROUND STRUT	E780
K	2	5.51" x 7.51" x 3.75' WOOD POSTS	P650
L	6	6" x 8" x 6' WOOD CRT POST	P671
М	6	6" x 8" x 14" TIMBER BLOCKOUT	P675
		HARDWARE	
а	32	0.63" Dia. x 1.25" SPLICE BOLT	B580122
b	2	0.63" Dia. x 7.52" HEX BOLT	B580754
С	2	0.63" Dia. x 10" HEX BOLT	B581004
d	1	0.63" Dia. x 7.52" H.G.R. BOLT (post 2 only)	B581002
е	6	0.63" Dia. x 18" H.G.R. BOLT (posts 3-8)	B581802
f	43	0.63" Dia. H.G.R. NUT	N050
g	11	H.G.R. WASHER	W050
h	2	0.98" ANCHOR CABLE HEX NUT	N100
j	2	0.98" ANCHOR CABLE WASHER	W100
k	2	0.37" X 3" LAG SCREW	E350
	8	CABLE ANCHOR BOX SHOULDER BOLTS	SB58A
m	8	0.50" A325 STRUCTURAL NUTS	N055A
n	16	1.06" OD x 0.55" ID A325 STR. WASHER	W050A



Leaveout Requirement:

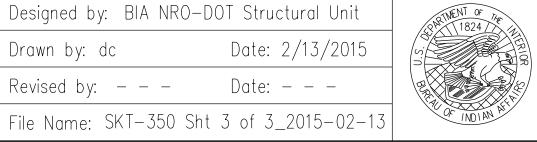
For posts 3 through 8, leaveouts in the asphaitic concrete pavement, as shown in Section B-B and LEAVEOUT DETAIL, around guardrail posts shall be provided where asphalt thickness exceeds 1.53" nominal (2" max) and where the asplalt extends more than 18" behind the posts. The leaveout shall be used at the post locations indicated and the leaveout material to place in the void shall consist of a 1-sack grout mix or other non-cohesive material as approved by the NRDOT Materials Unit.

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

GUARDRAIL END TREATMENT SKT-350 LAYOUT; SHEET 3 of 3

REVISED ON 4/21/15

Designed by: BIA NRO-DOT Structural Unit Date: 2/13/2015 Drawn by: dc Date: - - -Revised by: - - -

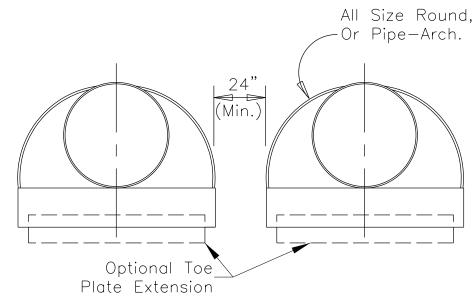


APPROVED AS NCHRP 350 T3 TERMINAL

ROUTE PROJECT NO. SHEET TOTAL SHEETS STATE RESERVATION REGION OLAVAJO ARIZONA NAVAJO N2007 | N2007(1-1)1,2&419 63

GENERAL NOTES

- 1. FOR MULTIPLE INSTALLATION OF ALL TYPES, A MINIMUM OF A 24" SPACING MEASURED ALONG THE HORIZONTAL BETWEEN FLARED END SECTIONS AT THEIR WIDEST CROSS SECTION SHALL BE USED.
- 2. ALL THREE (3) PIECE BODIES TO HAVE 0.109" THICKNESS SIDES AND 0.138" THICKNESS CENTER PANELS. WIDTH OF CENTER PANELS TO BE GREATER THAN 20% OF THE PIPE PERIPHERY. MULTIPLE PANEL BODIES TO HAVE LAP SEAMS WHICH ARE TO BE TIGHTLY JOINED BY 0.375" Ø GALVANIZED RIVETS OR BOLTS.
- 3. END SECTIONS FOR STEEL PIPE-ARCHES: FOR THE 77" x 52" AND 83" x 57" SIZES, REINFORCED EDGE TO BE SUPPLEMENTED BY 2" x 0.25" GALVANIZED ANGLES. THE ANGLES TO BE ATTACHED BY 0.375" DIA. GALVANIZED NUTS AND BOLTS. ANGLE REINFORCEMENT WILL BE PLACED UNDER THE CENTER PANEL SEAMS.
- 4. END SECTIONS FOR STEEL CIRCULAR PIPES: FOR 60" ø THRU 84" ø SIZES. REINFORCED EDGE TO BE SUPPLEMENTED WITH GALVANIZED STIFFENER ANGLES. THE ANGLES WILL BE 2" x 2" x 0.25" FOR 60" ø THRU 72" ø, AND 2.52" x 2.52" x 25" FOR 78"ø AND 84" ø. THE ANGLES TO BE ATTACHED BY 0.375" ø GALVANIZED NUTS AND BOLTS.
- 5. WELDING SHALL NOT BE PERMITTED IN CONNECTING END SECTIONS TO CONNECTOR SECTIONS OR CONNECTOR SECTIONS TO PIPE.
- 6. TYPE NO. 1 STEEL END SECTION, CONNECT END SECTION WITH THREADED ROD WITH CONNECTOR LUG, FOR 24" ø ROUND PIPE AND 28" x 20" CSPA.
- 7. TYPE NO. 2 STEEL END SECTION, CONNECT END SECTION WITH THREADED ROD WITH ROD HOLDER FOR 30" ø AND 36" ø ROUND PIPE AND 17" x 13" THRU 57" x 38" CSPA.
- 8. TYPE NO. 3 STEEL END SECTION, THE CONNECTION INCLUDES 12" OF THE PIPE LENGTH AS A CONNECTOR SECTION FOR PIPE ARCH SIZES 64" x 43" THRU 84" x 57" AND ROUND PIPE SIZES 42" Ø THRU 84" Ø. GAGES OF CONNECTOR SECTION SHALL BE THE SAME AS THE END SECTIONS AS MENTION ABOVE. THE CONNECTOR SECTION WILL BE ATTACHED TO THE END SECTION BY 0.374 Ø GALVANIZED RIVETS OR BOLTS APPROXIMATELY 6" CENTERS.
- 9. HELICALLY CORRUGATED PIPE, FOR TYPE NO. 5 AND TYPE NO. 3 THE DIMPLE BAND OR CORRUGATED PIPE CONNECTOR SECTION SHALL BE ATTACHED TO THE END SECTION BY 0.374" Ø GALVANIZED STEEL RIVETS OR BOLTS SPACED AT APPROXIMATELY 6" CENTERS.
- 10. TYPE NO. 1, TYPE NO. 2, AND TYPE NO. 3 CONNECTIONS MAY BE USED WITH WELDED SEAMS HELICALLY CORRUGATED PIPE WITH RE-ROLLED ENDS. RE-ROLLED ENDS SHALL INCLUDE A MINIMUM OF TWO (2) ANNULAR CORRUGATIONS OF THE SAME SIZE AS THE PIPE CORRUGATIONS.
- 11. ALL CUT ENDS OF PIPE MUST BE CLEANED AND EITHER REGALVANIZED OR PAINTED AT THE FABRICATION PLANT USING GALVANIZED-ZINC PAINT PER THE MANUFACTURES RECOMENDATIONS WITH MATERIAL CERTIFICATIONS SUBMITTED.



MULTIPLE INSTALLATION SPACING

NOTE: At Roadway Culverts Place End Sections On Inlet End Only. At Driveway Culverts Place End Sections On Both Ends.

REVISED ON 12/12/2013

Rerolled End

UNITED STATES THE INTERIOR OF INDIAN AFFAIRS

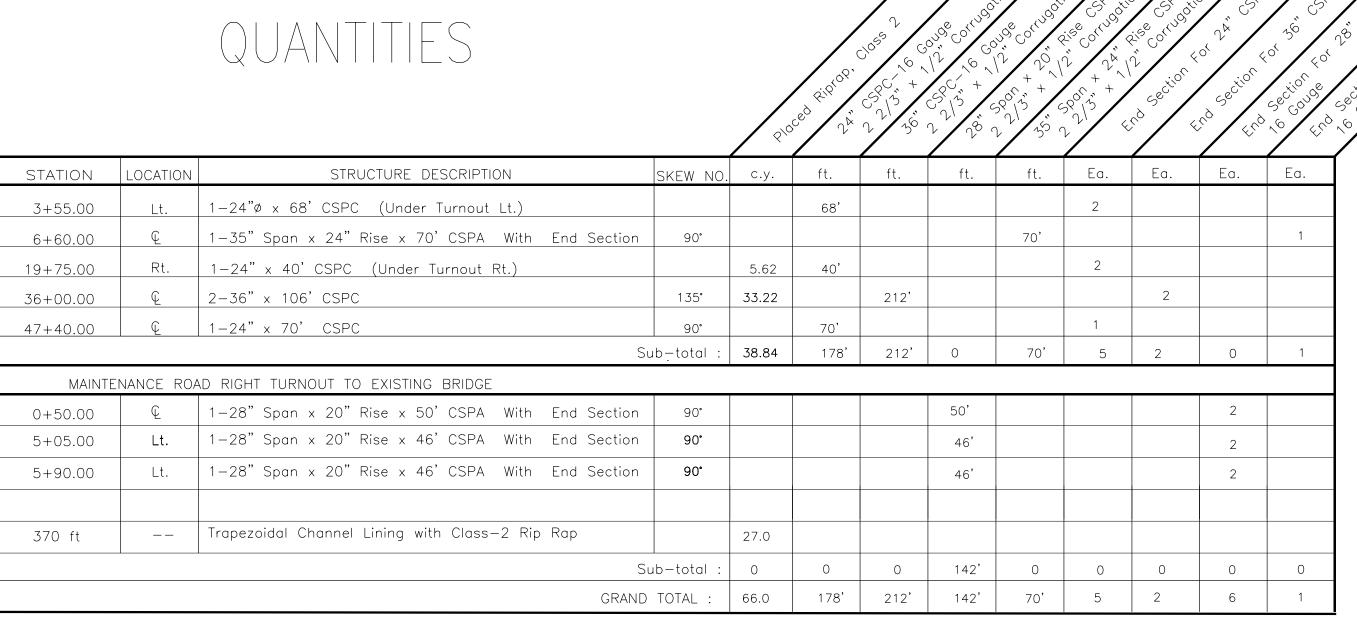
NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

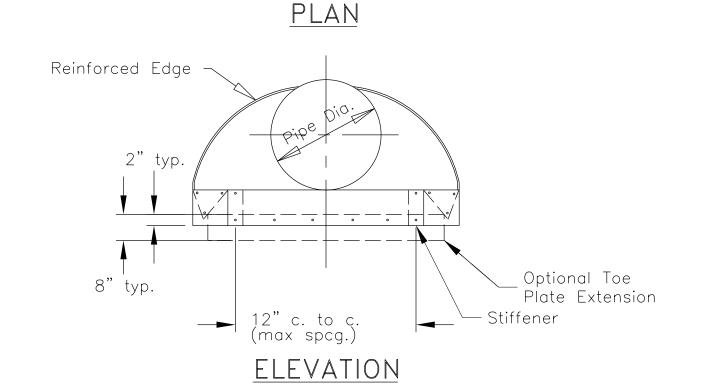
STRUCTURE QUANTITIES

DRAWN BY: Gerald.Hood	DATE:5/7/2009
DESIGNED BY: NRDOT	DATE:5/7/2009
REVISED: 1/28/2013	BY: Peterson.Yazzie
ANNOTATION SCALE: Full	Size 1=1

FILENAME: Sht.19_Drainage Str. Quantity.dgn

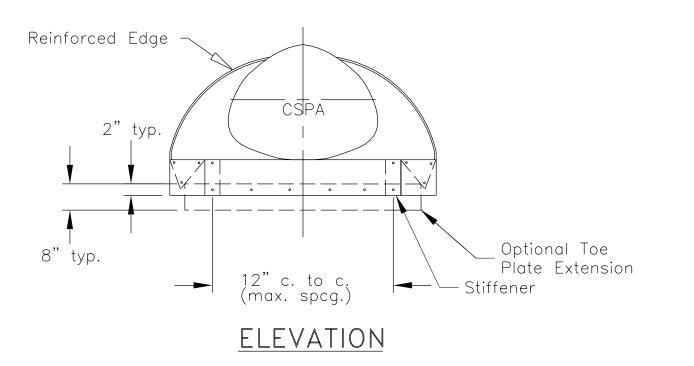


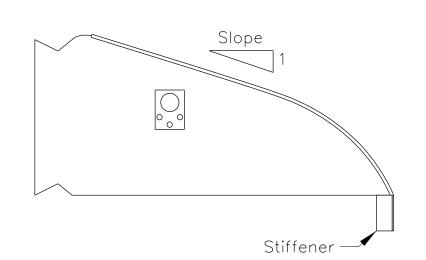




Galvanized

Steel





TYPICAL CROSS SECTION

DIMENSIONS:

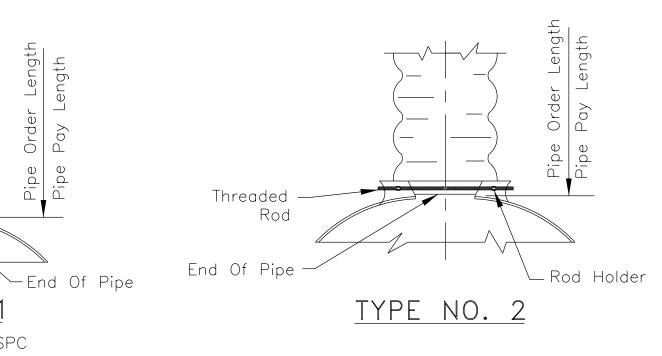
A, B, F, H, & L See Table 2.39, 2.40, And 2.41 In "Handbook Of Steel Drainage & Highway Construction Projects" Latest Editions.

Helical Pipe

TYPE NO. 5

(SEE NOTE NO. 6)

Corrugation Only)



Connector Lug —

TYPE NO. 1

For 1.0' Thru 2.0' CSPC

& 3'-**%**" x 1'-8" CSPA

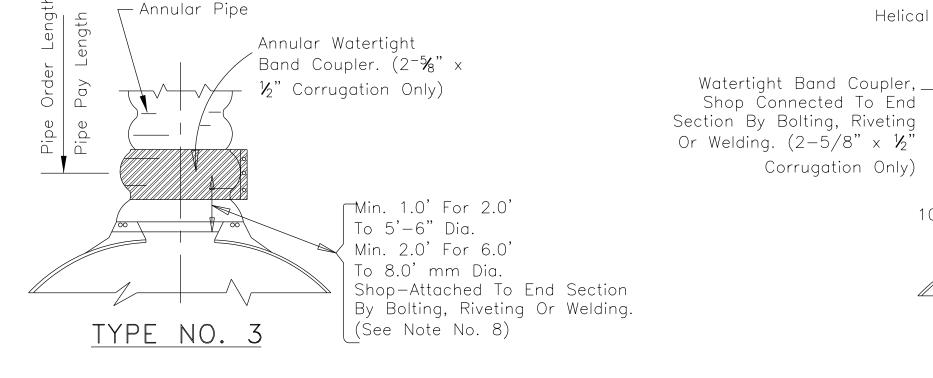
(See Note No. 6)

Threaded Rod

Gauge 1.0"

Wide Flat Strap

Or 1/8"



For 2'-6" & 3.0' CSP And $1'-5" \times 1'-1"$ thru $4'-9" \times 3'-2"$ CSPA Only (See Note No. 7)

REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	OLAVAN	N2007	N2007(1-1)1,2&4	20	63

GENERAL NOTES

- 1. The contractor shall be required to adjust the length of sign support posts. This work shall be included in the unit price for the appropriate bid items shown in the bid schedule.
- 2. Sign dimension equal to or in exceed of 30" x 30" size shall be install with a minimum of two (2) steel posts.

illustration of posts/weight determination:

REQUIRED: Determine Post Requirement For a 5.00 ft. wide x 4.00 ft. high traffic Sign. Located On A Rural Highway. GIVEN: W = 5.00 ft.D = 4.00 ft. C = 6.00 ft, for Rural areas SOLUTION: 1) B = C + (D/2)

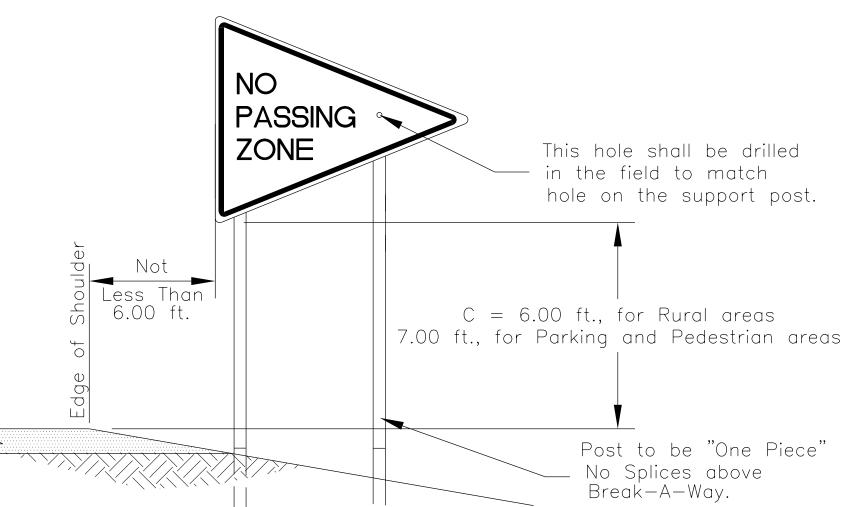
> B = 6.00 ft. + (4 ft/2)B = 8.00 ft.2) A = W X D

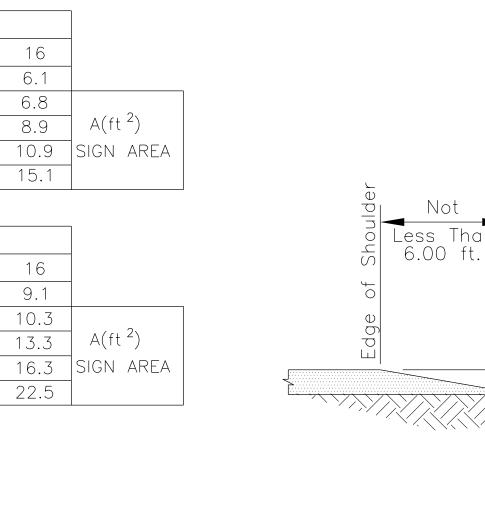
 $A = 5.00 \text{ ft.} \times 4.00 \text{ ft}$ $A = 20 \text{ ft.}^2$ 3) K factor = A X B $K = 20 \times 8$ K = 160

4. Begin with single post chart for column of B=8.00 ft. and continue down until area of sign equal or exceed 20 sq./ft. or down the K factor column until the value equaals or exceeds 160. Both the Area and K factor exceed the single post chart so go to the double post chart.

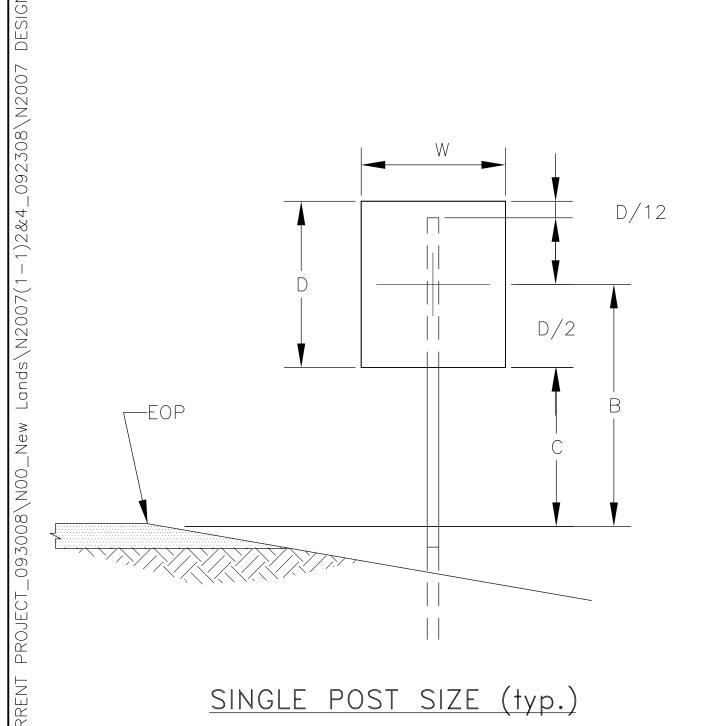
Select two (2) Posts of 3.00 lb/ft. Yields a factor of 174 Which Is Optimum.

EXAMPLE:	K factor	Post weight	B=8	
160	142	2.75 lb/ft	17.8	20
	174	3.00 lb/ft	21.8	
	241	4.00 lb/ft	30.1	





TYPICAL ROADSIDE SIGN LOCATION



K FACTOR

 $(B \times A)$

K FACTOR

 $(B \times A)$

97.00

109.00

142.00

174.00

241.00

K FACTOR

 $(B \times A)$

145.00

164.00

213.00

361.00

3.9

5.1

6.7

7.3

19.4 16.1

21.9 | 18.2

28.4 23.7

34.8 29.0

42.6 35.5

72.2 60.1

261.00 52.2 43.5

3.9

7.3

3.9

3.9

8

24.9 21.8 19.3

5.1 5.1

6.7 6.7

7.3 7.3

9.4 9.4

13.8 | 12.1

15.6 13.7

20.3 | 17.8

23.4 20.5

30.4 26.6

51.5 45.1

29.0 | 24.2 | 20.7 | 18.1 |

POST

SIZE

2.00 lb/ft

2.25 lb/ft

2.75 lb/ft

3.00 lb/ft

4.00 lb/ft

SIZE

2.00 lb/ft

2.25 lb/ft

2.75 lb/ft

3.00 lb/ft

4.00 lb/ft

SIZE

2.00 lb/ft

2.25 lb/ft

2.75 lb/ft

3.00 lb/ft

4.00 lb/ft

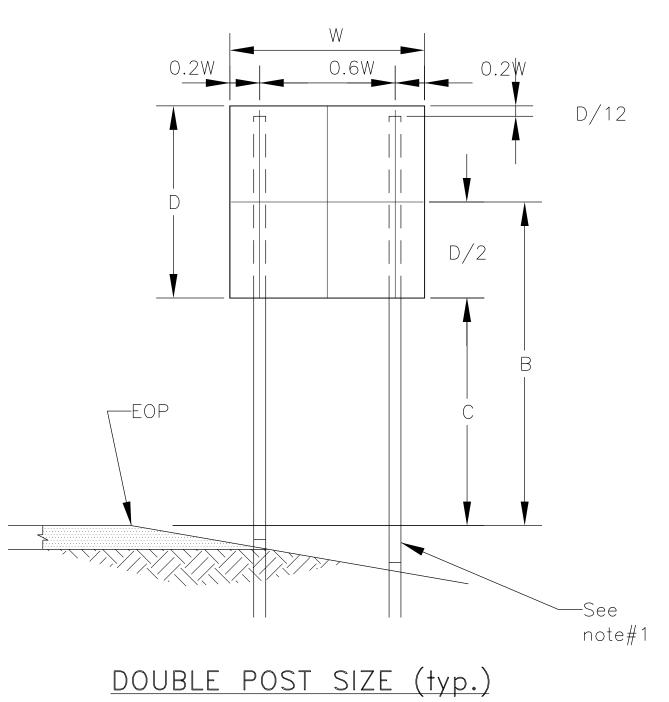


CHART TO DETERMINE SINGLE POST SIZE

10

3.9 3.9

5.1 4.6

6.7 6.0

7.3 7.3

9.4 9.4

10.9 9.9

17.4 15.8

21.3 | 19.3 |

11

8.8

14.2 | 12.9 | 11.8

11

37.3 32.6 29.0 26.1 23.7 21.7 20.0 18.6 17.4

CHART TO DETERMINE DOUBLE POST SIZE

10

9.7

CHART TO DETERMINE THREE POST SIZE

10

12

5.5

6.7

12

8.1

14.5

12

17.7

40.1 36.1 32.8 30.0 27.7 25.8 24.0

3.7

4.2 3.9 3.6

5.1

9.4 8.6 8.0

13

7.5

10.9

13

14.5 | 13.2 | 12.1 | 11.2 | 10.4 | 9.7 | 9.1

16.4 | 14.9 | 13.7 | 12.6 | 11.7 | 10.9 |

9.1 8.4

6.2 5.6

3.4 3.2 2.9

4.7

6.9

7.8

10.1

13.4 | 12.4 | 11.6

16.3 15.2 14.2

3.4

4.4

5.1

6.5

9.5

7.3 6.8

2.8

3.2

7.0

6.1

8.9

10.3

4.1

5.0 SIGN AREA

B DIMENSION (Feet)

3.9

5.1

6.7

7.3

9.4

10.8

12.2

15.8

16.1

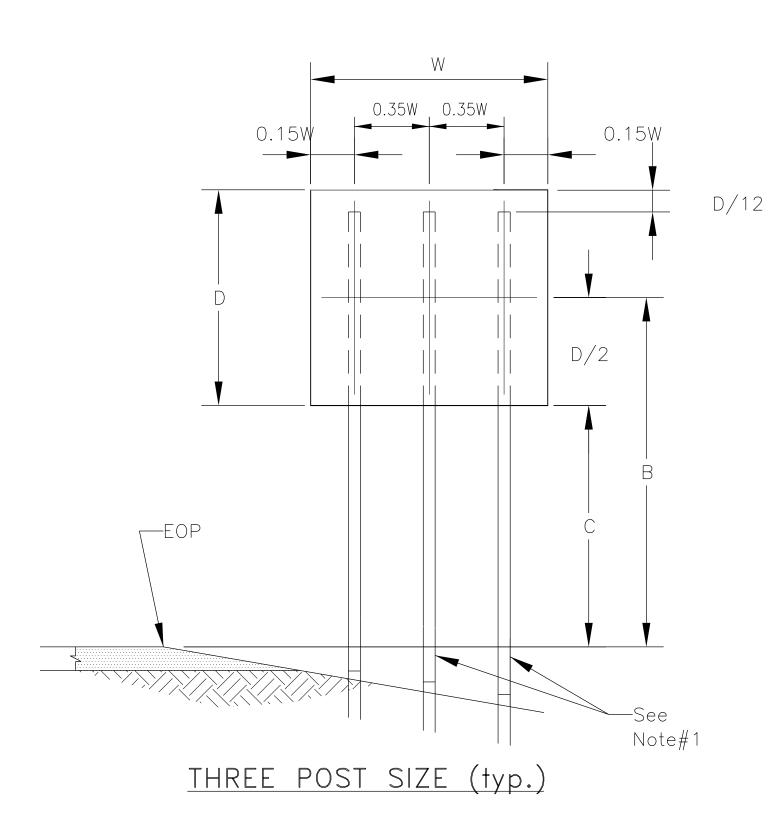
18.2

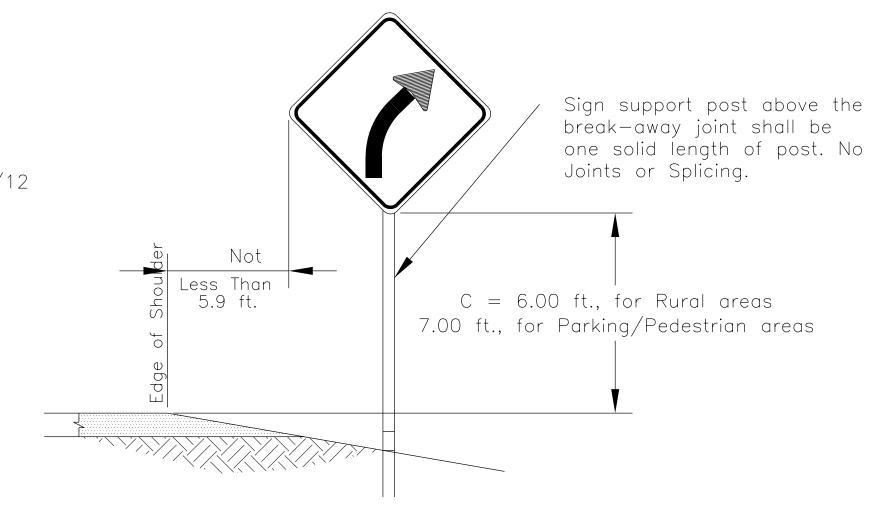
23.6

B DIMENSION (Feet)

48.2 40.2 34.4 30.1 26.8 24.1 21.9 20.1 18.5 17.2

B DIMENSION (Feet)





TYPICAL ROADSIDE SIGN LOCATION

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

PERMANENT SIGNING DETAIL

DRAWN BY: Gerald.Hood	DATE: 5/7/2009
DESIGNED BY: NRDOT	DATE: 5/7/2009
REVISED: 1/25/2013	BY: Peterson.Yazzie
ANNOTATION SCALE: Full	Size 1=1

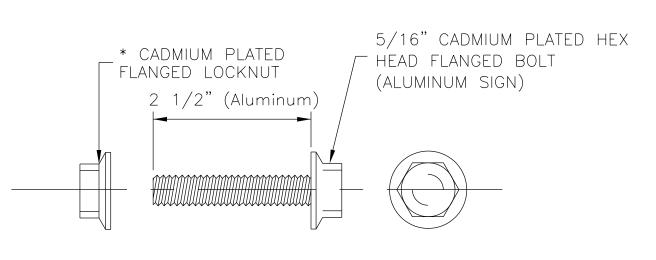
FILENAME: Sht.20 Perm Sign Std Details1.dgr



RIB-BAK U-CHANNEL SIGN SUPPORTS

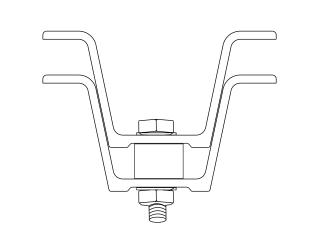
WEIGHT	DIME	ENSION	S (inc	hes)	AREA	X-X	AXIS	Y-Y	AXIS
*Ib/ft	А	В	С	D	in ²	in ⁴	in 3	in ⁴	in 3
2.0	1.462	3.062	1.278	.669	.556	.155	.195	.422	.276
2.5	1.516	3.062	1.278	.669	.701	.208	.249	.550	.359
2.75	1.536	3.062	1.278	.669	.756	.228	.270	.598	.391
3.0	1.881	3.500	1.336	.834	.840	.376	.340	.837	.478
4.0	1.968	3.500	1.336	.834	1.112	.540	.467	1.147	.655

* ±5%

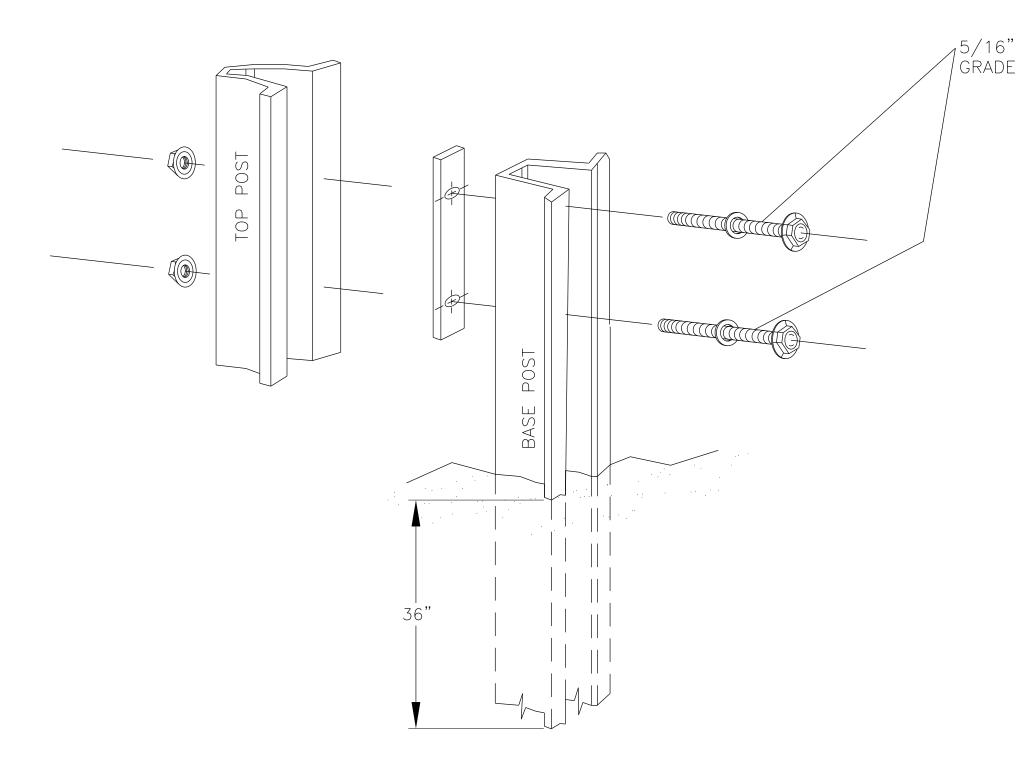


BOLTS AND LOCKNUT - SIGN ATTACHMENT

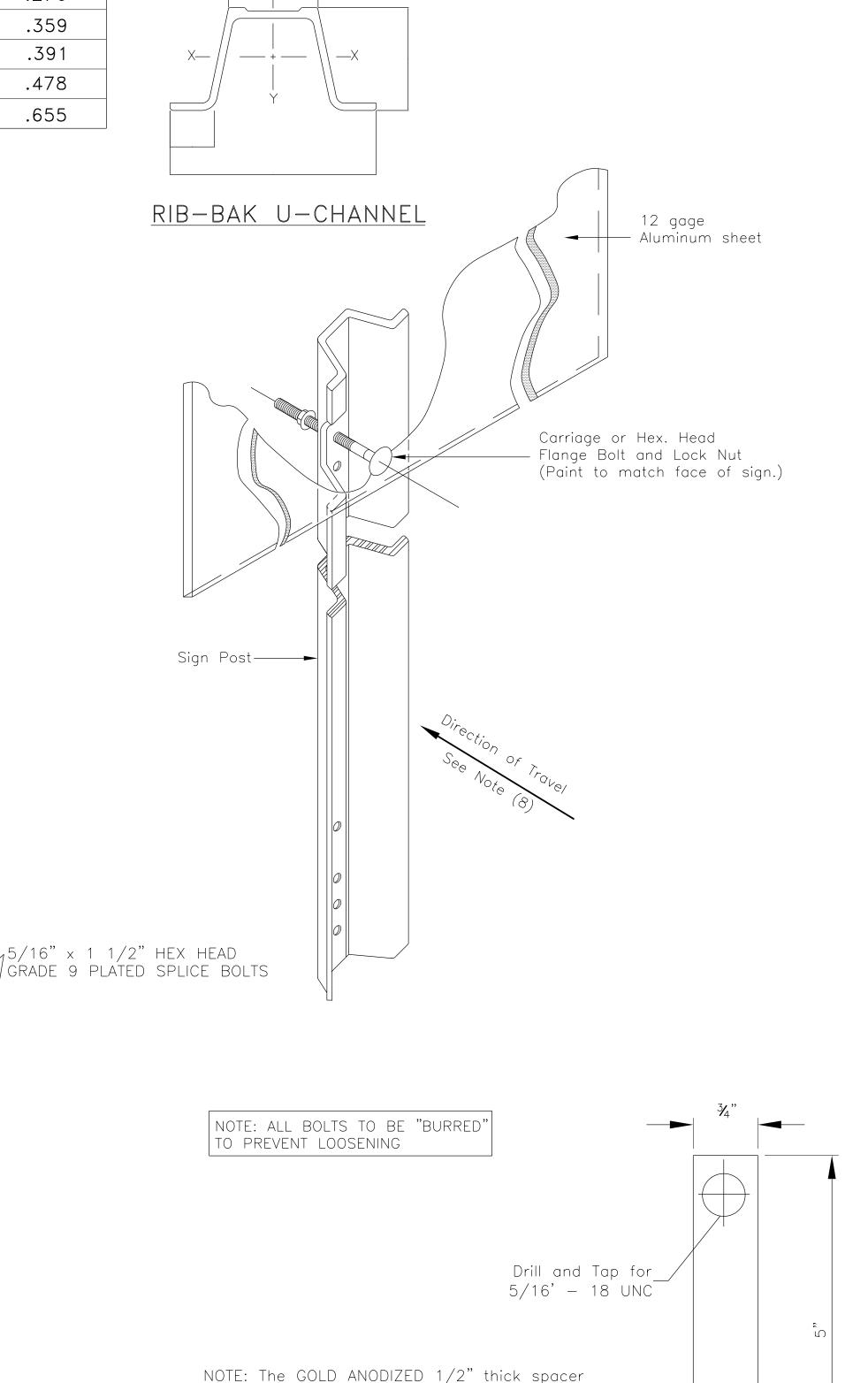
* FLANGED LOCKNUT REQUIRED FOR CARRIAGE AND HEX.



LAP SPLICE - TOP VIEW



LAP SPLICE CONNECTION DETAIL



bar is to be used with 3.00 lb/ft & 4.00 lb/ft

spacer bar is to be used with 2.00 lb/ft, 2.50

LAP SPLICE SPACER BAR

posts only. The SILVER ANODIZED 3/8" thick

lb/ft, and 2.75 lb/ft posts only.

REGION STATE RESERVATION ROUTE PROJECT NO. SHEET TOTAL SHEETS

NAVAJO ARIZONA NAVAJO N2007 N2007(1-1)1,2&4 21 63

GENERAL NOTES

- 1. BASE POST AND SIGN POST SHALL BE RIB-BAK U-CHANNEL FABRICATED FROM HOT ROLLED CARBON STEEL BARS CONFORMING TO THE REQUIREMENTS OF ASTM A499. YIELD POINT OF THE STEEL SHALL BE 550 MPa (MINIMUM) TENSILE SHALL BE 689.47 MPa (MINIMUM).
- 2. POSTS SHALL BE A UNIFORM, MODIFIED, FLANGED CHANNEL SECTION OF THE RIB-BAK DESIGN. WEIGHT OF THE POSTS SHALL BE AS SPECIFIED BY THE USER, ±5% BEFORE PUNCHING. THE POSTS SHALL BE PUNCHED WITH WITH CONTINUOUS 3/8" HOLES ON 1" CENTERS FOR THE ENTIRE LENGTH OF THE POST.
- 3. THE POSTS SHALL BE MACHINE STRAIGHTENED TO HAVE A SMOOTH UNIFORM FINISH, FREE FROM DEFECTS AFFECTING THEIR STRENGTH, DURABILITY, OR APPEARANCE. ALL HOLES AND ROUGH EDGES SHALL BE FREE FROM BURRS. THE PERMISSIBLE TOLERANCE FOR STRAIGHTNESS SHALL BE WITHIN 1/4" IN 5 FEET
- 4. POSTS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A 123. BOLTS, NUTS, WASHERS AND SPACER SHALL BE CADMIUM PLATED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A 165 OR ZINC PLATED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM B 633.
- 5. SPLICE HARDWARE SHALL CONSIST OF TWO FULLY THREADED, 5/16" x 1 1/2" GRADE 9 PLATED, HEX HEAD BOLTS, WITH FLAT WASHERS, AND SELF LOCKING HEX NUTS PER POST. IN ADDITION, ONE 3/4" x 5" PLATED SPACER BAR SHALL BE USED, PER POST, TO STIFFEN THE SPLICE CONNECTION. EACH SPACER BAR SHALL BE DRILLED AND TAPPED WITH 5/16"-18 UNC THREADS. THE SPACER SHALL BE FABRICATED FROM HOT ROLLED CARBON STEEL BARS CONFORMING TO ASTM A 36 OR M 1020. BOLTS SHALL BE RED IN COLOR, WITH THE HEAD MARKING "M180".
- 6. BOLTS AND LOCK NUT HARDWARE FOR SIGN ATTACHMENT SHALL BE HEX HEAD FLANGE TYPE, SIZE SHALL BE 5/16"-18 UNC.
- 7. AN APPROVED ALTERNATE BREAKAWAY POST ASSEMBLY MAY BE SUBMITTED TO THE C.O.R. FOR REVIEW AND APPROVAL.
- 8. SUPPLEMENTAL SIGNS ON THE OPPOSITE SIDE OF ROAD SHALL HAVE THE POST REVERSED SO THAT RIB-BAK IS FACING AWAY FROM THE OPPOSING TRAFFIC.
- 9. THE POST SHALL BE COATED WITH A BAKED ON GREEN ALKYD RESIN, PAINT, PAINTED WITH A POLYESTER POWDER OR GALVANIZED PER NOTE 4 ABOVE. POWDER COATING SHALL CONFORM TO AASHTO M 284-08.

INSTALLATION PROCEDURE

- STEP 1:

 DRIVE BASE POST TO WITHIN APPROXIMATELY ONE FEET ABOVE GROUND LEVEL. PLACE ONE BOLT AND CUT WASHER IN FIFTH HOLE FROM THE TOP, AND SECURELY TIGHTEN THREADED SPACER ONTO BOLT.
- STEP 2:

 DRIVE BASE POST TO 4" ABOVE GROUND LEVEL. PLACE REMAINING
 BOLT AND CUT WASHER IN FIRST HOLE FROM THE END, AND SECURELY
 TIGHTEN THREADED SPACER ONTO BOLT.
- STEP 3:

 DIG OUT APPROXIMATELY 2" FROM AROUND BACK OF GROUND POST TO ALLOW ROOM FOR TOP POST TO BE ATTACHED.
- STEP 4:

 NEST TOP POST ONTO PROTRUDING BASE POST BOLTS, THROUGH THE FIRST AND FIFTH HOLES OF THE TOP POST.
- STEP 5:

 PLACE A SELF-LOCKING FLANGE NUT ON EACH BOLT. TIGHTEN NUTS AND TAMP EARTH AROUND POST FIRMLY.

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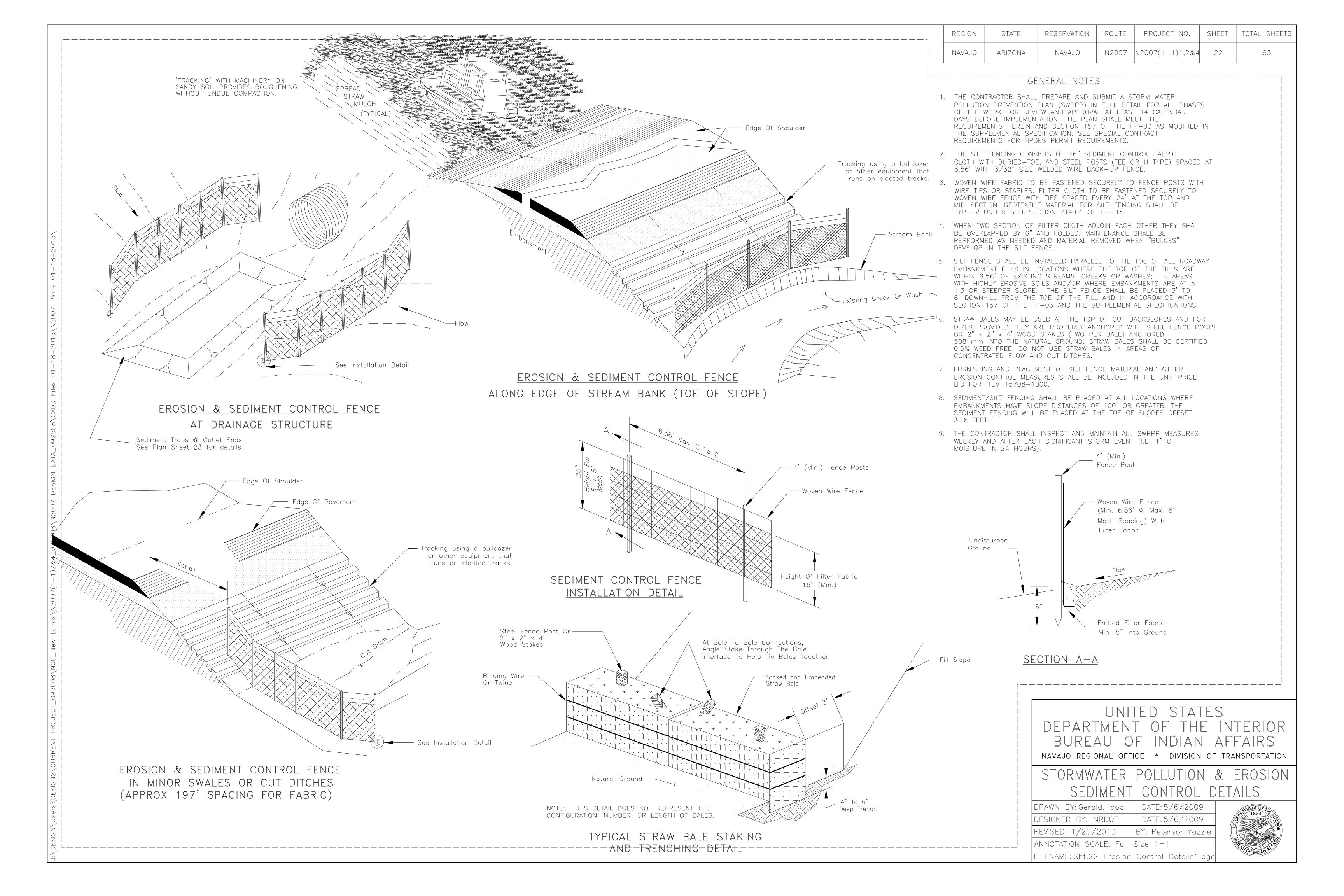
NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

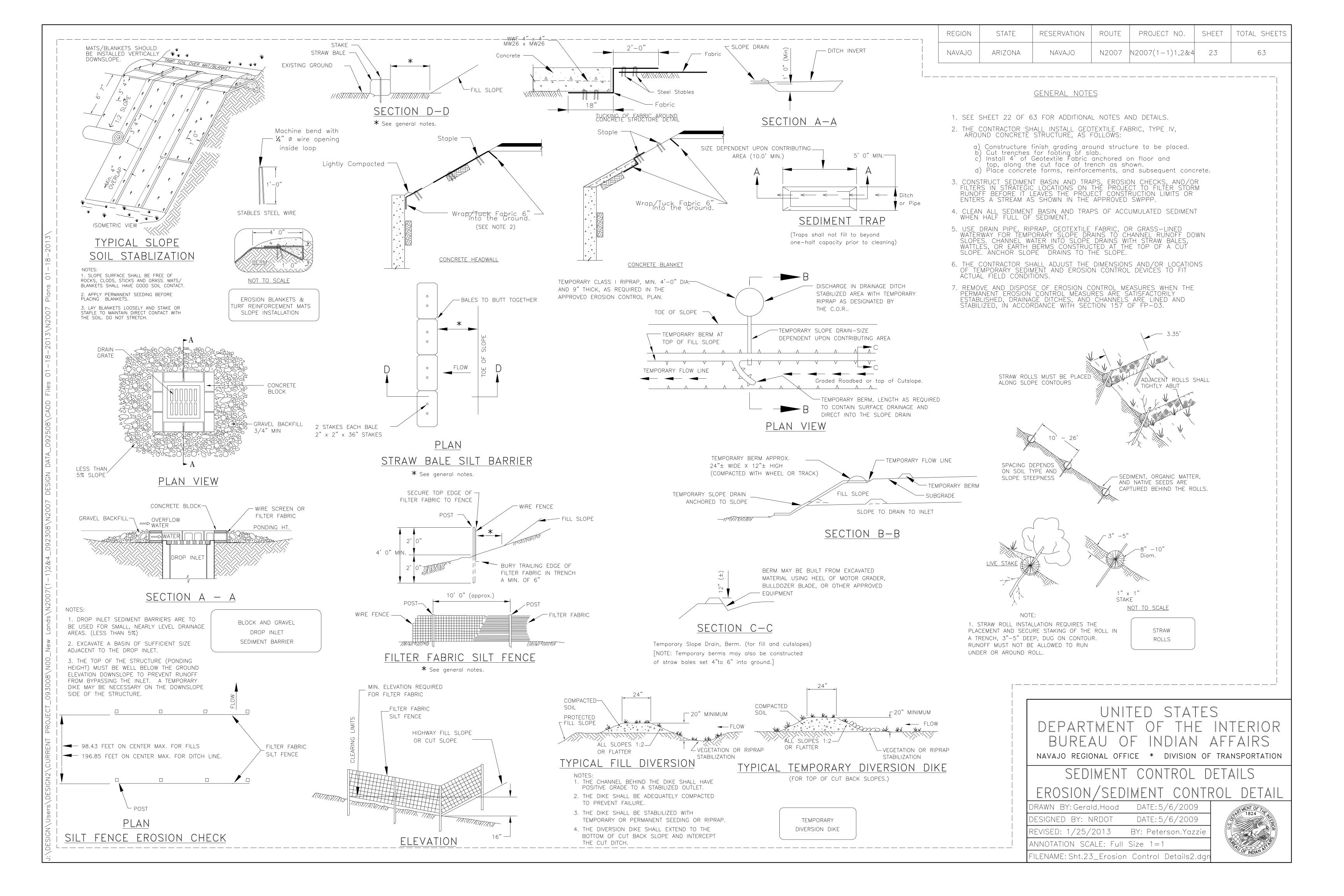
LAP SPLICE U-CHANNEL BREAKAWAY SYSTEM

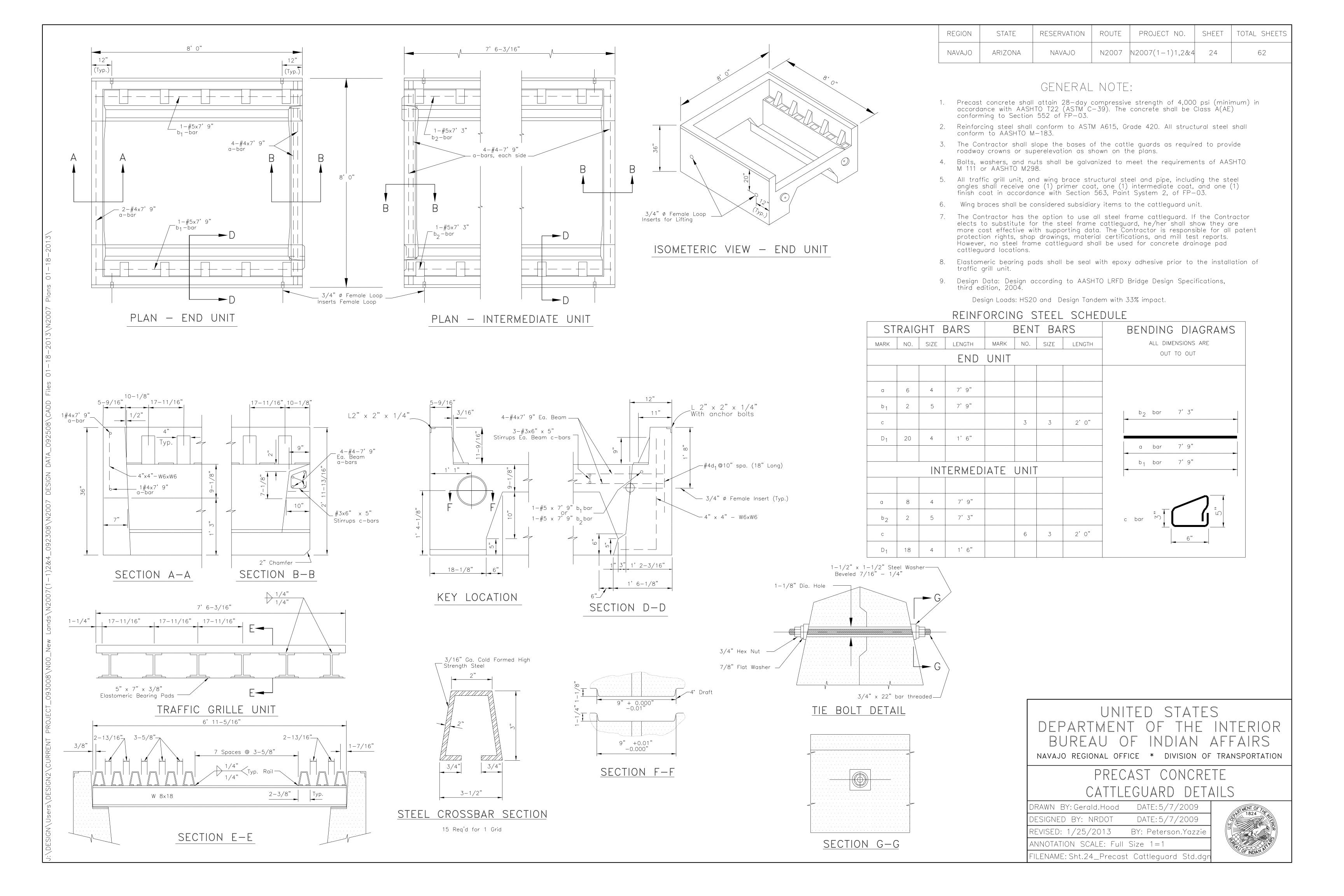
DRAWN BY: Gerald.Hood	DATE: 5/7/2009
DESIGNED BY: NRDOT	DATE: 5/7/2009
REVISED: 1/25/2013	BY: Peterson.Yazzie
ANNOTATION SCALE: Full	Size 1=1

ANNOTATION SCALE: Full Size 1=1 FILENAME: Sht.21 Perm Sign Std Details2.dgn









REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	OLAVAN	N2007	N2007(1-1)1,2&4	25	63
			GENE	RAL NOTES		
1.	Structural pip structural stee	oe shall conform el shall conform to	to ASTM ASTM-A36	A53-93a, Grade E 5.	3. All othe	r

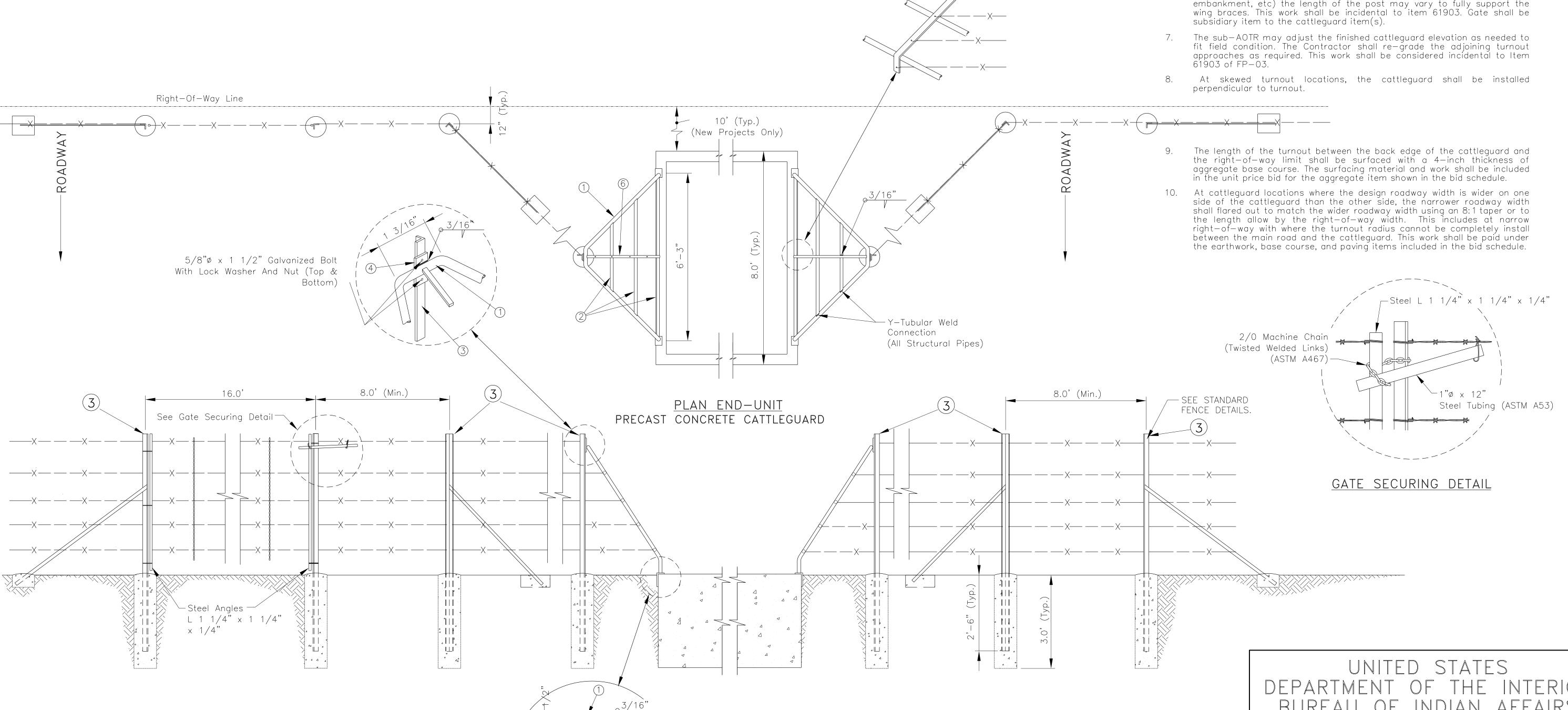
ESTIMATED MATERIAL LIST

PART NO.	MATERIAL	SIZE AND THICKNESS	LENGTH	QUANTITY
1	Structural Steel Pipe	2 1/2" Ø Nominal	14'-2"	2
2	Structural Steel Pipe	1 1/4" Ø Nominal	13'-9"	2
3	Steel Angle (See Note 4 & 6)	L 2 1/2" x 2 1/2" x 3/8"	7' 6-3/16"	2
4	Steel Plate	3 1/2" × 3/8"	7"	2
5	Steel Angle	L 6" × 3 1/2" × 3/8"	3 1/2"	4
6	Bar	1" × 1/4"	5' 6-1/8"	2
	Bolts, Nuts and Washers	As Shown		

3/3"ø x 4" Galvanized Bolt with Lock Washer And Threaded Ring — Wedge Cinch Anchor in Drilled

Holed or Approved Equivalent.

- 2. Bolts, nuts, and washers shall be galvanized in accordance with AASHTO
- 3. All wing brace structural steel and pipe shall receive one (1) primer coat, one (1) intermediate coat, and one (1) finish coat in accordance with Section 563, Paint System 2 of FP-03.
- 4. All structural pipe joints shall be fabricated in accordance with AISC Manual of Steel Construction, Latest Edition.
- Welding design details shall conform to the AASHTO Standard Specifications for welding at Structural Steel Highway Bridges, Latest
- 6. The supporting wing brace posts length (part 3) shall be 7'-6" (minimum). Under certain conditions (such as drain through cattleguard, high embankment, etc) the length of the post may vary to fully support the wing braces. This work shall be incidental to item 61903. Gate shall be



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CATTLEGUARD WING-BRACE DETAILS

DATE: 5/6/2009 DRAWN BY: Gerald. Hood DESIGNED BY: NRDOT DATE: 5/6/2009 REVISED: 1/25/2013BY: Peterson.Yazzie ANNOTATION SCALE: Full Size 1=1

FILENAME: Sht.25_C-GuardWingBraceDetails.dg



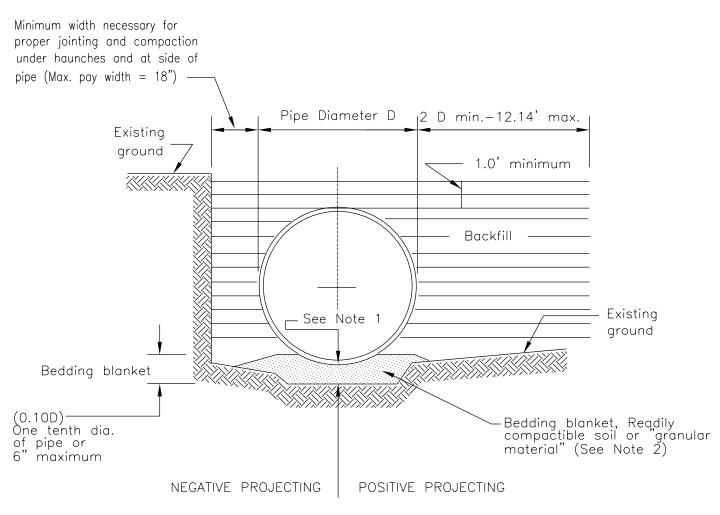


FIG. A. CLASS C BEDDING

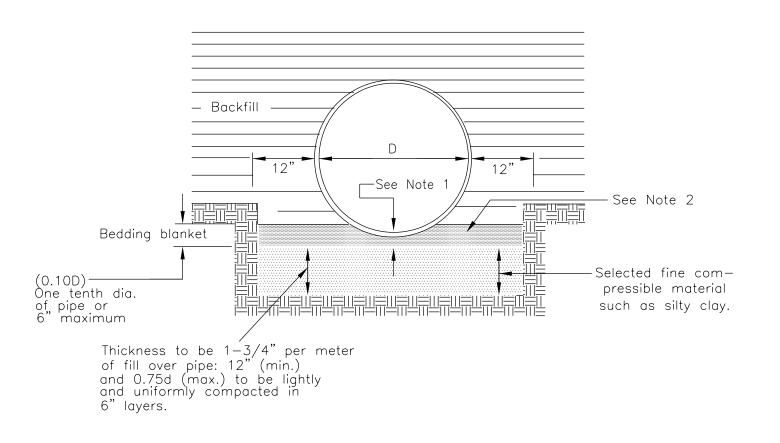


FIG. B ROCK BEDDING

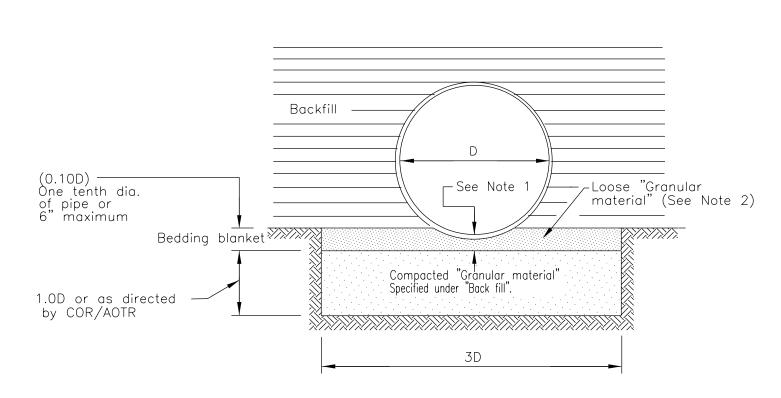
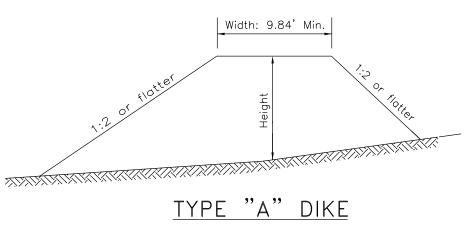


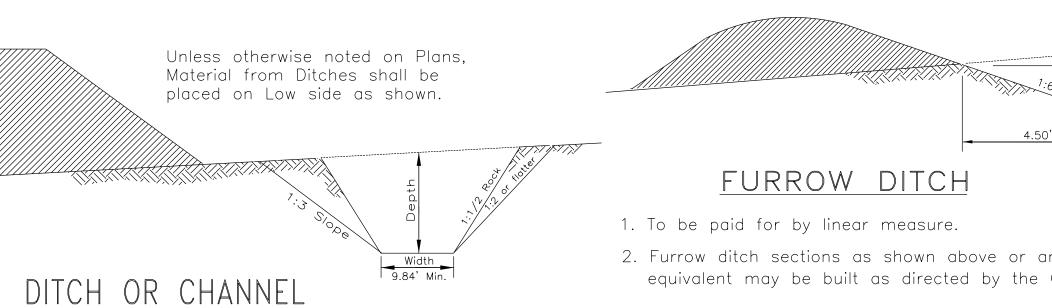
FIG. C. FOUNDATION STABILIZATION BEDDING



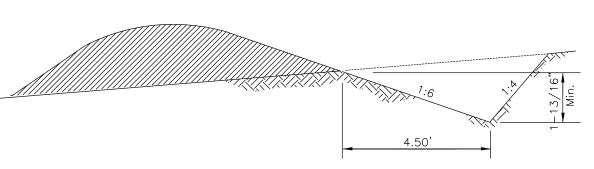
Width: 9.84' Min.

TYPE "B" DIKE

NOTE: When necessary the slope may be flattened to 1:6 with bit. mix or soil cement protection as called for on plans.

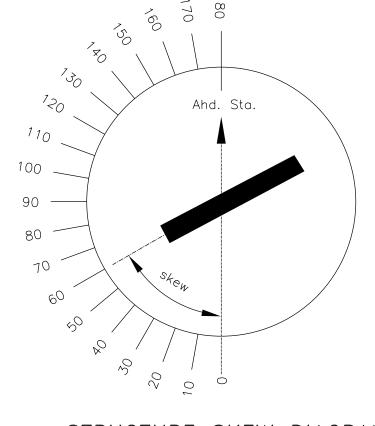


Note: DIMENSIONS OF DITCHES AND DIKES as shown on plans are respectively width, depth or height and length.

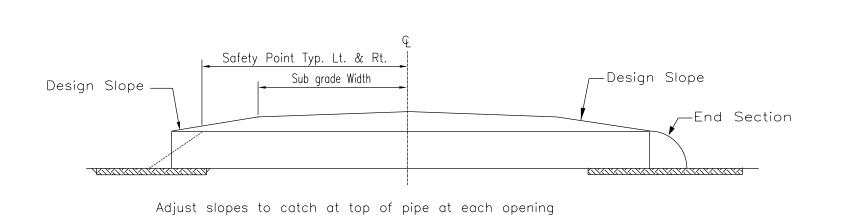


2. Furrow ditch sections as shown above or an approved equivalent may be built as directed by the C.O.R./AOTR

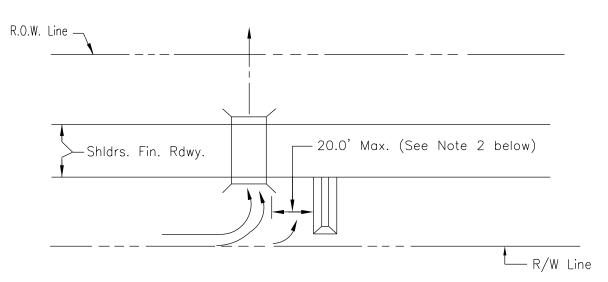
DITCH BLOCK DETAILS



STRUCTURE SKEW DIAGRAM



TYPICAL PIPE INSTALLATION



TYPICAL DITCH BLOCK INSTALLATION AT STRUCTURE

- 1. Ditch Block at structures to be so placed that they create a water cushion. Elevation top of Ditch Block shall be 6" above elevation of top of pipe unless otherwise shown.
- 2. Ditch Block shall be located a distance equal to the largest dimension of Box Culvert or pipe from the face of the drainage structure. In no case shall the distance exceed 20.0'.

GENERAL NOTES

SUPPLEMENTAL SPECIFICATION FOR ADDITIONAL NOTES.

- PLACE LOOSE BEDDING ROUGHLY SHAPED TO BOTTOM OF PIPE, THEN
- COMPACTED UNDER HAUNCHES AFTER PIPE PLACEMENT. SEE SECTION 204, 209, 602, AND 704 OF FP-2003, INCLUDING THE
- ALL DRAINAGE STRUCTURE MATERIAL SHALL BE UNLOADED AND HANDLED WITH REASONABLE CARE. NO STRUCTURE SHALL BE DRAGGED OR ALLOWED TO STRIKE ANY HARD SURFACE DURING PLACEMENT. ANY DAMAGED STRUCTURE SHALL BE REPAIRED OR REPLACED, BY THE CONTRACTOR, AT NO ADDITIONAL COST TO THE GOVERNMENT.
- ALL STRUCTURAL PLATE PIPE STRUCTURES SHALL BE ASSEMBLED AND INSTALLED IN ACCORDANCE WITH THE FABRICATOR'S RECOMMENDATION.
- BACKFILL MATERIAL SHALL BE PLACED 12-INCH (MIN.) TO 40-INCH (MAX.) PIPE DIAMETER WIDTH ON THE SIDES AND 12-INCH OVER THE PIPE. BACKFILL MATERIAL BEYOND THESE LIMITS SHALL BE REGULAR EARTHWORK EMBANKMENT MATERIAL. THE BACKFILL MATERIAL SHALL BE APPROVED BY THE AOTR/COR PRIOR TO IT'S USE AND SHALL BE PLACED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.
- PONDING OR JETTING PIPE BACKFILL SHALL NOT BE PERMITTED.
- ALL PIPE EXCAVATION, BACKFILLING, DE-WATERING, PUMPING OR COFFERDAMS REQUIRÉD TO PROPERLY INSTALL THÉ DRAINAGE PIPE SHALL BE CONSIDERED INCIDENTAL TO COMPLETION OF THE PROJECT AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- MULTIPLE PIPE INSTALLATIONS SHALL BE PLACED 24-INCH BETWEEN END SECTIONS UNLESS OTHERWISE DIRECTED BY THE AOTR/COR OR AS SHOWN ON THE PLANS.
- ALL PIPES SHALL BE PROTECTED BY A COVER OF NOT LESS THAN 36-INCH OF EMBANKMENT ABOVE PIPE BEFORE ANY HEAVY EQUIPMENT IS ALLOWED TO PASS OVER THE STRUCTURE(S) DURING CONSTRUCTION.
- 10. ALL DRAINAGE STRUCTURES SHALL BE INSTALLED AT THE ORIGINAL GROUND LINE AND SLOPE TO ASSURE POSITIVE DRAINAGE UP TO THE R.O.W. LIMITS. IN NO CASE SHALL THE PIPE(S) BE PLACED BELOW THE ORIGINAL GROUND ELEVATIONS. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO COMPLETION OF PROJECT AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- 11. AT DRAINAGE PIPE REPLACEMENTS, INSTALLATIONS, EXTENSIONS, AND IN-PLACE PIPE CLEANING LOCATIONS, THE CONTRACTOR SHALL RESHAPE, REGRADE AND CLEAN THE INLET AND OUTLET CHANNELS TO THE RIGHT-OF-WAY LINE AND/OR EXISTING DRAINAGE CHANNEL, TO PRODUCE SMOOTH FLOWS AT CULVERT INTAKES AND DISCHARGES AS DIRECTED BY THE AOTR/COR. THIS WORK SHALL BE INCIDENTAL TO BID ITEMS UNDER SECTIONS 602, 603, AND 607.
- 12. ALL CULVERTS UNDER TURNOUTS AND DRIVEWAYS SHALL BE PLACED AT THE PROPOSED DITCH FLOWLINE. THE CONTRACTOR SHALL BE REQUIRED TO FIELD ADJUST THE PROFILE GRADES OVER PIPE AS DIRECTED BY THE AOTR/COR TO PROVIDE FOR THE MINIMUM COVER.
- 13. TYPE "B" DIKE SHALL BE USED ON THIS PROJECT UNLESS OTHERWISE NOTED ON THE PLANS. EMBANKMENT MATERIAL NEEDED TO BUILD EARTHEN DIKES SHALL BE CONSIDERED INCIDENTAL TO ITEM 20443-2000.
- 14. IF DIRECTED BY THE AOTR/COR TO BETTER FIT FIELD CONDITIONS, TO MORE SMOOTHLY DIRECT THE FLOW INTO THE PIPE AND/OR LESSEN THE WATER'S IMPACT ON THE FACE OF THE DITCH BLOCKS, THE DITCH BLOCK TO BE <u>CURVED</u>. THIS WORK TO BE INCIDENTAL TO BID ITEM 20443-2000.

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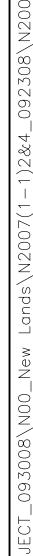
NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

STANDARD PIPE INSTALLATION DITCH DETAILS

DATE: 5/7/2009 DRAWN BY: Gerald. Hood DESIGNED BY: NRDOT DATE: 5/7/2009 REVISED: 1/25/2013 BY: Peterson.Yazzie

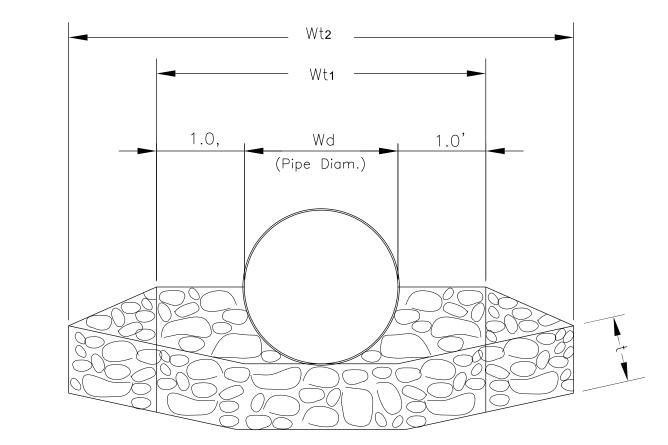
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SECTION A-A - DOUBLE BARREL

<u>Distance Between Pipe's</u> X = 72" For 24" Ø CSPC X = 82" For 30" Ø CSPC X = 93" For 36" Ø CSPC



SECTION A-A - SINGLE BARREL

GENERAL NOTES

STATE

ARIZONA

REGION

OLAVAIO

1. WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE STANDARD SPECIFICATION FOR CONSTRUCTION OF ROADS AND BRIDGES ON FEDERAL HIGHWAY PROJECTS [FP-03].

OLAVAN

RESERVATION ROUTE PROJECT NO. SHEET TOTAL SHEETS

63

 $N2007 \ N2007(1-1)1,2&4 27$

2. THE QUANTITIES SHOWN ARE ONLY AN ESTIMATE. ACTUAL QUANTITIES SHALL BE DETERMINED IN THE FIELD. THE CONTRACTOR WILL BE REQUIRED TO MAKE ANY NECESSARY ADJUSTMENTS IN THE FIELD TO MATCH EXISTING FIELD CONDITIONS. THESE FIELD ADJUSTMENTS ARE THE OBLIGATIONS OF THE CONTRACTOR.

3. EXCAVATION FOR RIPRAP BELOW FLOW LINE AS SHOWN SHALL BE CONSIDERED INCIDENTAL TO ITEM 25101-2000.

4. STONE SIZE SHALL CONFORM TO FP-03, TABLE 705-1, CLASS 2.

5. FILTER FABRIC TYPE IV-B SHALL BE INSTALLED UNDER ALL RIP RAP AND SHALL CONFORM TO SECTION 251, FP-03, AND SHALL BE CONSIDERED INCIDENTAL TO ITEM 25101-2000.

6. THE TOP OF THE RIPRAP SHALL BE INSTALLED TO MATCH THE EXISTING OR FINISHED GROUND ELEVATIONS.

7. FILTER FABRIC SHALL BE TUCKED OR EMBEDDED 16" INTO EMBANKMENT ALONG ALL EDGES AS SHOWN. FILTER FABRIC IS NOT REQUIRED UNDER GROUTED RIPRAP.

8. RIPRAP DOWN DRAIN SHALL BE CARRIED DOWN SLOPE TO INTERSECTION OF FILL SLOPE AND EXTENDED UNTIL A 2% OR LESS GRADE IS ACHIEVED BEFORE TERMINATION.

9. FOR GROUTED RIPRAP, FILL ALL ROCK VOIDS WITH GROUT AS FP-03 SECTION 251. LEAVE 0.5 TO .25 OF THE RIP RAP SURFACE ROCK EXPOSED.

10. THE QUANTITIES SHOWN ARE ONLY AN ESTIMATE. ACTUAL QUANTITIES SHALL BE DETERMINED IN THE FIELD. THE PROJECT MANAGER, AOTR/COR, AND CONTRACTOR SHALL REVIEW ALL ROCK CUT AREAS AFTER THE CONSTRUCTION OF DITCHES, DOWN DRAINS, AND RIPRAP BASINS HAVE BEEN "ROUGH IN". IF IN THE OPINION OF THE AOTR/COR, THAT THE ROCK CUT IS STABLE, THE AOTR/COR MAY ELECT TO DELETE SECTIONS OF THE RIP RAP PROTECTION.

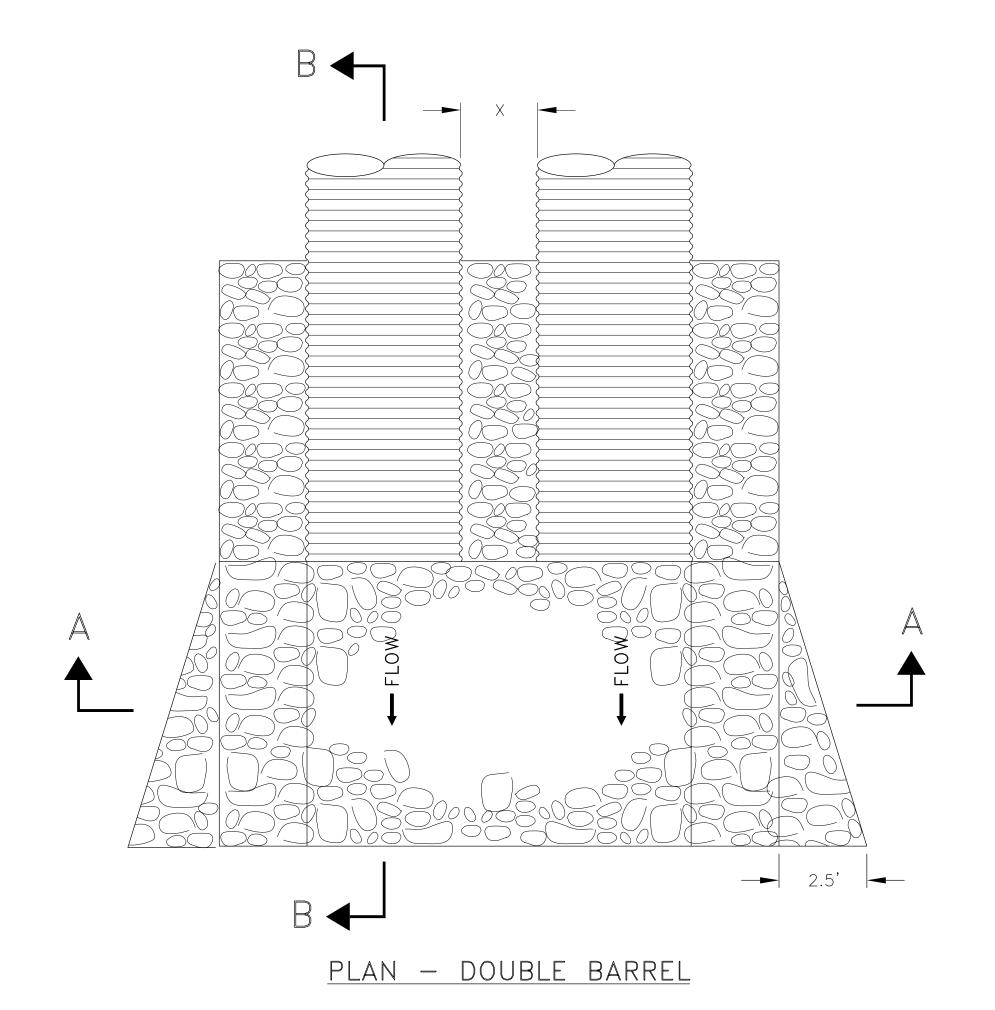
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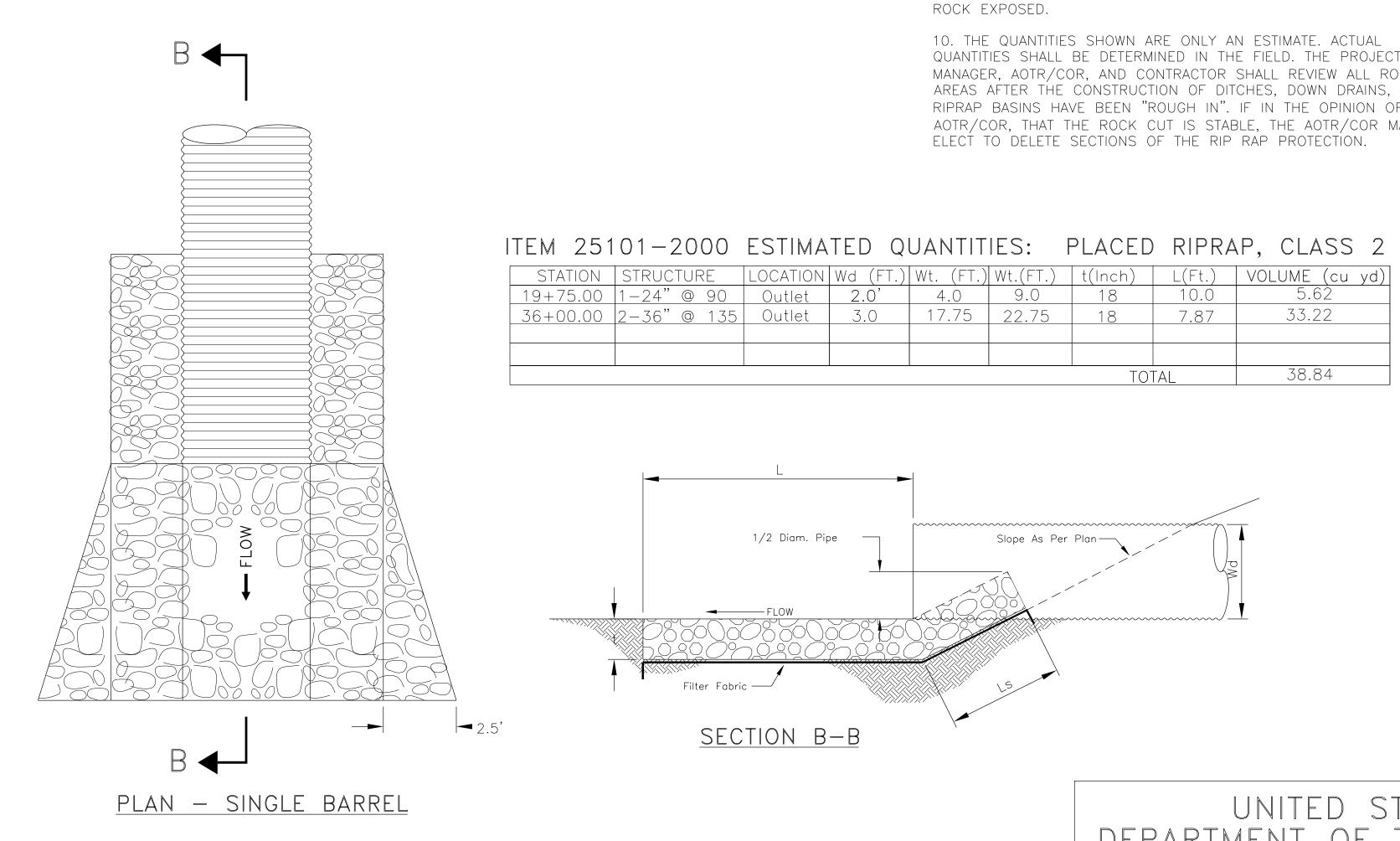
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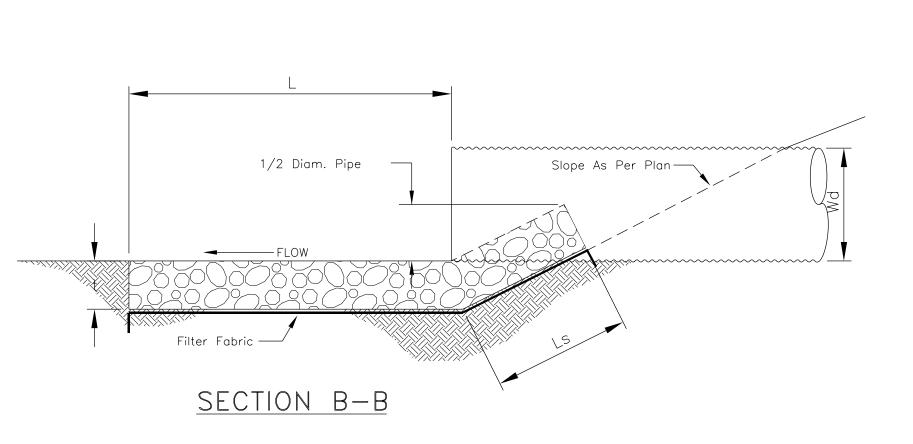
TOTAL

33.22

38.84







3.0

9.0

22.75

17.75

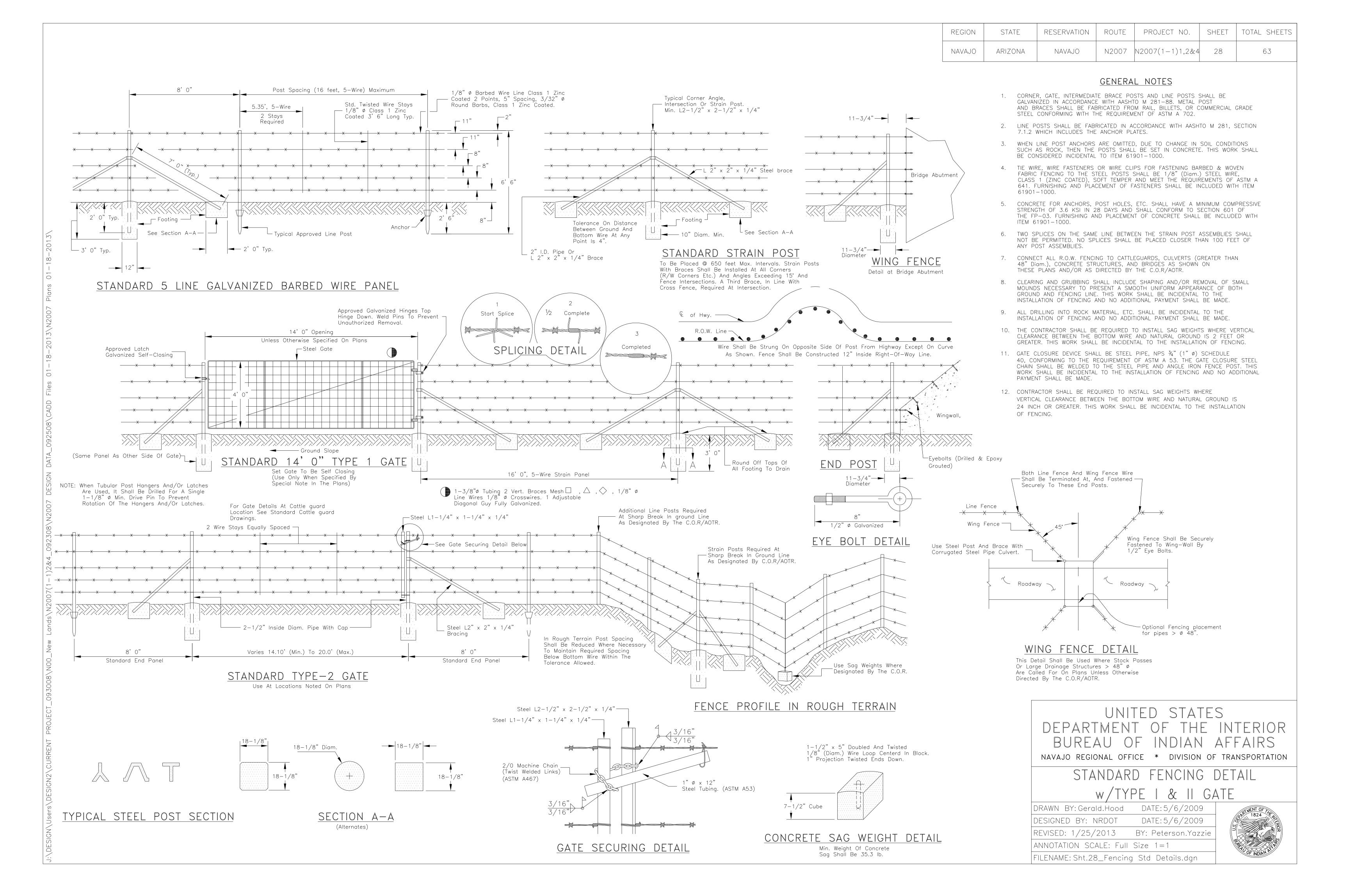
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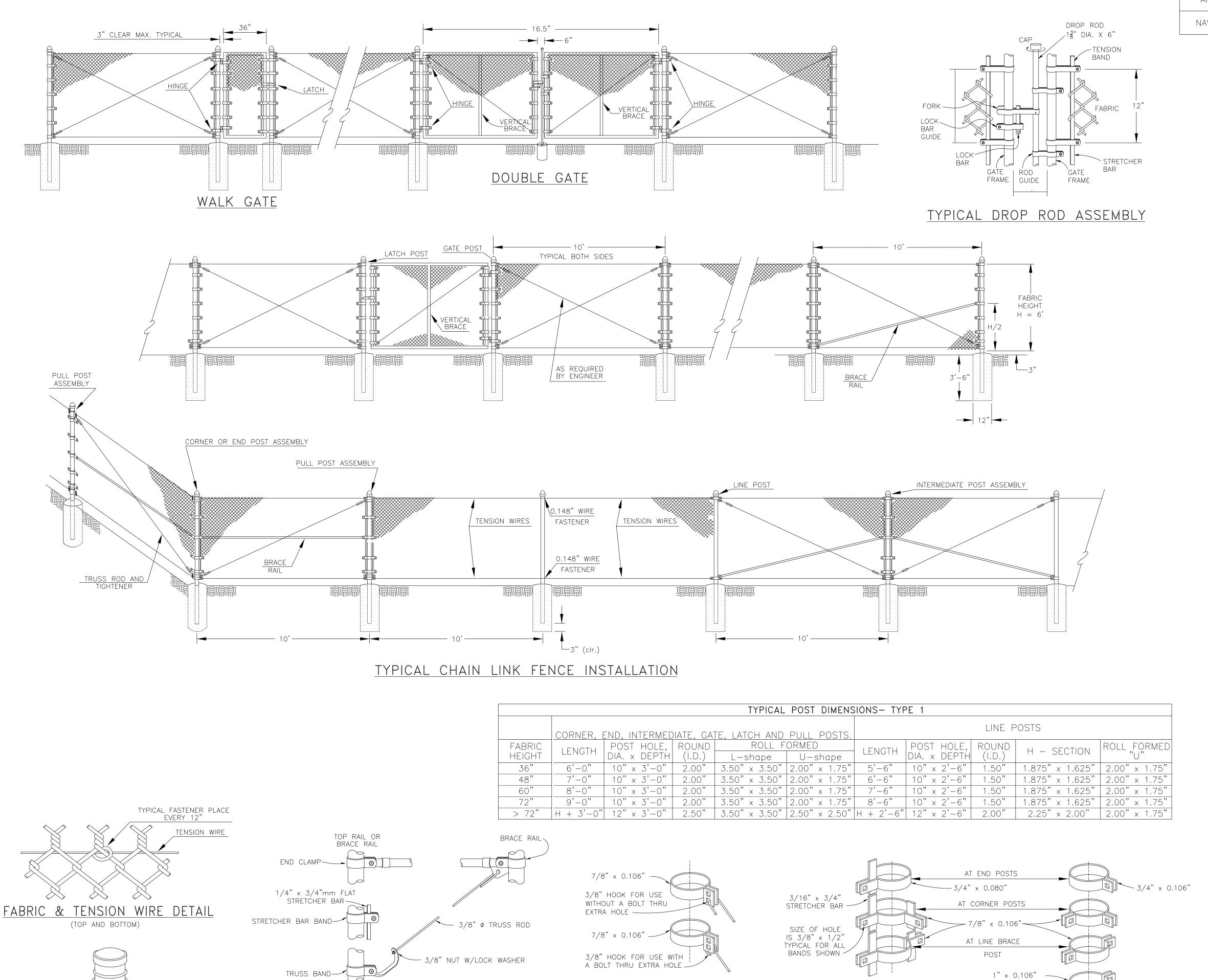
NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

PLACE RIPRAP DROWN DRAIN DETAIL

DRAWN BY: Gerald.Hood	DATE:5/7/2009
DESIGNED BY: NRDOT	DATE:5/7/2009
REVISED: 1/25/2013	BY: Peterson.Yazzie
ANNOTATION SCALE: Full	Size 1=1
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NOTE: BANDS TO BE RECTANGULAR FOR H-BEAM POSTS.

BRACE & TRUSS BANDS

GATE DETAIL

0.148" WIRE FASTENER

(TOP AND BOTTOM OF POST)

USE 3/16" x 1-1/4" CARRIAGE BOLTS FOR ALL BANDS SHOWN -

TENSION BANDS

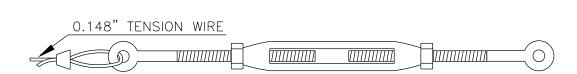
BRACE BANDS

AREA STATE RESERVATION ROUTE PROJECT NO. SHEET TOTAL SHEETS

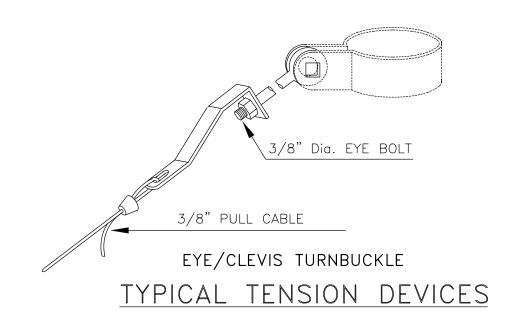
NAVAJO ARIZONA NAVAJO N2007 N2007(1-1)1,2&4 29 63

GENERAL NOTES

- POSTS SHALL BE ROUND PIPE, H-SECTION OR ROLL FORMED AND SHALL CONFORM TO THE NOMINAL DIMENSIONAL REQUIREMENTS SHOWN ON THE PLANS. IN ADDITION, THE MATERIAL OF WHICH POST ARE FABRICATED SHALL HAVE A NOMINAL THICKNESS, BEFORE GALVANIZING, OF NOT LESS THAN 0.111 INCH.
- 2. CHAIN LINK FABRIC SHALL BE EITHER ZINC-COATED OR ALUMINUM-COATED STEEL WIRE FENCE FABRIC. ZINC-COATED STEEL FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A392, CLASS 1 COATING. ALUMINUM-COATED STEEL FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A491, WITH A MINIMUM WEIGHT OF COATING OF 0.40 OUNCE PER SQUARE FOOT OF WIRE SURFACE AREA. FABRIC SHALL BE 11 GAUGE FOR ALL FENCE FABRIC 60 INCHES OR LESS IN HEIGHT AND SHALL BE 9 GAUGE FOR FABRICS GREATER THAN 60 INCHES IN HEIGHT.
- 3. TENSION WIRES SHALL BE 7 GAUGE (0.177 INCH DIAMETER) COIL SPRING STEEL WIRE WITH A MINIMUM TENSILE STRENGTH OF 75,000 PSI, AND SHALL BE ZINC—COATED OR ALUMINUM—COATED.
- 4. TRUSS RODS SHALL BE 3/8-INCH DIAMETER ADJUSTABLE RODS. TRUSS TIGHTENERS SHALL HAVE A STRAP THICKNESS OF NOT LESS THAN 1/4-INCH.
- 5. STRETCHER BARS SHALL BE 3/16-INCH BY 3/4-INCH STEEL FLAT BARS. STRETCHER BAR BANDS SHALL BE 1/8-INCH BY 1-INCH PREFORMED STEEL BANDS.
- 6. BOTTOM TENSION WIRE SHALL BE 5-INCHES FROM TOP OF CROWN ON CONCRETE FOOTINGS.
- 7. INTERMEDIATE POST ASSEMBLIES SHALL BE SPACED AT 500 FOOT INTERVALS OR MIDWAY BETWEEN PULL POSTS WHEN THE DISTANCE BETWEEN SUCH POSTS IS LESS THAN 1,000 FEET AND MORE THAN 500 FEET
- 3. CHAIN LINK FENCE POST DIAMETERS SHALL BE , AS FOLLOWS:
 - a) 1.66" O.D. TOP & BRACE RAILS AND GATE FRAMES TO 6'
 - b) 1.90" O.D. FOR LINE POSTS AND GATE FRAMES TO 13' WIDTH. c) 2.875" O.D. FOR END POSTS, CORNER POSTS AND GATE POSTS FOR SINGLE GATE OPENINGS TO 6' WIDTHS.
- d) 4.00" O.D. FOR GATE POSTS FOR SINGLE GATE OPENINGS TO 13' WIDTH AND DOUBLE GATE OPENINGS.
- 9. NEW DOUBLE GATE ASSEMBLIES SHALL INCLUDE ALL MATERIALS AND LABOR BETWEEN AND INCLUDING THE OUTER GATE POSTS. WHEN NEW GATE ASSEMBLIES ARE A PART OF EXISTING FENCE RELOCATION, THE RESET (EXISTING) MATERIAL SHALL STOP AT AND CONNECT TO THE OUTER GATE POSTS. IF THE ADJOINING FENCE IS EQUIPPED WITH A BARBED WIRE TOP, INSTALL A MATCHING BARBED WIRE TOP ON THE GATE ASSEMBLY FROM OUTER GATE POST TO OUTER GATE POST INCLUDING THE GATE LEAFS. ON THE GATE LEAFS, INSTALL BARBED WIRE SUPPORTS AT BOTH GATE ENDS AND AT EACH VERTICAL BRACE. FENCING BETWEEN GATE ENDS SHALL INCLUDE TOP AND/OR BOTTOM TENSION WIRES OR TOP RAIL AS NEEDED TO MATCH ADJOINING FENCINGS.



3/8" EYE & EYE/TURNBUCKLE



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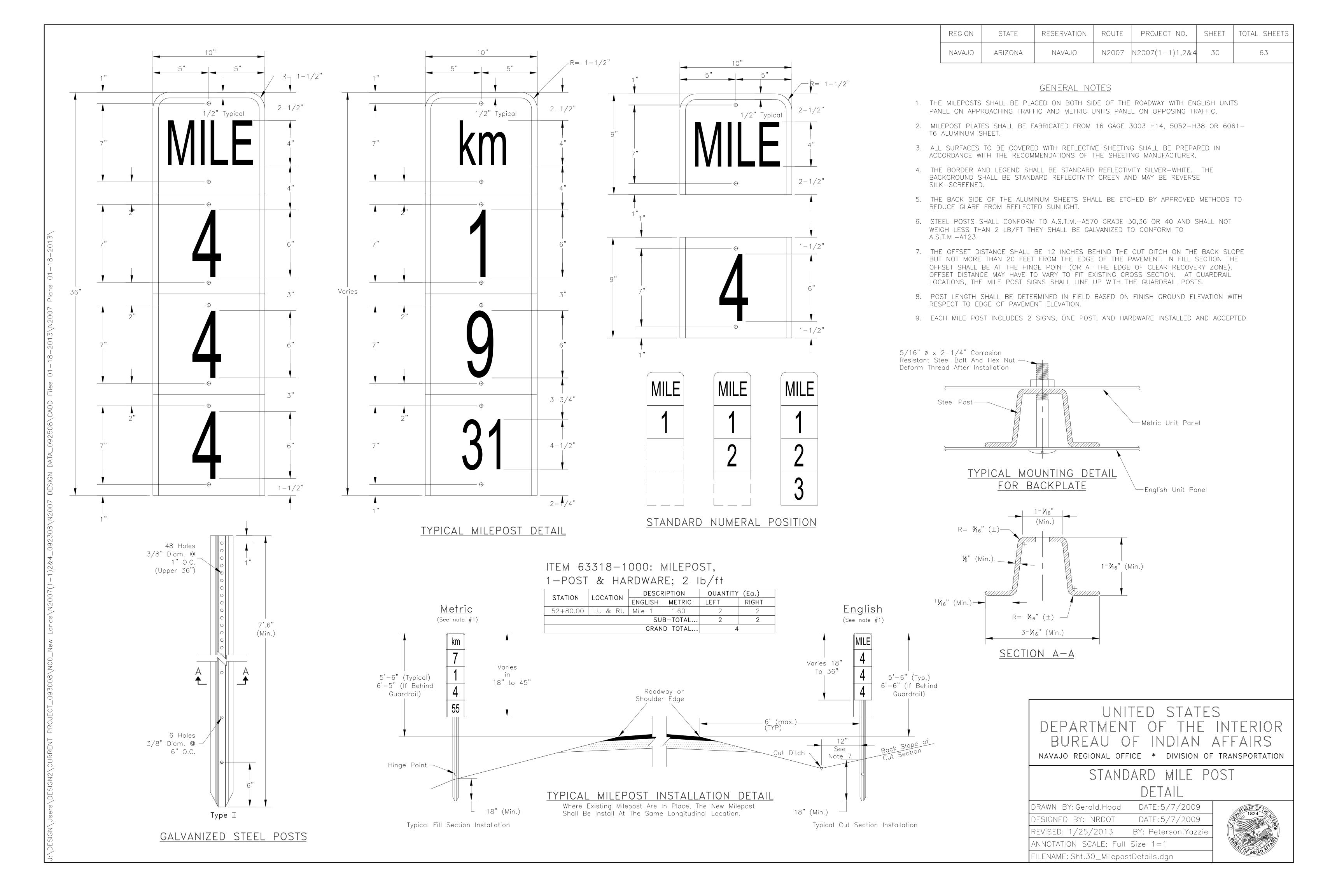
CHAIN LINK FENCE DETAILS

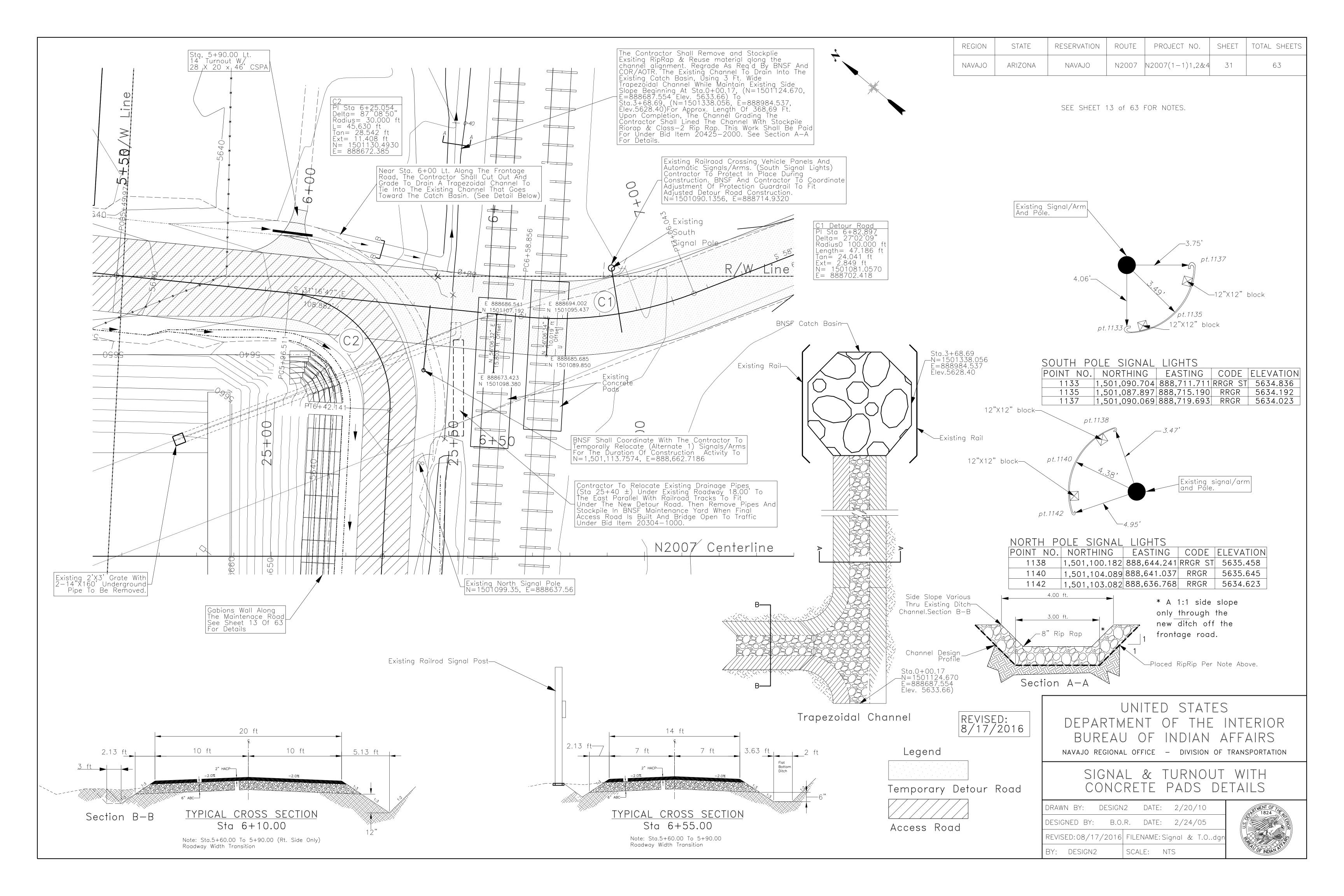
DRAWN BY: Gerald.Hood	DATE: 1/24/2013
DESIGNED BY: NRDOT	DATE: 1/24/2013
REVISED: 2/1/2013	BY: Peterson.Yazzie

ANNOTATION SCALE: Full Size 1=1

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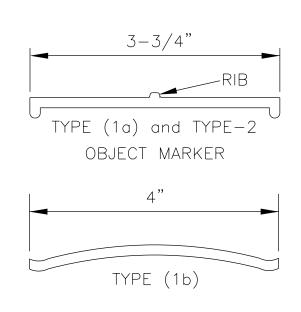




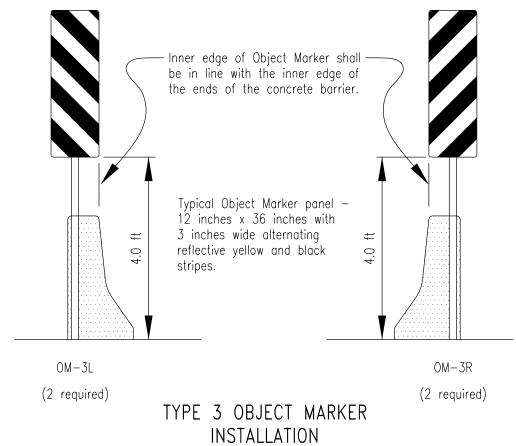
GENERAL NOTES

- 1. THE CONTRACTOR HAS THE OPTION TO EITHER USE GLASS FIBER OR ENGINEERED PLASTIC TYPE FOR DELINEATOR AND/OR TYPE II OBJECT MARKER. THE CONTRACTOR SHALL NOT USE A COMBINATION OF BOTH, STEEL "U" CHANNEL SHALL BE ATTACHED TO THE DELINEATOR, AND SHALL BE INCLUDED IN CONTRACT ITEM 63309-0010 AND 63309-0020.
- 2. TYPE "B" DIKE SHALL BE USED ON THIS PROJECT UNLESS OTHERWISE NOTED. EMBANKMENT MATERIAL NEEDED TO BUILD EARTHEN DIKE SHALL BE CONSIDERED INCIDENTAL TO ITEM 20410-2000, FURROW DITCHES, DITCH BLOCKS AND DIKES.

TYPE	POST COLOR	HIGH INTENSITY REFLECTIVE SHEETING
1a	WHITE	WHITE, ONE SIDE
1b	WHITE	WHITE, BOTH SIDES
2	YELLOW	AMBER, ONE SIDE



Glass Fiber Type SECTION A-A



	Inner edge of Object Marker be in line with the inner edg the ends of the concrete ba	ge of
4.0 ft	Typical Object Marker pane 12 inches x 36 inches with 3 inches wide alternating reflective yellow and black stripes.	

Type "2" at Culvert Locations

-3-3/4" (Type 2)

Typical spacing on tangent = 525 ft

525 ft max.

2nd∮

3.2 ft from edge of paved shoulder

3" x 12" REFLECTIVE

SHEETING

3-3/4" (Type 1a) 4" (Type 1b)

5.5 ft

3rd 👌

 $2nd\phi$

(Type-2 Object Marker 6" x 12")

"U" Channel attached to post with (2) 5/16"ø hex head bolts with flat washer against post, lock washer and

and hex nut against "U" Channel as shown.

DELINEATOR

(FLEXIBLE TYPE)

	Approximate			
Radius of	Spacing (S) on	Spacing	on Advan	ce of or Beyond a
Curve (feet)	Curve (feet)	Curve (·
		A (2S)	B (3S)	C (6S)
50	20	40	60	120
115	25	50	75	150
180	35	70	105	210
250	40	80	120	240
300	50	100	150	300
400	55	110	165	330
500	65	130	195	390
600	70	140	210	420
700	75	150	225	450
800	80	160	240	480
900	85	170	255	510
1000	90	180	270	540
1100	97	194	292	583
1200	102	203	305	610
1300	106	212	318	636
1400	110	220	331	661
1500	114	228	343	685
1600	118	236	354	709
1700	122	244	366	731
1800	125	251	376	753
1900	129	258	387	774
2000	132	265	397	795
2500	148	297	445	891
3000	163	326	489	978
3500	176	352	529	1057
4000	189	377	566	1131
4500	200	400	600	1201
5000	211	422	633	1266
5500	221	443	664	1329
6000	231	463	694	1388
6500	241	482	723	1446
7000	250	500	750	1501
7500	259	518	777	1554

S = 3.0 * sq. rt.(R-50).Spacing for specific radii may be interpolated from table. The spacing on curves should not exceed 300 feet.

Shaded areas denotes to use 300 feet spacings.

Delineators should be spaced 200 to 530 feet apart on mainline tangent sections. NOTE: When uniform spacing is interrupted by such features as culverts, signs, driveways, intersections, delineators which would ordinarily be located within the features may be relocated in either direction for a distance not exceeding one quarter of the uniform spacing. Delineators still falling within such features may be eliminated.

63309-0010	Delineators,	Type	"1a"
63309-0020			

		· · · / / / · ·	
STATION	LOCATION	DESCRIPTION	Each
1+36.06	Lt & Rt.	DL_1A	2
21+36.07	Lt & Rt.	DL_1A	2
45+77.31	Lt & Rt.	DL_1A	2
50+77.31	Lt & Rt.	DL_1A	2
55+77.31	Lt & Rt.	DL_1A	2
TOTAL			10
STATION	LOCATION	DESCRIPTION	
33+98.93	Rt.	DL_1B	1
34+83.39	Rt.	DL_1B	1
36+52.31	Rt.	DL_1B	1
38+22.31	Rt.	DL_1B	1
40+77.31	Rt.	DL_1B	1
		TOTAL.	5

ITEM 63308-2000 FLEXIBLE

YPE 2 OI	BJECI MA	RKER
STATION	LOCATION	QTY.
6+60	Lt. & Rt.	2
36+00	Lt. & Rt.	2
47+40	Lt. & Rt.	2
	TOTAL	6

ITEM 63308-3000 TYPE 3 OBJECT MARKER

STATION	LOCATION	QTY.
24+68.00	Rt.	1 (OM-3R)
24+68.00	Lt.	1 (OM-3L)
33+43.77	Rt.	1 (OM-3R)
33+43.77	Lt.	1 (OM-3L)
	TOTAL	4

Included with Bridge Quantities on Sheet B-1

62101-0000 R/W MONUMENTS 62102-0000 Reference Markers

STATION	REQUIRED	LOCATION	REMARK
0+68.940	1	Left	
0+68.940	1	Right	
9+56.340	2	Left	
9+56.340	2	Right	
28+94.000	2	Left	
28+94.000	1	Right	
30+61.099	1	Left	
30+61.099	1	Right	
31+58.000	2	Right	
32+71.099	1	Left	
32+71.099	1	Right	
34+42.308	1	Left	
34+42.308	1	Right	
36+52.308	1	Left	
36+52.308	1	Right	
57+82.737	1	Left	
57+82.737	1	Right	
TOTA	L 21		

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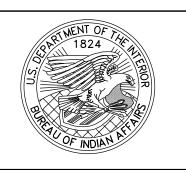
NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

MISCELLANCEOUS DETAILS

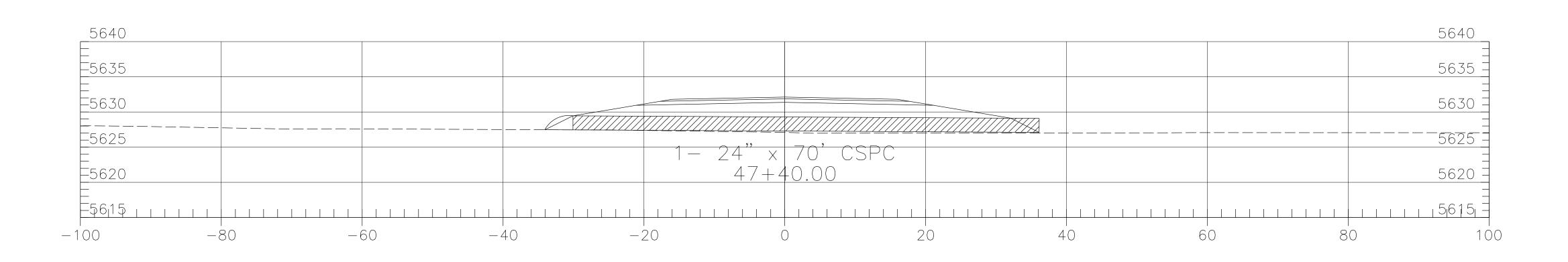
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DESIGNED BY: NRDOT	DATE:5/7/2009
REVISED: 1/25/2013	BY: Peterson.Yazzie

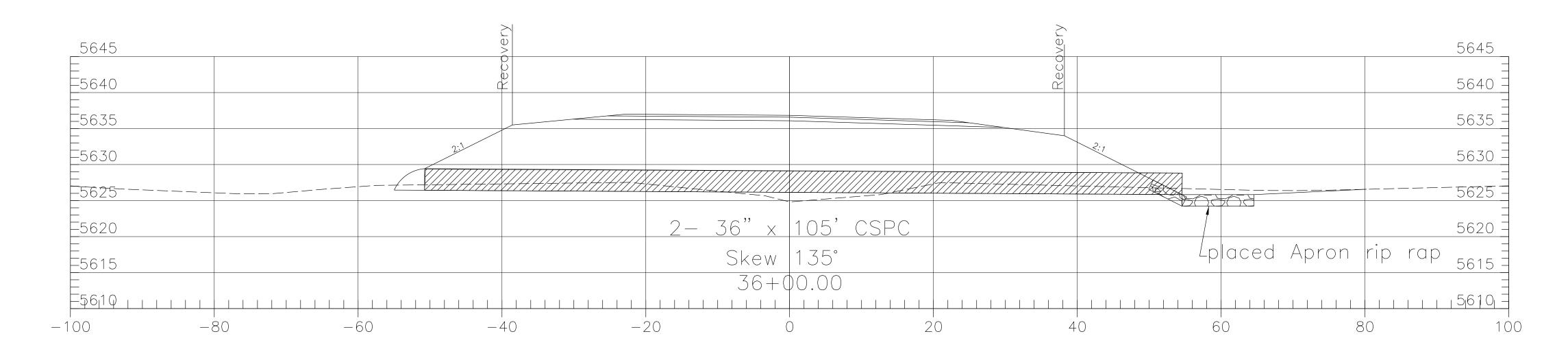
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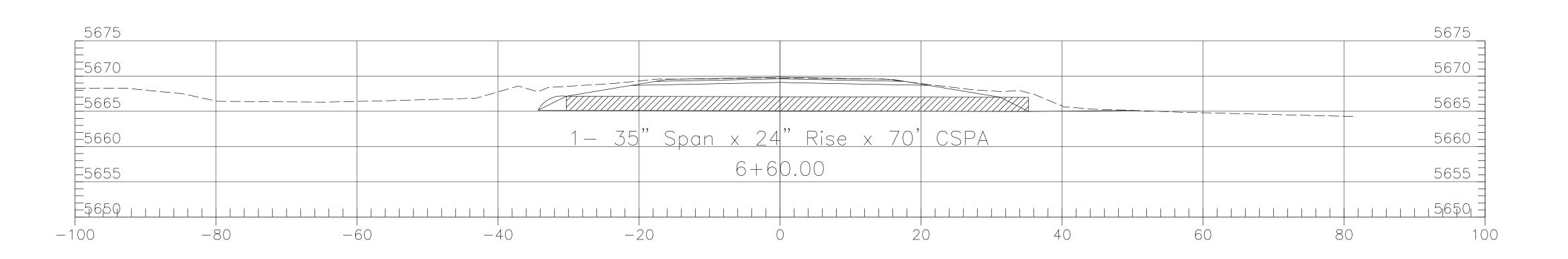




REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	OLAVAN	N2007	N2007(2)1,2&4	33	63







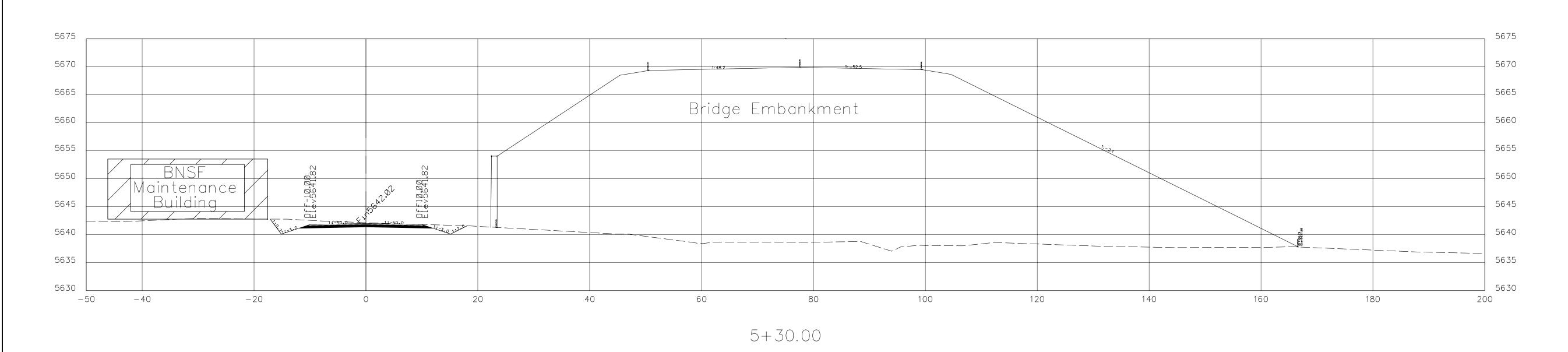
UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE * DIVISION OF TRANSPORTATION

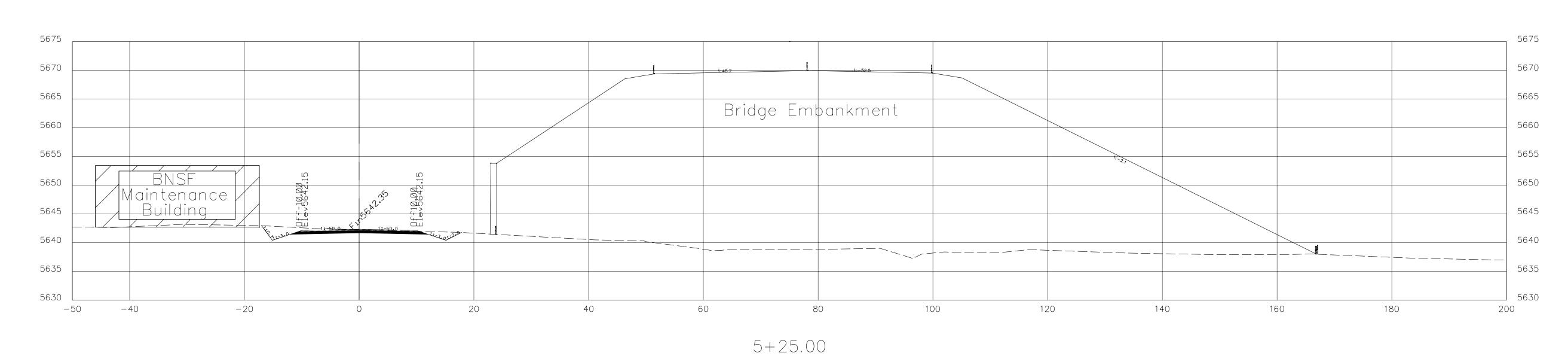
PIPE CROSS SECTIONS

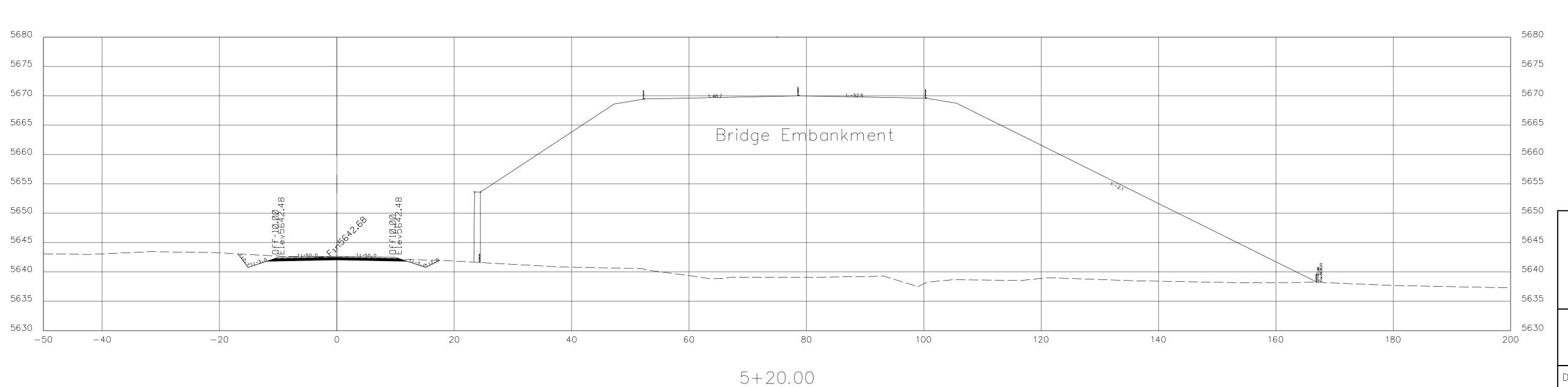
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DESIGNE	D BY:	Design	2	DATE:	07/29	9/0	5
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BY: B.	.O.R.		SCA	ALE:1:10	(Horiz.	&	Ver



REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	NAVAJO	N2007	N2007(1-1)1,2,4	34	63







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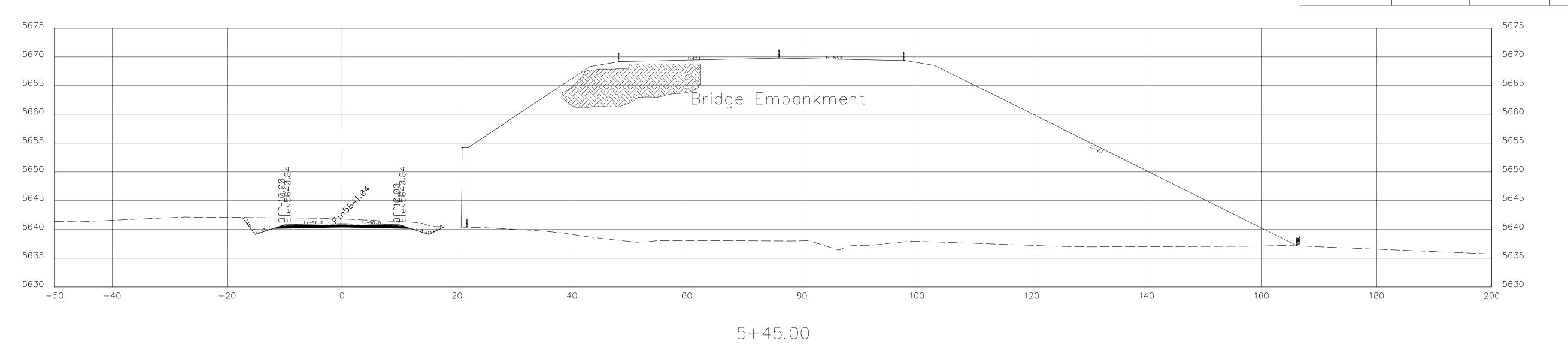
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

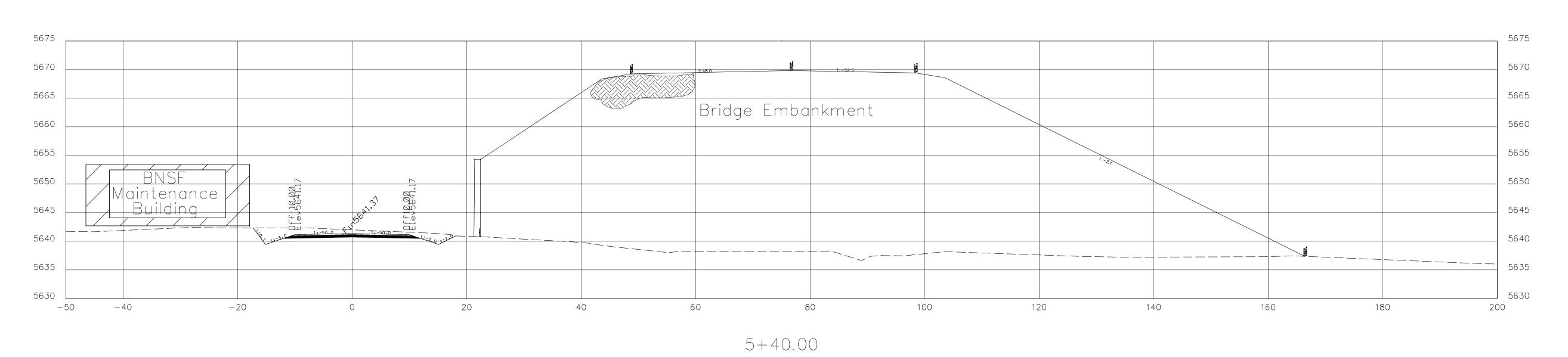
FRONTAGE & DETOUR RD SECTIONS

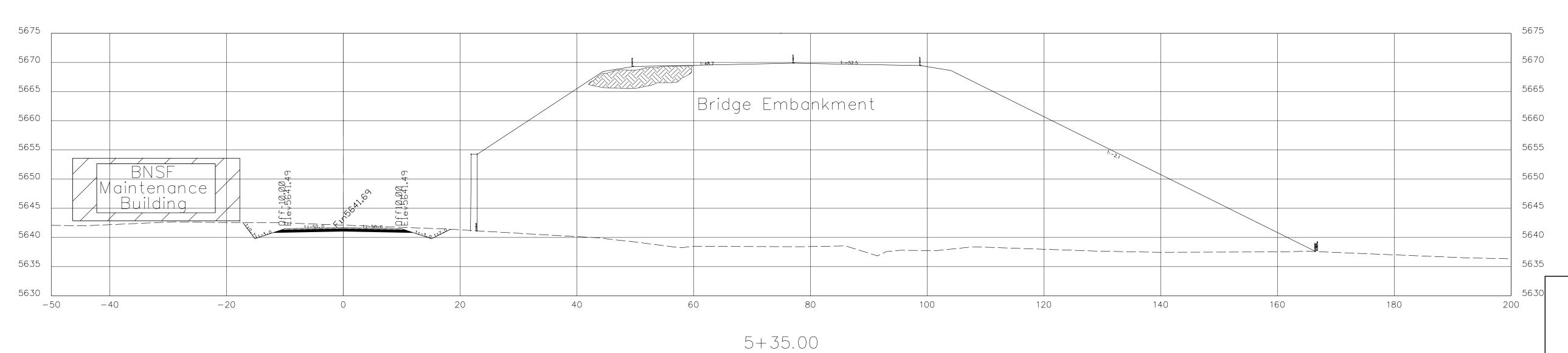
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REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	NAVAJO	N2007	N2007(1-1)1,2,4	35	63







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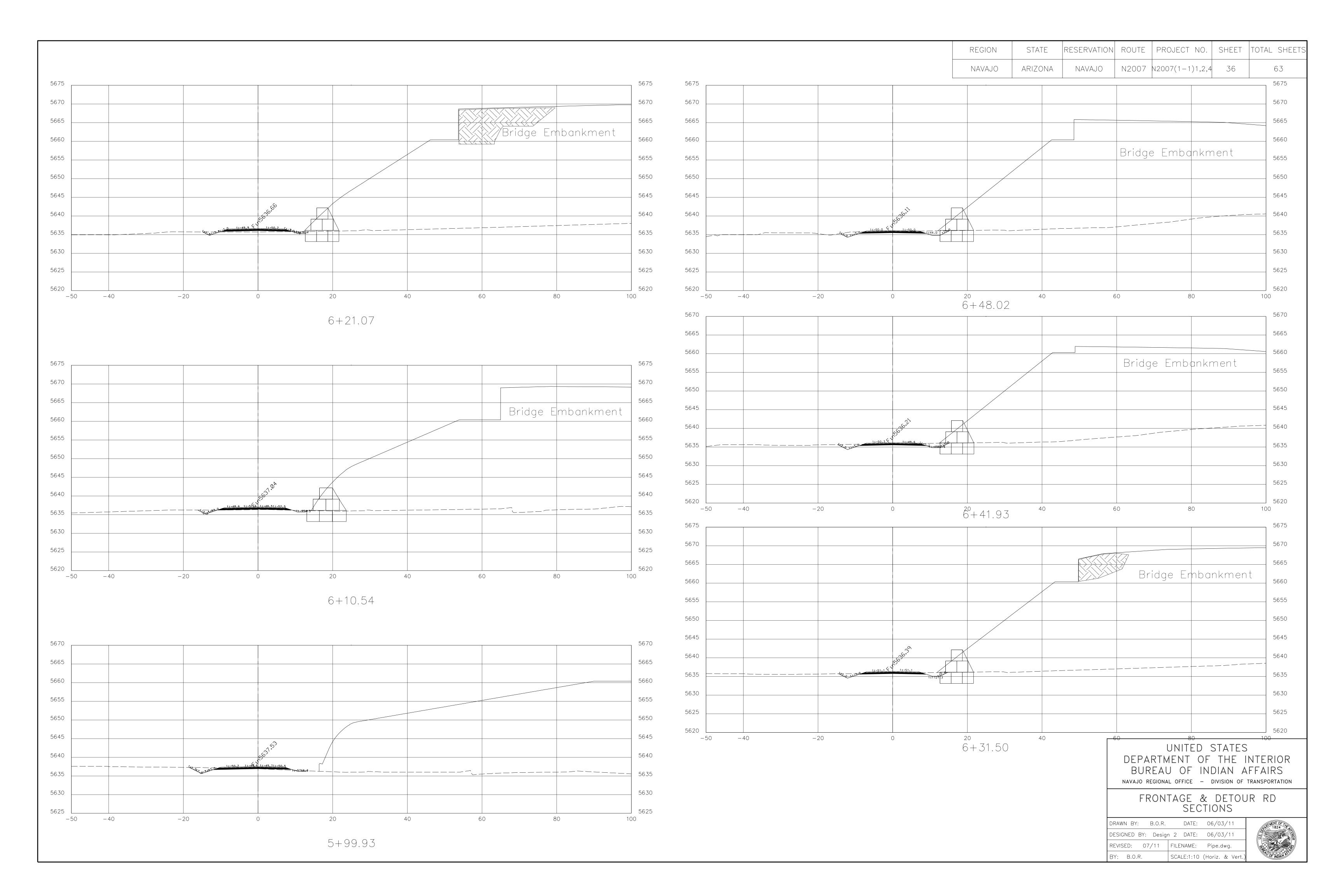
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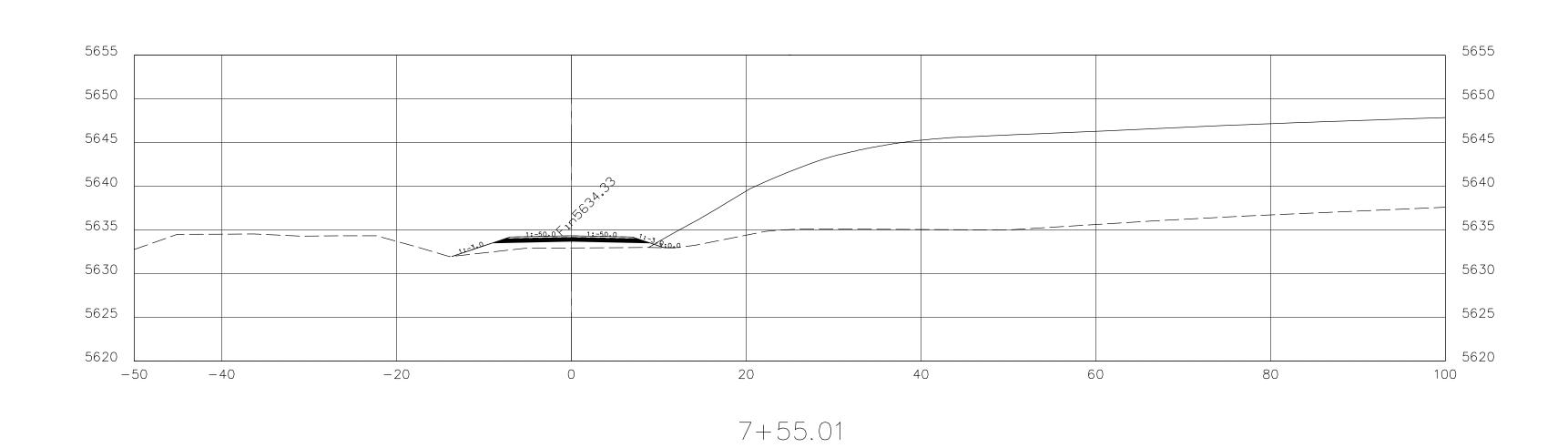
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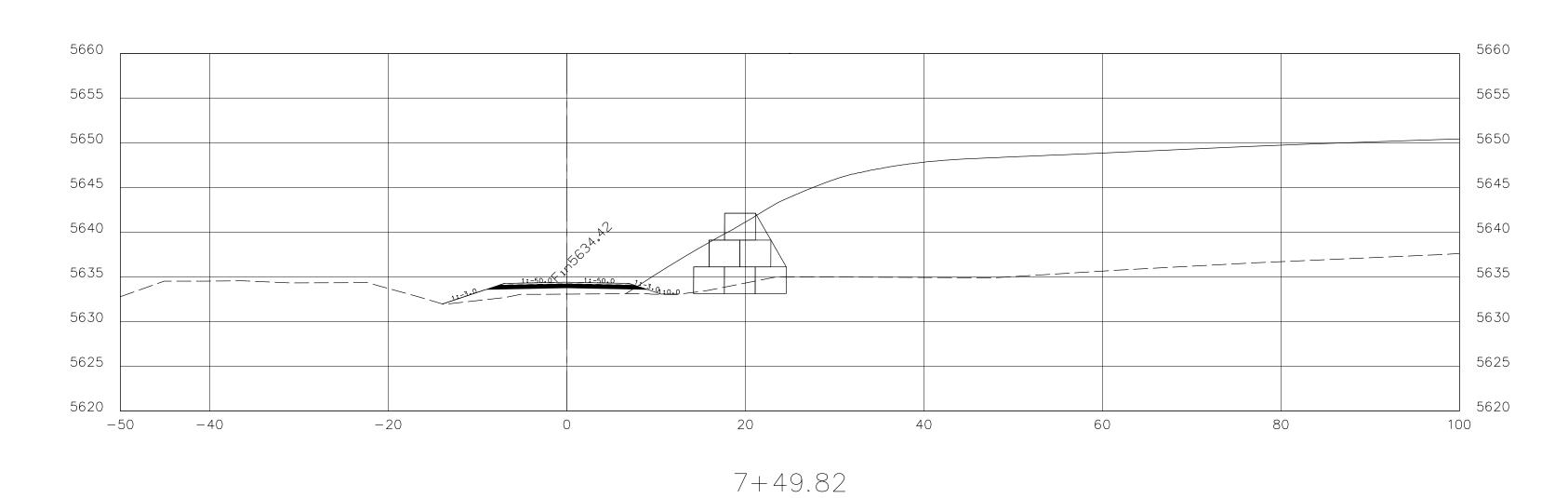
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DESIGNED BY: Design	n 2 DATE:	06/03/11
REVISED: 07/11	FILENAME:	Pipe.dwg.
BY: B.O.R.	SCALE:1:10	(Horiz. & Vert.)

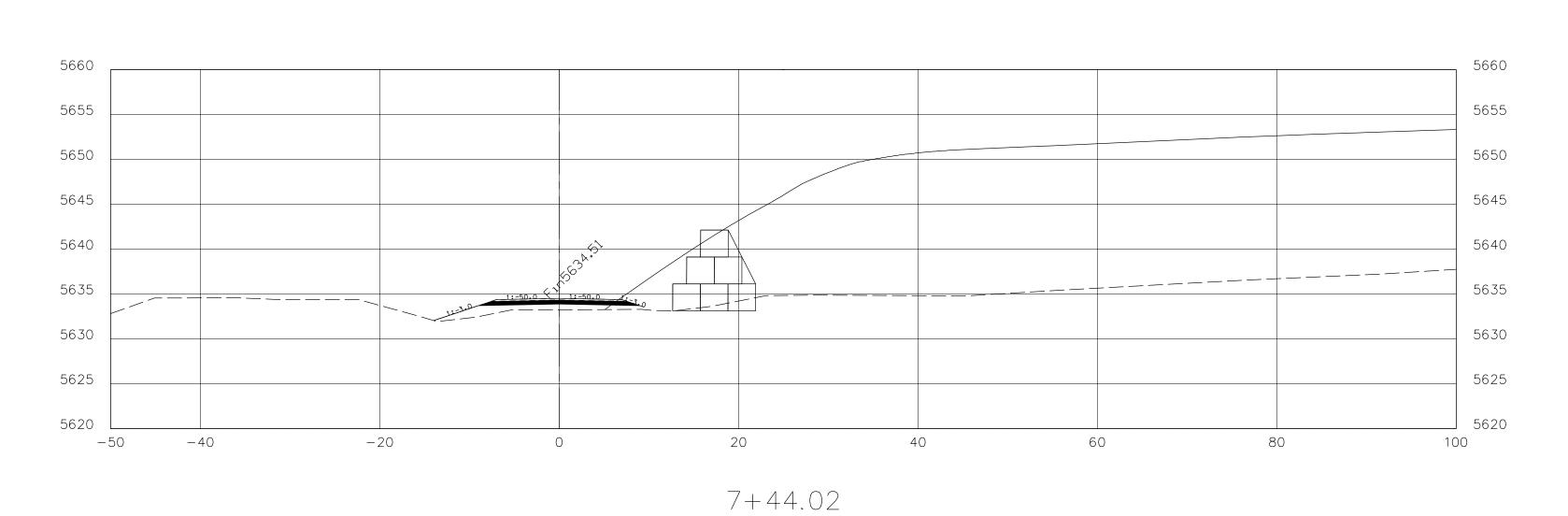




REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	ARIZONA	NAVAJO	N2007	N2007(1-1)1,2,4	37	63







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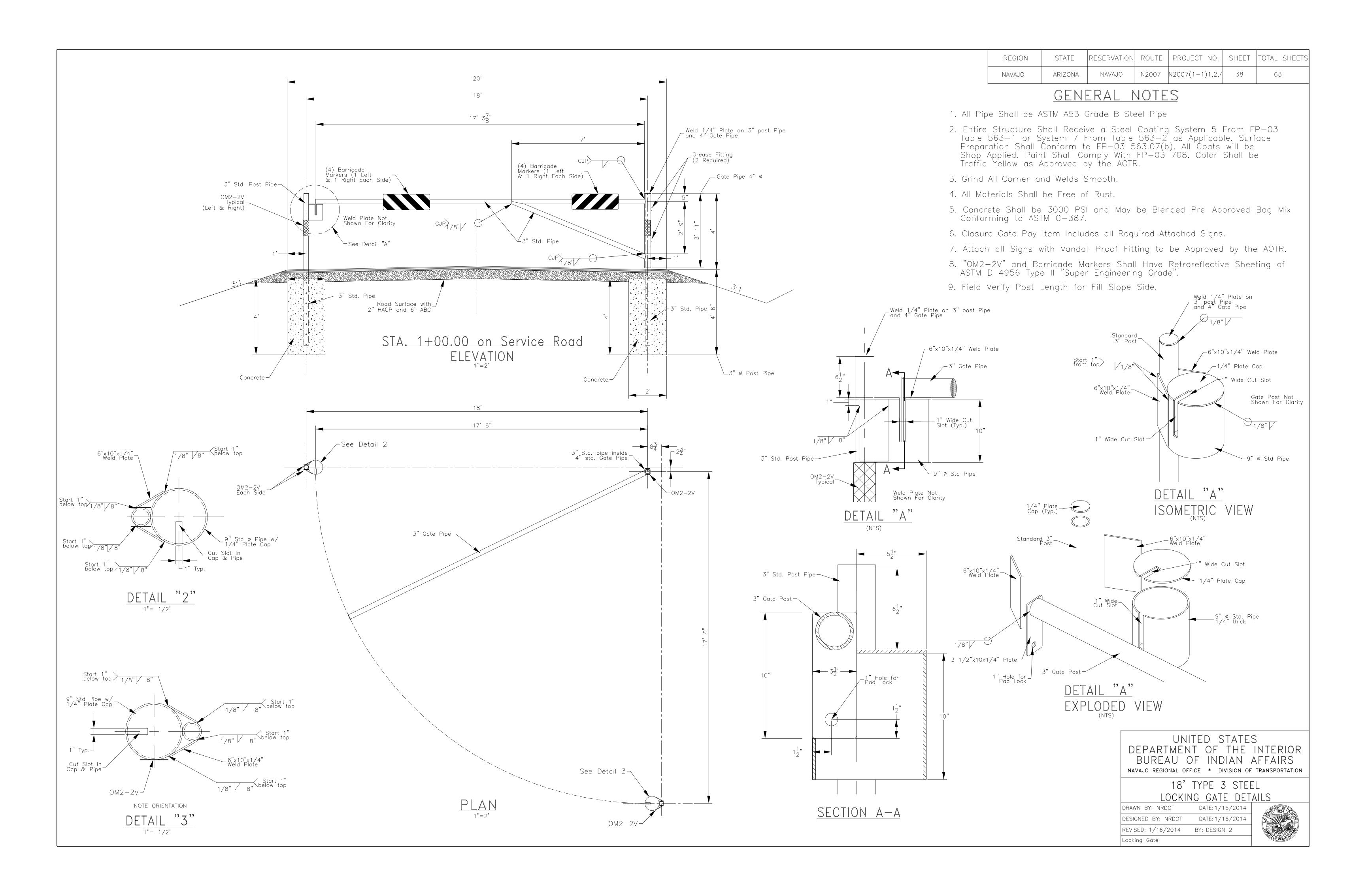
BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

FRONTAGE & DETOUR RD SECTIONS

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DESIGNED BY:	Desigr	n 2	DATE:	06/03	/11
REVISED: 07	7/11	FILE	NAME:	Pipe.d	wg.
BY: B.O.R.		SCA	LE:1:10	(Horiz.	& Vert





BRIDGE GENERAL NOTES

- 1. SPECIFICATIONS: DESIGN; AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1992, 15th EDITION. CONSTRUCTION: STANDARD SPECIFICATIONS FOR CONSTRUCTION OF ROADS AND BRIDGES ON FEDERAL HIGHWAY PROJECTS, FP-03, ENGLISH EDITION, AND SUPPLEMENTAL SPECIFICATIONS.
- 2. UNITS: THIS PROJECT HAS BEEN DESIGNED AND DRAWN USING THE U.S. CUSTOMARY (ENGLISH) SYSTEM OF UNITS. UNLESS OTHERWISE NOTED, ALL VALUES ARE GIVEN IN U.S. CUSTOMARY (ENGLISH) UNITS. SLOPES DESIGNATED ON THESE PLANS ARE IN ACCORDANCE WITH SECTION 101.03(d) OF THE FP-03, i.e.; V:H (VERTICAL : HORIZONTAL).
- 3. DESIGN LOADS: DEAD LOADS; CONCRETE = 150 pcf, STEEL = 490 pcf, FUTURE WEARING SURFACE = 25 psf OF ROADWAY SURFACE, EARTH PRESSURE = FLUID WEIGHING 36 pcf. LIVE LOADS; HS 20-44 PLUS IMPACT. IMPACT = 50/(L+125) WHERE L = SPAN LENGTH IN FEET. MAXIMUM IMPACT FACTOR = 0.30.
- 4. RATINGS: INVENTORY RATING = HS 22.0. OPERATING RATING = HS 36.8.
- 5. DESIGN PARAMETERS: REINFORCED CONCRETE DESIGNED BY LOAD FACTOR DESIGN WITH f'c = 4000 psi AND fy = 60,000 psi. TRANSVERSE DECK SLAB SERVICEABILITY STRESSES LIMITED TO f'c = 1,400 psi AND MAXIMUM STRESS IN REINFORCING STEEL OF fs = 20,000 psi. PRECAST, PRESTRESSED GIRDERS DESIGNED IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1992, 15th EDITION CRITERIA. ULTIMATE STRENGTH OF $\frac{1}{2}$ " DIA. SEVEN WIRE, BRIGHT, STRESS RELEIVED, LOW RELAXATION, PRESTRESSING STRAND SHALL BE 41,300 POUNDS WITH A MINIMUM f's = 270,000 psi.
- 6. CONCRETE: ALL CAST IN PLACE CONCRETE SHALL BE CLASS A(AE) WITH THE 28 DAY COMPRESSIVE STRENGTH INDICATED IN THESE PLANS. THE AIR CONTENT FOR ALL CLASS A(AE) CONCRETE SHALL NOT BE LESS THAN THAT SPECIFIED IN THE FP-03. CONCRETE IN PRECAST, PRESTRESSED CONCRETE GROERS SHALL BE CLASS P AND SHALL HAVE THE MINIMUM STRENGTHS INDICATED IN THESE PLANS. CONCRETE FOR THE DECK SLAB AND ALL DIAPHRAGMS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF I'C = 4,500 psi. ALL OTHER CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF I'C = 4,600 psi. CONCRETE IN PRESTRESSED GIORERS SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH I'C = 6,000 psi. WITH A MINIMUM INDICATED CONCRETE SHALL CONTAIN TYPE II PORTLAND CEMENT. ALL CONCRETE SHALL BE VIBRATED IN ACCORDANCE WITH SPECIFICATIONS. ALL CEMENT SHALL BE LOW ALKALAI CEMENT AND NO ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL BE USED. THE TIME LIMITS FOR CONCRETE SHALL BE VIBRATED IN TABLE 552-4 OF THE FP-03 SHALL APPLY. IF CONCRETE CANNOT BE DISCHARGED WITHIN THE SPECIFIED TIME LIMIT, AN ALTERNATE METHOD OF DELIVERY SUCH AS DRY BATCHING, AN ONSITE BATCHING PLANT CONFORMING TO APPLICABLE SPECIFICATIONS, OR AN APPROVED CONCRETE MIX DESIGN CONTAINING SET RETARDING ADMIXTURES SHALL BE USED. ALTERNATE METHODS OF DELIVERY SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO IMPLEMENTATION. APPROVAL OF ALTERNATE METHODS SHALL BE USED. ALTERNATE METHODS OF DELIVERY SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO IMPLEMENTATION. APPROVAL OF ALTERNATE METHODS SHALL BE GIVEN A GROOVED FINISH IN ACCORDANCE WITH SECTION 552.14 (a), (b) AND (c)(1) OF THE FP-03. THE CONCRETE BARRIER AND PARAPET SURFACES, VERTICAL EDGE OF BRIDGE DECK SURFACES AND BOTTOM OF BRIDGE DECK OVERHANG SURFACES SHALL BE GIVEN A CLASS 2 RUBBED FINISH. ALL OTHER FD-03. ALL STEEL OTHER THAN REINFORCING STEEL EMBEDDED IN CONCRETE SUCH AS EXPANSION JOINTS, GUARD ANGLES, ANCHOR BOLTS, ETC.... SHALL BE CONSIDERED INCIDENTAL TO ITEM 55201-0200, STRUCTURAL CONCRETE CLASS OTHERWISE NOTED.
- 7. REINFORCING STEEL: ALL REINFORCING STEEL SHALL CONFORM TO AASHTO M31, GRADE 60. EPOXY COATED REINFORCING STEEL SHALL ALSO CONFORM TO AASHTO M284. CONVENTIONAL AND EPOXY COATED REINFORCING STEEL SIZES SHOWN ON THESE PLANS ARE U.S. CUSTOMARY (ENGLISH) REINFORCING STEEL SIZES. EPOXY COATED REINFORCING STEEL SHALL BE USED IN THE CONCRETE DECK, ALL DIAPHRAGMS, PARAPETS AND APPROACH SLABS. THE MINIMUM COVER FOR ALL REINFORCING STEEL SHALL BE 2 INCHES UNLESS OTHERWISE SPECIFIED. LENGTHS OF REINFORCING STEEL BARS SHOWN IN PLANS INCLUDE REQUIRED SPLICE LENGTHS FOR SPLICES SHOWN. ANY OTHER SPLICES FOR THE CONVENIENCE OF THE CONTRACTOR AND SHALL NOT BE UTILIZED UNTIL WRITTEN APPROVAL IS GRANTED BY THE AWARDING OFFICIAL/CONTRACTING OFFICER (AO/CO). REINFORCING STEEL QUANTITIES FOR APPROVED SPLICES FOR THE CONVENIENCE OF THE CONTRACTOR.
- 8. PRESTRESSING STEEL: PRETENSIONED, PRESTRESSING STEEL STRANDS SHALL BE 270 ksi, 1/2 INCH Ø, SEVEN WIRE BRIGHT, LOW RELAXATION PRESTRESSING STEEL STRANDS CONFORMING TO AASHTO M203, INCLUDING THE REQUIREMENTS OF SUPPLEMENT 1. EACH STRAND SHALL BE PRETENSIONED TO A TOTAL LOAD OF 31,000 POUNDS AT fsi = 0.75 (f's) = 202,500 psi. ESTIMATED LOSSES AND FINAL PRESTRESSED FORCES ARE AS FOLLOWS; SPAN 1 AND 4 GIRDERS = 47,200 psi LOSSES, 23,800 POUNDS PER STRAND; SPAN 2 AND 3 = 48,400 psi LOSSES, 23,600 POUNDS PER STRAND.
- 9. PRESTRESSED CONCRETE GIRDERS: PRESTRESSED CONCRETE GIRDERS SHALL BE MANUFACTURED AS DETAILED IN THESE PLANS. ALL CONCRETE, REINFORCING STEEL, PRESTRESSING STEEL, LIFTING DEVICES, INSERTS AND ACCOMPANYING BOLTS, NEOPRENE ELASTOMERIC BEARING PADS, AND ANY OTHER MATERIALS NECESSARY FOR THE FABRICATION, TRANSPORTATION AND ERECTION OF THE PRESTRESSED CONCRETE GIRDERS SHALL BE CONSIDERED INCIDENTAL TO ITEM 55301-2000 AND ITEM 55301-2010. NEOPRENE ELASTOMERIC BEARING PADS SHALL CONFORM TO AASHTO M251 AND SHALL BE 60 DUROMETER HARDNESS.
- 10. STRUCTURAL STEEL: STRUCTURAL STEEL FOR EXPANSION JOINT RAILS AND PLATES SHALL CONFORM TO AASHTO M270 GR. 36. WELDED ANCHOR STUDS SHALL CONFORM TO AASHTO 169. DIAPHRAGM ANCHOR BOLTS SHALL CONFORM TO ASTM A307. ALL THE ABOVE ITEMS INCLUDING GIRDER SHOE PLATES AND ALL BOLTS, EXCEPT DIAPHRAGM ANCHOR BOLTS, SHALL BE GALVANIZED AFTER FABRICATION.
- 11. WELDING: ALL WELDING SHALL BE IN ACCORDANCE WITH ANSI/AASHTO/AWS D1.5M/D1.5:2008 BRIDGE WELDING CODE, INCLUDING MATERIALS, WORKMANSHIP, INSPECTION AND QUALITY CONTROL. INSPECTION OF SHOP WELDS SHALL BE OUTLINED AND PERFORMED IN THE SHOP AS PART OF THE QUALITY CONTROL PROCESS OF THE FABRICATION PLANT. QUALITY CONTROL PLANS OF THE FABRICATION PLANT SHALL BE SUBMITTED ALONG WITH SHOP DRAWINGS FOR ALL FABRICATED PRODUCTS. INSPECTION OF ALL FIELD WELDS SHALL BE IN ACCORDANCE WITH SPECIAL CONTRACT REQUIREMENT 16(k) AND SHALL BE MEASURED AND PAID UNDER ITEM 15301-0000.
- 12. STRUCTURE TRANSITION RAILINGS: QUANTITIES SHOWN UNDER ITEM 61707-0000 ARE FOR THE THRIEBEAM TRANSITIONS BETWEEN CONCRETE PARAPET AND STANDARD GUARDRAIL SHOWN ON SHEET B-21, INCLUDING
 THE THRIE BEAM TERMINAL CONNECTOR ATTACHMENT TO THE CONCRETE PARAPET, ALL ATTACHMENT HARDWARE AND WORK, THE W-BEAM TO THRIE BEAM TRANSITION RAIL, ALL POSTS AND BLOCKS AS DETAILED ON
 SHEET B-21, AND ALL ASSOCIATED HARDWARE. SEE SHEET 3 OF THE ROADWAY PLANS FOR STANDARD GUARDRAIL QUANTITIES AND SHEETS 12 AND 13 OF THE ROADWAY PLANS FOR STANDARD GUARDRAIL DETAILS.
- 13. DRILLED SHAFTS: CASING OF THE DRILLED SHAFTS MAY BE REQUIRED TO PREVENT CAVING OF SURROUNDING MATERIAL. PAYMENT FOR ANY NECESSARY CASING WORK, INCLUDING MATERIALS, EQUIPMENT AND LABOR, SHALL BE INCLUDED IN ITEMS 56501—0600 AND 56501—0800. ITEMS 56501—0600 AND 56501—0800 QUANTITIES INCLUDE DRILLED SHAFTS FROM THE APPROVED TIP ELEVATIONS TO THE BOTTOM OF ABUTMENT CAP ELEVATIONS AT ABUTMENTS, AND FROM THE APPROVED TIP ELEVATIONS TO THE CONSTRUCTION JOINT (CJ) ELEVATIONS SHOWN ON SHEET B—11 FOR ALL PIERS, AND INCLUDES ALL WORK, LABOR, MATERIALS (INCLUDING CONCRETE AND REINFORCING STEEL), EQUIPMENT AND WORKMANSHIP NECESSARY FOR THE CONSTRUCTION OF THE DRILLED SHAFTS. CONSTRUCTION OF THE CONCRETE COLUMNS FROM THE CJ ELEVATIONS TO THE PIER CAPS, INCLUDING THE HORIZONTAL STRUTS SHALL NOT BE PAID UNDER THE DRILLED SHAFT ITEMS BUT SHALL BE MEASURED AND PAID FOR UNDER ITEM 55201—0200, STRUCTURAL CONCRETE, CLASS A(AE) AND ITEM 55401—1000. REINFORCING STEEL. GRADE 60.
- 14. PERMANENT STEEL DECK FORMS: PERMANENT STEEL (STAY IN PLACE) DECK FORMS SHALL BE USED FOR THE CONSTRUCTION OF THE CONCRETE DECK OF SPAN 1. AT THE CONTRACTOR'S OPTION, PERMANENT STEEL (STAY IN PLACE) DECK FORMS MAY ALSO BE USED FOR THE CONSTRUCTION OF THE CONCRETE DECK FOR SPANS 2, 3 AND 4. COMPLETE SHOP DRAWINGS/PLANS, DESIGN CALCULATIONS, AND REQUIREMENTS SHALL BE SUBMITTED FOR REVIEW AND SHALL BE APPROVED IN WRITING BY THE AO/CO PRIOR UTILIZATION OF THE PROPOSED SYSTEM. ALL MATERIALS AND WORK (INCLUDING SUBMITTALS INDICATED ABOVE) SHALL BE IN ACCORDANCE WITH SECTION 562 OF THE FP-03 AND SUPPLEMENTAL SPECIFICATIONS.
- 15. REMOVAL OF EXISTING BRIDGE: EXISTING BRIDGE N666 IS TO REMAIN IN PLACE DURING AND AFTER CONSTRUCTION OF THE NEW BRIDGE. THE APPROACH ROADWAYS TO EXISTING BRIDGE N666 SHALL BE OBLITERATED AFTER THE NEW BRIDGE IS CONSTRUCTED AND TRAFFIC IS USING THE NEW BRIDGE, AND CONCRETE BARRIERS SHALL BE PLACED AT THE ENDS OF THE EXISTING BRIDGE N666 AS SHOWN ELSEWHERE IN THESE PLANS TO PREVENT VEHICLES FROM DIRVING ONTO EXISTING BRIDGE N666.
- 16. SPAN 1 CONSTRUCTION: THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION OF SPAN 1, INCLUDING ABUTMENT 1 AND PIER 1, WITH THE BURLINGTON NORTHERN SANTA FE (BNSF) RAILROAD IN ORDER TO PREVENT ANY UNNECESSARY DOWN TIME OR INTERRUPTION OF TRAIN TRAFFIC THROUGH THE CONSTRUCTION SITE. ALL PROJECT STAKEHOLDERS SHALL BE NOTIFIED OF COORDINATION EFFORTS SO THAT ALL WORK CAN BE PERFORMED AS SMOOTHLY AS POSSIBLE.
- 17. SEE SECTION 107 OF THE SUPPLEMENTAL SPECIFICATIONS FOR SECTION (c) AND (c1), RAIL ROAD REQUIREMENTS. THE CONTRACTOR IS ALSO REFERRED TO THE BNSF AGREEMENT WITH THE BIA, SECTION C-1 FOR ADDITIONAL REQUIREMENTS.
- 18. ARTICULATED CONCRETE BLOCK REVETMENT: THIS WORK SHALL CONSIST OF FURNISHING ALL LABOR, EQUIPMENT AND MATERIALS TO PLACE AN ARTICULATED CONCRETE BLOCK REVETMENT SYSTEM ON THE SLOPES OF THE ABUTMENT EMBANKMENTS AS DETAILED IN THESE PLANS. THE SYSTEM SHALL CONSIST OF 8 FT. BY 16 FT. SECTIONS PLACED ADJACENT TO EACH OTHER AND POSITIVELY CONNECTED BY THE MANUFACTURERS RECOMMENDED METHOD TO PROVIDE A HOMOGENEOUS EROSION PROTECTION SYSTEM. IF NECESSARY, IRREGULARLY SHAPED SECTIONS SHALL BE DESIGNED AND FABRICATED TO FIT CORNERS AND OTHER IRREGULAR AREAS. THE SECTIONS SHALL BE MADE OF CONCRETE BLOCKS INTERCONNECTED LATERALLY AND TRANSVERSELY BY STAINLESS STEEL CABLES CAST INTO THE CONCRETE BLOCKS. A GEOTEXTILE FABRIC CONFORMING TO SPECIFICATIONS SHALL BE PLACED/ATTACHED TO THE BOTTOM OF EACH SECTION. THE CONCRETE BLOCKS SHALL BE A TRUNCATED PYRAMID SHAPE WITH THE TOP SURFACE 11.5 IN. BY 11.5 IN. SQUARE AND THE BASE SURFACE 15.5 IN. BY 15.5 IN. SQUARE. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND DESIGN DATA FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION AND USE OF THE SYSTEM.
- 19. CONCRETE BARRIER: ALL CONCRETE AND REINFORCING STEEL SHALL BE PAID UNDER ITEMS 55201-0200 AND 55401-2000.
- 20. THE CONTRACTOR SHALL HAVE A QUALIFIED GEOLOGIST PRESENT DURING THE DRILLING OF THE SHAFTS AND SHALL VERIFY THAT THE HOLES FOR THE SHAFTS ARE AT LEAST 3 m INTO COMPETENT SANDSTONE BEFORE DRILLING FOR THE NEXT SHAFT.

REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	ΑZ	Navajo	N2007	N2007(1-1)1,2&4	B-1	63

BRIDGE ESTIMATED QUANTITIES

ITEM	DESCRIPTION DESCRIPTION	QUANTITY	UNIT	AS BUILT
20403-0000	Unclassified Borrow (Bridge Abutment Embankments)	9868	C. y.	
25112-3000	Articulated Concrete Block Revetment	2846	s. y.	
25302-1000	Gabions, galvanized coated, Class 2.	721	C. y.	
55201-0200	Structural Concrete Class A(AE)	1888	C. y.	
55301-2000	Precast Prestressed Concrete BT-72 Grider 72", 130'-5" long	12	ea.	
55301-2010	Precast Prestressed Concrete BT-72 Girder 72", 129'-4" long	12	ea.	
55401-1000	Reinforcing Steel, Grade 60	186,034	<i>lb</i>	
55401-2000	Reinforcing Steel, Epoxy Coated, Grade 60 $ riangle$	236,909	<i>lb</i>	
56501-0600	Drilled Shafts, 4'-0" diameter	507	lf	
56501-0800	Drilled Shafts, 5'-0" diameter	407	1f	
61707-0000	Structure Transistion Railing (Thrie Beam)	75	lf	
61711-5000	Impact Attenuator, QUADGUARD	2	ea.	
61901-1300	Fence, Chain Link Pedestrain Fence	552	lf	
61901-1800	Fence, Chain Link, 60—inch height	552	lf	
63308-3000	Object Markers, Type 3, 1 Post and Hardware; 2.00 lb/ft.	4	ea.	

The quantites shown above are related to bridge construction only and are not included in the quantities shown on Sheet 3. The quantities shown above shall be combined with the quantities shown on Sheet 3 to obtain the total estimated quantities for the entire project. The total estimated quantities for the entire project are shown on the Bid Schedule.

ITEM 61707-0000 STRUCTURE TRANSITION RAILING

STATION	<i>TO</i> S	STATION	LOCATION	LENGTH (ft)
24+49.83	to	24+68.58	LT.	18.75
24+49.83	to	24+68.58	RT.	18.75
30+21.92	to	30+40.67	LT.	18.75
30+21.92	to	30+40.67	RT.	18.75
			TOTAL:	75.00

The quantities show above include only thrie-beam transitions from concrete barriers to standard guardrailing as detailed on Sheet B-21.

ITEM 20403-0000

UNCLASSIFIED BORROW (Bridge Abutment Embankments)

LOCATION - STATION TO STATION	CUT (c.y.)	FILL (c.y.)	* BORROW (c.y.)	WASTE (c.y.)
ABUT 1 - 24+83.00 to 25+24.98	0	2246	2808	0
ABUT 2 - 29+23.75 to 30+07.50	299	5947	7060	0
TOTAL	299	8193	9868	0

* 25% Shrinkage Factor applied

NOTE: The quantity shown above is not included in the quantity shown on Sheet 3 for Item 20403-0000.

 \triangle - Revised Item 55401-2000 quantity.

REVISED 04/21/2015

UNITED STATES
DEPARTMENT OF THE INTERIOR
BUREAU OF INDIAN AFFAIRS
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

RIO PUERCO BRIDGE BRIDGE GENERAL NOTES, ESTIMATED QUANTITIES & TABLES

Designed by: cdh

Drawn by: rsh, dc, cdh

Revised by: cdh

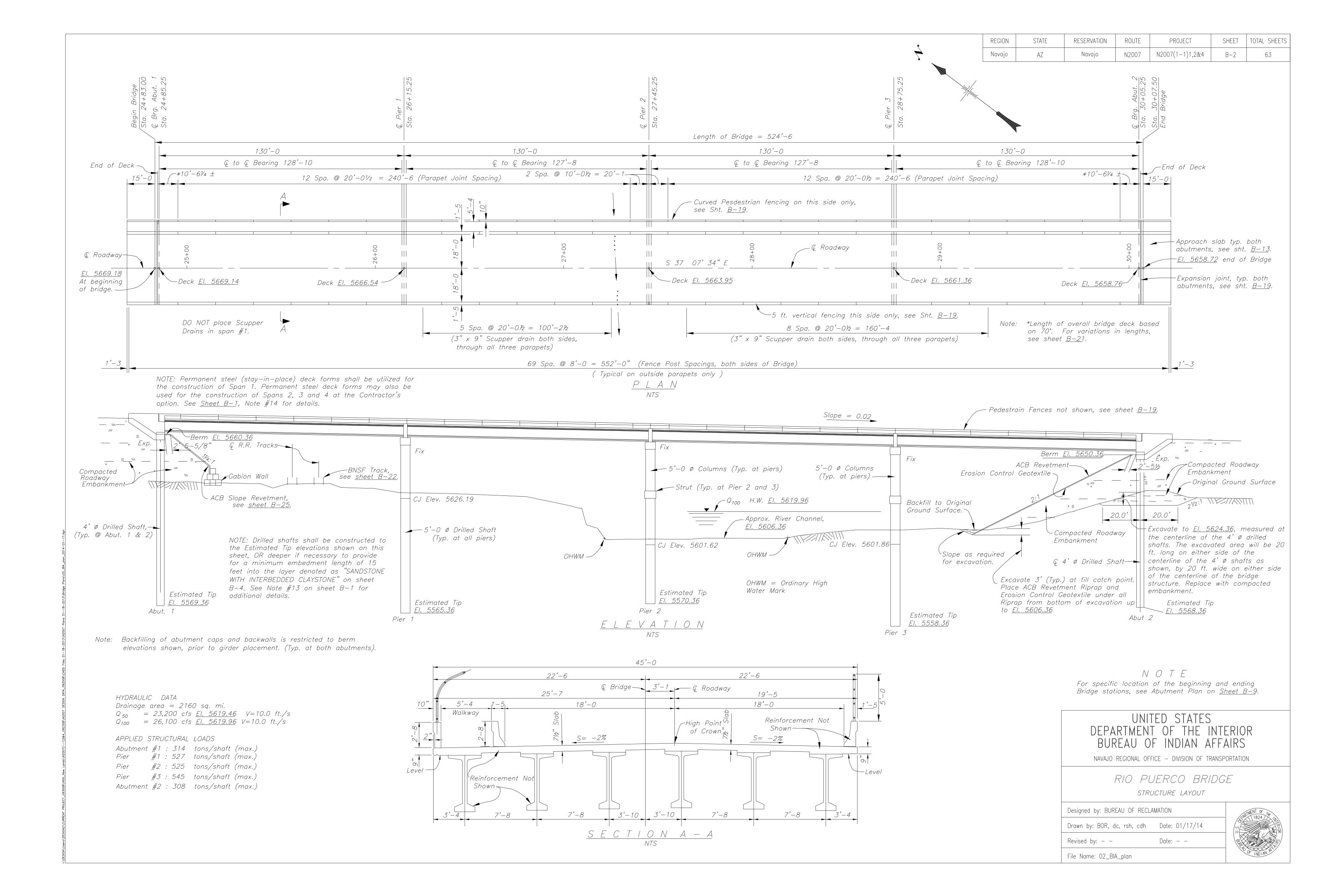
Date: 01/17/14

Revised by: cdh

Date: 04/21/2015

File Name: 01 BlAanrl





WEATHERING

FRESH (W1): Body of rock that is not oxidized or discolored; fracture surfaces are not oxidized or discolored*; no separation of grain boundaries; no change of texture and no solutioning. Hammer rings when crystalline rocks are struck.

SLIGHTLY WEATHERED TO FRESH (W2):**

SLIGHTLY WEATHERED (W3): Discoloration or oxidation is limited to surface of, or short distance from fracture; some feldspar crystals are dull; olivine or pyroxene phenocrysts may be altered to iddingsite; fracture surfaces have minor to complete discoloration or oxidation; no visible separation of grain boundaries; texture preserved and minor leaching of soluble minerals may present. Hammer rings when crystalline rocks are struck, body of rock is not weakened by weathering.

MODERATELY TO SLIGHTLY WEATHERED (W4):**

MODERATELY WEATHERED (W5): Discoloration or oxidation extends from fractured, usually throughout body of rock; ferromagnesian minerals are "rusty". feldspar crystals are "cloudy;" all fracture surfaces are discolored or oxidized; partial opening of grain boundaries visible; texture generally preserved, but soluble minerals may be mostly leached. Hammer does not ring when rock is struck, body of rock is slightly weakened.

INTENSELY TO MODERATELY WEATHERED (W6):**

INTENSELY WEATHERED (W7): Body of rock is discolored or oxidized throughout; all feldspar and ferromagnesian minerals are altered to clay to some extent. All fracture surfaces are discolored or oxidized, surface friable; partial separation of grain boundaries, rock is friable; in situdisaggregation of granitics common in semi-arid regions; texture altered and leaching of soluble minerals may be complete. Rock has dull sound when struck with hammer, rock is weakened, usually can be broken with moderate to heavy manual pressure or by light hammer blow without reference to planes of weakness.

VERY INTENSELY WEATHERED (W8):**

DECOMPOSED (W9): Body of rock is discolored or oxidized throughout, but resistant minerals such as quartz may be unaltered; all feldspar and ferromagnesian minerals are completely altered to clay; complete separation of grain boundaries (disaggregated), partial or complete remnant rock structure may be preserved, but resembles a soil.

NOTE: Weathering categories are established primarily for crystalline rock and those with ferromagnesian minerals, weathering in various sedimentary rocks will not always fit the categories established. The term "weathering" includes all alterations due to any process including surface weathering and hydrothermal alteration.

*Characteristics of fracture surfaces does not include directional weathering along sherars or faults and their associated fracture zones; for example a shear that carries weathering to great depths in a fresh rock mass would not require the whole rock mass to be classified as weathered.

** Combination description are used where equal distribution of both weathering characteristics are present over signification intervals or where characteristics noted are "in between" the diagnostic characteristics.

SOIL CONSISTENCY

Vary soft

Thumb will penetrate soil more than 1 in. (25 mm).

Soft

Thumb will penetrate soil about 1 in. (25 mm).

Firm

Thumb will indent soil about 1/4 in. (5 mm).

Hard

Thumb will not indent soil but readily indented with thumbnail.

Very hard

Thumbnail will not indent soil.

BEDROCK HARDNESS / STRENGTH

EXTREME HARD (H1): Core, fragment or exposure cannot be scratched with knife or sharp pick; can only be chipped with repeated heavy hammer blows.

VERY HARD (H2): Cannot be scratched with knife or sharp pick. Core or fragments breaks with repeated heavy hammer blows.

HARD (H3): Can be scratched with knife or sharp pick with difficulty (heavy pressure). Heavy hammer blow required to break specimen.

MODERATELY HARD (H4): Can be scratched with knife or sharp pick with light or moderate pressure. Core or fragment breaks with moderate hammer blow.

MODERATELY SOFT (H5): *Can be grooved 1/16 in. (2 mm) deep by knife or sharp pick with (moderately or heavy) pressure. Core or fragment breaks with hammer blow or heavy manual pressure.

SOFT (H6): Can be grooved or gouged easily by knife or sharp pick with light pressure, can be scratched with fingernail. Breaks with light to moderate manual pressure.

VERY SOFT (H7): Can be readily indented, grooved or gouged with fingernail, or carved with a knife. Breaks with light pressure.

NOTE: Bedrock units softer than H7, Very Soft, are described using USCS (soils) consistency descriptors.

FRACTURE DENSITY

Alpha— numeric descriptor	Descriptor	Criteria
FDO	Unfractured	No fractures.
FD1	Very slightly fractured	Core recovered mostly in lengths greated than 3 feet (1 m).
FD2	Slightly to ve	ry slightly fractured
FD3	Slightly fractured	Core recovered mostly in lengths from 1 to 3 feet (300 to 1000 mm) with few scattered lengths less than 1 foot (300 mm) or greater than 3 feet (1000 mm).
FD4	Moderately to	slightly fractured
FD5	Moderately fractured	Core recovered mostly in 0.33 to 1.0 foot (100 to 300 mm) lengths with most lengths about 0.67 foot (200 mm)
FD6	Intensity to r	moderately fractured
FD7	Intensity fractured	Lengths average from 0.1 to 0.33 foot (30 to 100 mm) with scattered fragmented intervals. Core recovered mostly in lengths less than 0.33 foot (100 mm).
FD8	Very intensely	to intensely fractured
FD9	Very intensely fractured	Core recovered mostly as chips and fragments with a few scattered short core lengths.

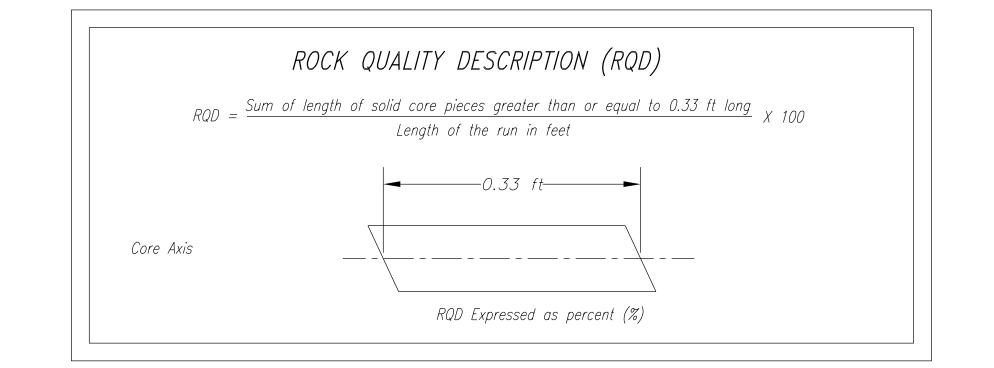
Sedimentary and Pyroclastic rock particle—size descriptors

Size in	SEDI Roundea sub	PYROCLASTIC			
mm (inches)	Particle or fragment	Lithified product	Fragment	Lithified product	
300 (12)	Boulder	BOULDER CONGLOMERATE	Boulder		
256 (10) 75 (3)	Cobble	COBBLE CONGLOMERATE	Cobbler	AGGLOMERATE (Boulder, cobble, gravel, and sand)	
64 (2.5)	Coarse gravel		Coarse gravel		
(0.8)	Fine gravel	— PEBBLE CONGLOMERATE	Fine gravel		
4.75 (0.2) 2.00	Coarse sand		Coarse sand		
(0.08)	Medium sand	SANDSTONE (Coarse sand, medium sand and fine sand)	Medium sand		
0.42 (0.02)	Fine sand		Fine sand	TUFF (Coarse, gravel to fine sand,	
(0.003)	Silt	SILTSTONE/ SHALE		and ash)	
0.005	Clay	CLAYSTONE/ SHALE	Ash		

Rock Color

All colors used to describe rock are taken from the Geological Society of America Rock color Chart (7th pprinting, 1991, with revised text).

Unless indicated all colors are described from wet samples.



UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

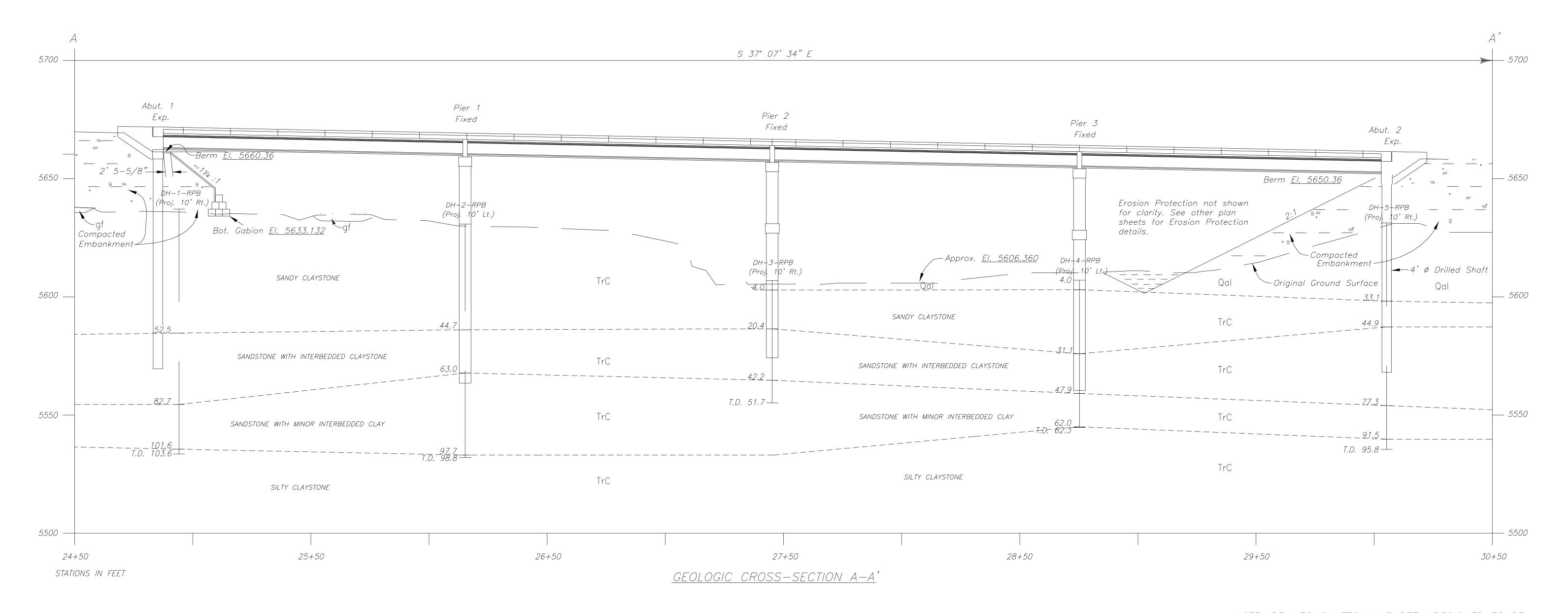
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

RIO PUERCO BRIDGE STANDARD DESCRIPTIONS AND DESCRIPTIVE CRITERIA FOR ROCK

Designed by: BUREAU OF	RECLAMATION
Drawn by: BOR, cdh, rsh	Date: 01/24/13
Revised by:	Date: — —
File Name: 03_BIAsoil	



REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	AZ	Navajo	N2007	N2007(1-1)1,2&4	B-4	63



GENERAL GEOLOGIC LEGEND

- gf <u>General Fill</u>: The railroad tracks have a foundation consisting of poorly graded gravel (GP) that varies in thickness from 1.0 to 3.0 feet. The dirt road is composed of a combination of poorly graded sand (SP) and sandy lean clay s(CL). The roadfill material is the same as the alluvium, but it has been reworked and graded.
- Qal <u>Quaternary Alluvium</u>: In the river channel, alluvium is composed predominately of poorly graded sand (SP) that is generally medium to fine grained, subangular to subrounded, no reaction with HCl, with a trace of subrounded gravel and nonplastic fines. In thicker sections of alluvium (the southern river bank) the poorly graded sand (SP) is interbedded with lenses of sandy lean clay s(CL) and clay seams CL.
- TrC <u>Triassic Chinle Formation</u>: For the purposes of this work, the section of rock encountered on the surface and in drill holes has been divided into four subunits. These units, from top to bottom, consist of a sandy claystone that ranges from 10 to 52 feet thick. A sandstone with interbedded claystone that ranges in thickness from 16 to 32 feet thick. The bottom of this subunit is marked by a pebble conglomerate which ranges in thickness from 1.1 to 3.3 feet. A sandstone with minor interbedded claystone that ranges in thickness from 10 to 35 feet. At the base of the sandstone with minor interbedded clay lies a silty claystone. Since all five drill holes were terminated in or above this unit its thickness is unknown.

GENERAL GEOLOGIC EXPLANATION

Contact between geologic units, dashed where approximate. Contact between geologic subunits, dashed where SANDY CLAYSTONE approximate. SANDSTONE Location of drill hole. ● DH-1-RPB Approximate location of test pit. TP-1-RPB Stick log, dashed where projected to cross-section. DH-3-RPB (Proj. 10' Lt.) — Drill hole number, with distance and direction 4.0 = SANDY CLAYSTONE Typical abbreviation for geologic unit. 20.35 \perp SANDSTONE WITH Typical name.
INTERBEDDED CLAYSTONE — Depth (feet) and contact of geologic unit. 42.2 SANDSTONE — Total depth (feet).

A' Location of geologic cross—section.

GENERAL GEOLOGIC NOTES

For Standard Descriptions and Descriptive Criteria for Rock, see Sheet B-3.

For complete descriptions of test pit and drill holes, see the geologic logs which can be provided upon written request.

Qal was not mapped when found in thicknesses of less than 2.5 feet.

NOTE: Subsurface investigation information has been obtained for designer use only. The Government shall not be responsible for any misinterpretation or assumptions made by the Contractor based on the subsurface investigation information presented on these plans.

NOTE: DRILLED SHAFTS HAVE BEEN DESIGNED TO BE FOUNDED IN THE LAYER DENOTED AS "SANDSTONE WITH INTERBEDDED CLAYSTONE". DRILLED SHAFTS SHALL BE CONSTRUCTED TO THE ESTIMATED TIP ELEVATIONS SHOWN ON THIS SHEET, OR DEEPER IF NECESSARY TO PROVIDE FOR A MINIMUM EMBEDMENT LENGTH OF 15 FEET INTO THE LAYER SPECIFIED LAYER. SEE NOTE #13 ON SHEET B-1 FOR ADDITIONAL DETAILS.

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

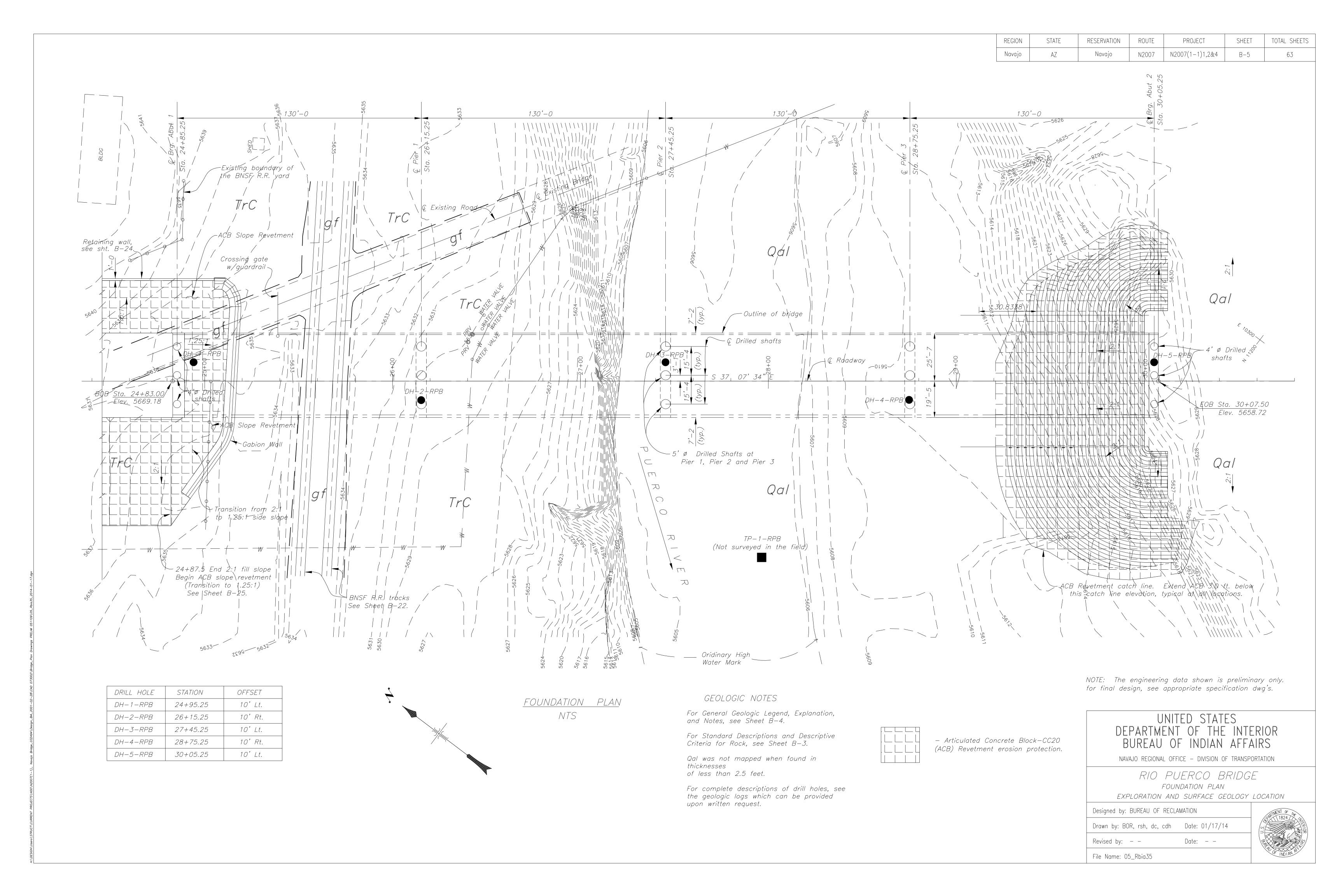
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

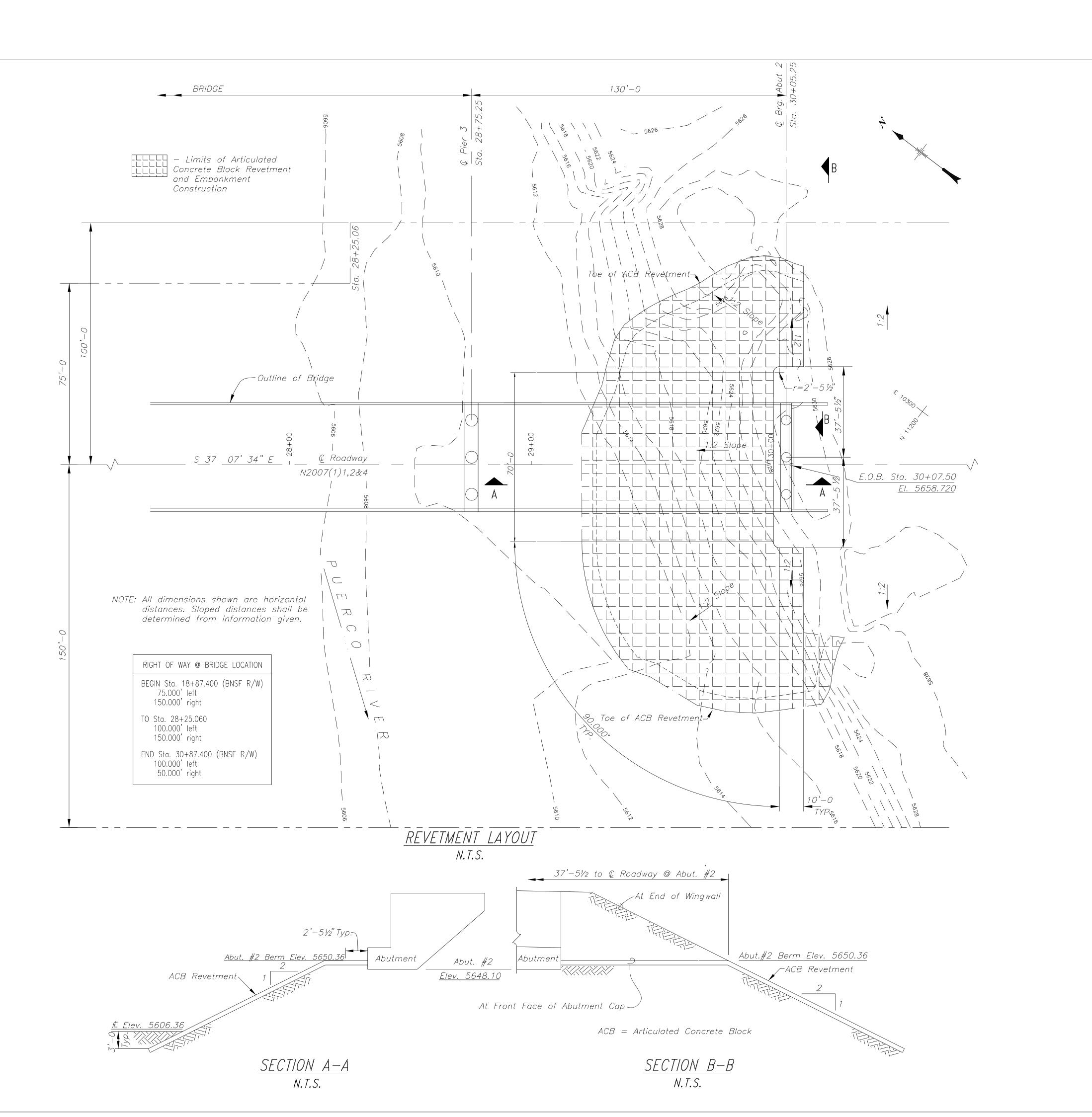
RIO PUERCO BRIDGE SOIL PROFILE SHEET

Designed by: BUREAU OF RECLAN	MATION
Drawn by: BOR, rsh, dc, cdh	Date: 01/17/14
Revised by:	Date:

File Name: 04_BIA0a33034







REGIONSTATERESERVATIONROUTEPROJECTSHEETTOTAL SHEETSNavajoAZNavajoN2007N2007(1-1)1,2&4B-663

EROSION PROTECTION GENERAL NOTES

- 1. Articulated Concrete Block (ACB) Revetment shall conform to Section 251 of the FP-03, all applicable Supplemental Specifications and to the details shown in these plans.
- 2. Embankment construction below ACB Revetment shall conform to Section 204 of the FP-03. Excavation for ACB Revetment shall conform to Section 209 of the FP-03. All embankment above natural ground at abutments is included in the quantity for Item 20403-0000 Unclassified Borrow and shall be paid for under Item 20403-0000, Unclassified Borrow. All excavation for ACB Revetment toe construction shall be considered incidental to Item 25112-3000, ACB Revetment and shall not be measured for payment. Suitable excavated material may be used as embankment material on the project as long as the material conforms to the specifications for embankment construction.
- 3. ACB Revetment shall have Erosion Control Geotextile place below it as shown on this sheet and elsewhere on the plans. Erosion Control Geotextile shall conform to Section 714.01 (a) (4) Type IV—C of the FP—03.
- 4. See sheet B-23 for additional ACB Revetment details. All work involved in the furnishing, fabricating and installation of the ACB Reventment shall be measured and paid for under Item 25112-3000.
- 5. See sheets B-22 through B-25 for ABUTMENT 1 details and installation details.

LOCATION	CUT(c.y.)	FILL(c.y.)	BORROW(c.y.)	WASTE(c.y.)
LEFT ABUT#2	3,525	618	0	2907
CENTER ABUT#2	,			
RIGHT ABUT#2	945	2104	1159	0
TOTAL (c.y.)	4,885	4947	62	0

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

RIO PUERCO BRIDGE

EROSION PROTECTION DETAILS ABUTMENT 2

Designed by: STRUCTURAL UNIT	
Drawn by: rsh, dc, cdh	Date: 05/21/14
Revised by:	Date: — —
File Name: 06_BIAersn	



DEPTH TO WATER DURING DRILLING;

BACKFILLED WITH EXCAVATED MATERIAL.

HOLE TERMINATED AT THE DISCRETION

RELOCATION ON SITE REPRESENTATIVE

10-HOUR SHIFTS

REASON FOR HOLE TERMINATION:

ESTIMATED DRILLING TIME:

OFFICE OF NAVAJO HOPI INDIAN

SET-UP AND DRILLING 2

110—

NA = NOT APPLICABLE

FA = 7-1/2 IN. FLIGHT AUGER

COMMENTS:

NOT DETERMINED.

HOLE COMPLETION;

(O.N.H.I.R.)

STA. 26+15 ABOUT 50 FEET NORTH CONTACT IS CONFORMABLE AND GRADATIONAL, MINOR SANDSTONE LENSES RANGING FROM 0.1 TO 0.5 FT. ARE ENCOUNTERED FROM 2.0 OF THE NORTH BANK OF THE RIVER CHANNEL. TO 5.0 FT. ABOVE THE LOWER CONTACT. 44.7 TO 63.0 FT. SANDSTONE WITH INTERBEDDED CLAYSTONE: DRILL EQIUPMENT: COLOR RANGES FROM GRAYISH PINK (5R 8/2) TO VERY LIGHT GRAY CME 1250 TRACK MOUNTED EARTH (N8) WITH CLAYSTONE INTERBEDS RANGING FROM PALE RED (5R 6/2) AUGER; 5 FT. LONG 7-1/2 INCH TO MODERATE RED (5R 4/6). SANDSTONE IS FINE TO MEDIUM HOLLOW-STEM FLIGHT AUGERS: 9-INCH GRAINED. SUBANGULAR TÓ SUBROUNDED. THINLY BEDDED. SLIGHTLY CARBIDE TIPPED BIT: 5 FT. LONG HQ (W3) TO PREDOMINATELY MODERATELY (W5) WEATHERED, HARD (H3) CORE BARREL WITH SPLIT TUBE INNER BARREL; SURFACE SET, DIAMOND BIT; TO PREDOMINATELY MODERATELY HARD (H4).. MODERATELY FRACTURED AND HQ RODS. (FD5), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 44.7 TO 61.3 FEET, INTENSELY FRACTURED (FD7), WATER TESTING EQUIPMENT; RECOVERED PREDOMINATELY IN LENGTHS FROM 0.1 TO 0.33 FOOT 40— NO WATER TESTS REQUIRED. FROM 61.3 TO 63.0 FEET. CLAYSTONE INTERBEDS ENCOUNTERED FROM 53.7 TO 54.2 FT., 54.9 TO 55.4 FT., AND 58.2 TO 60.4 FT. UPPER CONTACT IS CONFORMABLE AND GRADATIONAL WHILE LOWER J. HAYDEN. CONTACT IS CONFORMABLE AND SHARP. THE LOWER CONTACT IS LOCATED AT THE BASE OF A GRAVEL TO PEBBLE CONGLOMERATE DRILL FLUID: 100 100 FROM 61.3 TO 63.0 FT. THIS CONGLOMERATE IS CLAST SUPPORTED TrCNO DRILL FLUID FROM 0.0 TO 44.7 (90%) WITH A CLAY MATRIX. CLASTS COMPOSED OF INTENSELY (W7) FT.. USED WATER AS DRILL FLUID WEATHERED SANDSTONE; LARGEST CLAST 0.15 FT. . 96 | 66 | FROM 44.7 TO 98.8 FT. 63.0 TO 97.7 FT. SANDSTONE WITH MINOR INTERBEDDED CLAY: DRILL FLUID RETURN: COLOR RANGES FROM VERY LIGHT GRAY (N8) TO DARK GRAY (N3), INTERVAL (FT.) % RETURN 66 FINE TO MEDIUM GRAINED, SUBANGULAR TO SUBROUNDED, LAMINAR 0.0 - 44.7 FA-NA TO THINLY BEDDED, SLIGHTLY WEATHERED (W3), MODERATELY HARD 44.7 - 61.3 95 (H4) TO HARD (H3). MODERATELY FRACTURED (FD5), RECOVERED 61.3 - 86.3 90 PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 63.0 TO *86.3.* – *98.8 95* 66.3 FEET. SLIGHTLY TO VERY SLIGHTLY FRACTURED (FD2). RECOVERED IN LENGTH FROM 1.0 TO MORE THAN 3 FEET FROM 66.3 DRILL FLUID RETURN COLOR: TO 96.3 FEET. UPPER AND LOWER CONTACTS ARE CONFORMABLE AND 70 - 100 | 100 | INTERVAL (FT.) COLOR 0.0 - 44.7 FA-NA **∃** 95 | 95 44.7 - 98.8 REDDISH BROWN 97.7 TO 98.8 FT. SILTY CLAYSTONE: COLOR IS PALE OLIVE (10Y 6/2). MODERATELY TO THICKLY BEDDED, DRILLING METHODS: SLIGHTLY WEATHERED TO FRESH (W2) BUT VERY SOFT (H7). 100 100 INTERVAL METHOD/ MODERATELY FRACTURED (FD5), RECOVERED PREDOMINATELY IN (FT.) BARREL SIZE LENGTHS FROM 0.33 TO 1.0 FOOT FROM 96.3 TO 98.8 FEET. UPPER 0.0 - 44.7 7-1/2 IN. FA CONTACT IS CONFORMABLE AND SHARP WHILE LOWER CONTACT IS □ 100 | 100 | 44.7 – 98.8 HQ CORE PROBABLE REASON FOR CORE LOSS: DRILLING CONDITIONS AND DRILLER'S INTERVAL (FT.) AMOUNT INTERPRETATION COMMENTS: 51.3 - 56.3 0.2 WASHED OUT CLAY THROUGHOUT 0.0 - 20.3 FT. AUGERED SMOOTH; 20.3 TO 44.7 AUGERED ROUGH; AT 56.3 - 61.3 O.1 WASHED OUT CLAY THROUGHOUT □ 100 | 92 44.7 FT. AUGER MET REFUSAL. 61.3 - 66.3 0.3 WASHED OUT CLAY THROUGHOUT 71.3 - 76.3 0.25 WASHED OUT CLAY THROUGHOUT CASINS RECORD (FA): 96.3 - 98.8 0.4 WASHED OUT CLAY THROUGHOUT 5527.7 98.8 - 92 28 5 7 CASING CASING INTERVAL. SIZE DEPTH DRILLED

DRILLER;

7-1/2 IN. 0.0-44.7 0.0-44.7

52.5 - 56.0 3.5 BLOCKED OFF THROUGHOUT 56.0 - 61.0 0.7 WASHED OUT CLAY THROUGHOUT

71.0 - 76.0 0.2 WASHED OUT CLAY THROUGHTOUT

DRILL HOLE DH-1-RPB

PAGE 1 OF 1

7-1/2 IN. 44.7 44.7-98.8

DEPTH TO WATER DURING DRILLING;

REASON FOR HOLE TERMINATION:

ESTIMATED DRILLING TIME:

BACKFILLED WITH EXCAVATED MATERIAL.

HOLE TREMINATED AT THE DISCRETION

10-HOUR SHIFTS

OF OFFICE OF NAVAJO HOPI INDIAN

SET-UP AND DRILLING 2

RELOCATION ON SITE REPRESENTATIVE

NOT DETERMINED.

HOLE COMPLETION;

(O.N.H.I.R.).

NOTE: Subsurface investigation information has been obtained for designer use only. The Government shall not be responsible for any misinterpretation or assumptions made by the Contractor based on the subsurface investigation information presented on these plans.

PAGE 1 OF 1

DRILL HOLE DH-2-RPB

FA = 7-1/2 IN. FLIGHT AUGER

NA = NOT APPLICABLE

STATE RESERVATION ROUTE PROJECT TOTAL SHEETS REGION SHEET N2007(1-1)1,2&4N2007 B-763 Navajo Navajo

GEOLOGIC LOG OF DRILL HOLE DH-3-RPB

FEATURE: RIO PUERCO BRIDGE LOCATION: MID CHANNEL OF RIO PUERCO BEGUN: 8/17/93 FINISHED: 8/18/93 DEPTH AND ELEV. OF WATER LEVEL AND DATE MEASURED: SEE NOTES

PROJECT: O.N.H.I.R. COORDINATES: N. 11444.7 E. 10099.9 TOTAL DEPTH: 51.7 DEPTH TO BEDROCK: 4.0

STATE: ARIZONA GROUND ELEVATION: 5606.96 ANGLE FROM HORIZONTAL AND BEARING: 90° HOLE LOGGED BY: R. LUNG REVIEWED BY:

SHEET 1 OF 1

PURPOSE OF HOLE: DETERMINE FOUNDATION CONDITIONS AT PROPOSED PIER NO. 3 (STA. 27+45.3, OFFSFT 10 FT LT.). PFRFORM STANDARD PENETRATION TESTING (SPTS), COLLECT SOIL SAMPLES FOR LABORATORY ANALYSIS AND DETERMINE DEPTH TO ROCK.

DRILL SITE AND SET-UP: SITE LOCATED ON ORIGINAL GROUND AT STA. 27+45, ABOUT 20 FEET SOUTH OF THE NORTH BANK OF THE RIVER CHANNEL.

DRILL EQIUPMENT: CME 1250 TRACK MOUNTED EARTH AUGER WITH AUTOMATIC PENETRATION TEST HAMMER; 5 FT. LONG 7-1/2 INCH HOLLOW-STEM FLIGHT AUGERS; 9-INCH CARBIDE TIPPED BIT; 1-3/8 IN. I.D. STANDARD SPLIT-SPOON SAMPLER; 5 FT. LONG HQ CORE BARREL WITH SPLIT TUBE INNER BARREL; SURFACE SET, DIAMOND BIT;

AND HQ RODS. WATER TESTING EQUIPMENT; NO WATER TESTS REQUIRED. DRILLER;

DRILL FLUID RETURN:

J. HAYDEN.

DRILL FLUID: NO DRILL FLUID FROM 0.0 TO 20.2 FT., USED WATER AS DRILL FLUID FROM 20.2 TO 51.7 FT.

INTERVAL (FT.) % RETURN 0.0 - 20.2 FA-NA 20.2 - 51.7 90 DRILL FLUID RETURN COLOR:

INTERVAL (FT.) COLOR 0.0 - 20.2 FA-NA 20.2 - 51.7 REDDISH BROWN

INTERVAL METHOD/ (FT.) BARREL SIZE FT. INTERVALS 20.2 – 51.7 HQ CORE

DRILLING CONDITIONS AND DRILLER'S | CLAY: CL/CH; SMOOTH; AT 19.0 FT. AUGERED VERY REACTION TO HCI. HARD. PROBABLY FIRST SIGNIFICANT REFUSAL.

CASINS RECORD (FA): CASING CASING INTERVAL. SIZE DEPTH DRILLED 7-1/2 IN. 0-20.2 0-20.2 7-1/2 IN. 20.2 20.2-51.7 DEPTH TO WATER DURING DRILLING;

NOT DETERMINED. HOLE COMPLETION; BACKFILLED WITH EXCAVATED MATERIAL. REASON FOR HOLE TERMINATION:

OFFICE OF NAVAJO HOPI INDIAN (0.N.H.I.R.) ESTIMATED DRILLING TIME:

100 100 88 100 *−| 100* | *100* o______100 | 90 5550.9 11.8 TO 15.9 FT. SANDY CLAYSTONE: DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS AND

15.9 TO 16.9 FT. CLAYSTONE VISUALLY CLASSIFIED AS A LEAN TO FAT CLAY WITH; SAND (CL/CH): ABOUT 75% FINES WITH HIGH PLASTICITY, MEDIUM TOUGHNESS, SLOW DILATANCY, HIGH DRY STRENGTH; ABOUT 25% FINE TO COARSE, SUBANGULAR TO SUBROUNDED SAND: MAX. SIZE FINE SAND: MODERATE RED; WEAK REACTION TO HCI.

LAB DATA; 74% SAND; 26% FINES; 0% GRAVEL; PI 0%; LL 31%; MC 13.9%; CLAYEY SAND SC.

0.0 - 52.5 7-1/2 IN. FA; SPTS AT 5 | 16.9 TO 20.2 FT. SANDY CLAYSTONE; DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS AND CUTTINGS. 20.2 TO 20.4 FT. CLAYSTONE; VISUALLY CLASSIFIED AS A LEAN TO FAT

ABOUT 95% FINES WITH HIGH PLASTICITY, MEDIUM TOUGHNESS, SLOW 0.0 - 4.0 FT. AUGERED SMOOTH; 4.0 | DILATANCY, HIGH DRY STRENGTH; ABOUT 5% FINE, SUBANGULAR TO TO 19.0 FT. AUGFRFD PRFDOMINATFLY | SUBROUNDED SAND: MAX. SIZE FINE SAND: MODERATE RED: WEAK

SANDSTONE LENSE AT 20.2 FT. AUGER | LAB DATA; 26% SAND; 74% FINES; 0% GRAVEL; PI IS; LL IS; MC 5.5%; LEAN CLAY WITH SAND (CL)s.

20.4 TO 42.2 FT. SANDSTONE WITH INTERBEDDED CLAYSTONE:

COLOR RANGES FROM GRAYISH PINK (5R 8/2) TO VERY LIGHT GRAY (N8) WITH CLAYSTONE INTERBEDS RANGING FROM PALE RED (5R 6/2) TO MODERATE RED (5R 4/6). SANDSTONE IS FINE TO MEDIUM GRAINED, THINLY BEDDED, SLIGHTLY (W3) TO PREDOMINATELY MODERATELY (W5) WEATHERED, AND MODERATELY HARD (H4). VERY INTENSELY FRACTURED (FD9). RECOVERED PREDOMINATELY AS CHIPS AND FRAGMENTS FROM 20.2 TO 21.5 FEET. MODERATELY FRACTURED (FD5), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 21.5 TO 25.4 FEET. INTENSELY FRACTURED (FD7), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.1 TO 0.33 FOOT FROM 25.4 TO 26.4 FEET, HOLE TERMINATED AT THE DISCRETION | MODERATELY FRACTURED (FD5), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 26.4 TO 39.6 FEET, INTENSELY RELOCATION ON SITE REPRESENTATIVE FRACTURED (FD7), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.1 TO 0.33 FOOT FROM 39.6 TO 42.2 FEET. CLAYSTONE INTERBEDS ENCOUNTERED AT; 29.6 TO 29.9 FT., 36.7 TO 36.9 FT., UPPER CONTACT IS CONFORMABLE AND GRADATIONAL WHILE LOWER CONTACT IS CONFORMABLE AND SHARP. THE LOWER CONTACT IS LACATED AT THE

CLASTS COMPOSED OF INTENSELY (W7) WEATHERED SANSTONE; LARGEST

FA = 7-1/2 IN. FLIGHT AUGER PI = PLASTICITY INDEX

NA = NOT APPLICABLE

MC = MOISTURE CONTENT

10-HOUR SHIFTS SET-UP AND DRILLING 2 BASE OF A GRAVEL TO PEBBLE CONGLOMERATE FROM 39.6 TO 42.2 FT. THIS CONGLOMERATE IS CLAST SUPPORTED (90%) WITH A MATRIX.

CLAST 0.15 FT.

COMMENTS:

IS = INSUFFICIENT SAMPLE

LL = LIQUID LIMIT

GEOLOGIC DESCRIPTION

0.0 TO 4.0 FT. QUATERNARY ALLUVIUM (TrC): POORLY GRADED, SUBANGULAR TO SUBROUNDED SAND WITH MINOR AMOUNTS OF FINE GRAVEL. DESCRIPTIONS BASED ON VISUAL ANALYSIS OF CUTTINGS IN AUGERED INTERVALS.

4.0 TO 51.7 FT. TRIASSIC CHINLE FORMATION (TrC):

4.0 TO 20.4 FT. SANDY CLAYSTONE: DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS AND CUTTING IN AUGERED INTERVALS. COLOR RANGES FROM PALE RED (5R 6/2) TO MODERATE RED (5R 4/6). SANDY CLAYSTONE IS THINLY TO MODERATELY BEDDED AND INTENSELY (W7) TO MODERATELY (W5) WEATHERED WITH DEPTH (APPROXIMATELY 10 TO 12 FEET). VERY SOFT (H7) BREAKING WITH LIGHT MANUAL PRESSURE. THE UPPER CONTACT IS UNCONFORMABLE AND SHARP WHILE LOWER CONTACT IS CONFORMABLE AND GRADATIONAL. MINOR SANDSTONE LENSES RANGING FROM 0.1 TO 0.5 FT. ARE ENCOUNTERED FROM 2.0 TO

4.0 TO 5.8 FT. SANDY CLAYSTONE: DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS AND CUTTINGS..

5.0 FT. ABOVE THE LOWER CONTACT.

5.8 TO 6.8 FT. CLAYSTONE VISUALLY CLASSIFIED AS A LEAN CLAY WITH SAND; (CL)s; ABOUT 80% FINES WITH MEDIUM PLASTICITY, SLOW DILATANCY, HIGH TOUGHNESS, HIGH DRY STRENGTH, ABOUT 20% FINE, SUBANGULAR TO SUBROUNDED SAND, MAX. SIZE FINE SAND; MODERATELY RED; WEAK REACTION WITH HCI.

LAB DATA; 45% SAND; 55% FINES; 0% GRAVEL, PI 17%; LL 35%; MC 12.8%; LEAN CLAY TO CLAYEY SAND CL/SC.

6.8 TO 10.8 FT. SANDY CLAYSTONE: DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS AND CUTTINGS...

10.8 TO 11.8 FT. CLAYSTONE VISUALLY CLASSIFIED AS A LEAN ABOUT 95% FINES WITH MEDIUM PLASTICITY, MEDIUM TOUGHNESS, SLOW DILATANCY, HIGH DRY STRENGTH, ABOUT 5% FINE, SUBANGULAR TO SUBROUNDED SAND, MAX. SIZE FINE SAND; MODERATELY RED; WEAK REACTION TO HCI.

LAB DATA; 45% SAND; 55% FINES; 0% GRAVEL, PI 14%; LL 35%; MC 15.9%; LEAN CLAY TO CLAYEY SAND CL/SC.

42.2 TO 51.7 FT. SANDSTONE WITH MINOR INTERBEDDED CLAY: COLOR RANGES FROM VERY LIGHT GRAY (N8) TO DARK GRAY (N3), FINE TO MEDIUM GRAINED, SUBANGULAR TO SUBROUNDED, LAMINAR TO THINLY BEDDED, SLIGHTLY WEATHERED (W3), AND HARD (H3). SLIGHTLY TO VERY SLIGHTLY FRACTURED (FD2), RECOVERÉD IN LENGTHS FROM 1.0 TO MORE THAN 3 FEÉT FROM 42.2 TO 51.7 FEET. UPPER CONTACT IS CONFORMABLE AND SHARP. LOWER CONTACT IS UNKNOWN.

PROBABLE REASON FOR CORE LOSS: INTERVAL (FT.) AMOUNT INTERPRETATION 22.2 - 26.7 0.15 WASHED OUT CLAY THROUGHOUT 36.7 - 39.1 0.30 WASHED OUT CLAY THROUGHOUT

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

DRILL HOLE DH-3-RPB

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

RIO PUERCO BRIDGE BORING LOGS - SHEET 1 OF 2

Designed by: BUREAU OF RECLAMATION Drawn by: BOR, rsh, dc Date: 01/17/14 Revised by: - -Date: - -

PAGE 1 OF 1

File Name: 07_BIAdrillhole1



G10

GEOLOGIC LOG OF DRILL HOLE DH-4-RPB SHEET 1 OF 1 FEATURE: RIO PUERCO BRIDGE PROJECT: O.N.H.I.R. STATE: ARIZONA COORDINATES: N. 11329.25 E. 10162.02 COLLAR ELEVATION: 5607.09 LOCATION: SOUTHERN SIDE OF RIVER CHANNEL BEGUN: 8/21/93 FINISHED: 8/23/93 TOTAL DEPTH: 62.3 ANGLE FROM HORIZONTAL AND BEARING: 90° DEPTH OF ELEV. OF WATER DEPTH TO BEDROCK: 4.0 HOLE LOGGED BY: R. LUNG LEVEL AND DATE MEASURED: SEE NOTES REVIEWED BY: NOTES GEOLOGIC DESCRIPTION 0.0 TO 4.0 FT. QUATERNARY ALLUVIUM (QaL) PURPOSE OF HOLE: DETERMINE FOUNDATION CONDITIONS AT 5598.7 POORLY GRADED, SUBANGULAR TO SUBROUNDED SAND WITH MINOR AMOUNTS OF FINE GRAVEL. DESCRIPTIONS BASED ON VISUAL ANALYSIS PROPOSED PIER NO. 4 (STA. OF CUTTINGS IN AUGERED INTERVALS. 28+75.25. OFFSET 10 FT. RT.). DETERMINE DEPTH TO ROCK AND CORE 4.0 TO 51.7 FT. TRIASSIC CHINLE FORMATION (TrC) 10— FOR 20 FT. 4.0 TO 31.1 FT. SANDY CLAYSTONE: DRILL SITE AND SET-UP: DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS SITE LOCATED ON ORIGINAL GROUND AT AND CUTTING IN AUGERED INTERVALS. COLOR RANGES FROM PALE STA. 28+75 ABOUT 130 FEET SOUTH RED (5R 6/2) TO MODERATE RED (5R 4/6). SANDY CLAYSTONE IS OF THE NORTH BANK OF THE RIVER THINLY TO MODERATELY BEDDED AND INTENSELY (W7) TO MODERATELY CHANNEL. (W5) WEATHERED WITH DEPTH (APPROXIMATELY 10 TO 12 FEET), VERY SOFT (H7) BREAKING WITH LIGHT MANUAL PRESSURE. THE DRILL EQUIPMENT: UPPER CONTACT IS UNCONFORMABLE AND SHARP WHILE LOWER CME 1250 TRACK MOUNTED EARTH CONTACT IS CONFORMABLE AND GRADATIONAL. MINOR SANDSTONE AUGER; 5 FT. LONG 7-1/2 INCH LENSES RANGING FROM 0.1 TO 0.5 FT. ARE ENCOUNTERED FROM 2.0 HOLLOW-STEM FLIGHT AUGERS; 9-INCH TO 5.0 FT. ABOVE THE LOWER CONTACT. CARBIDE TIPPED BIT: 5 FT. LONG HQ CORE BARREL WITH SPLIT TUBE INNER 31.1 TO 47.9 FT. SANDSTONE WITH INTERBEDDED CLAYSTONE: BARREL; SURFACE SET, DIAMOND BIT; COLOR RANGES FROM GRAYISH PINK (5R 8/2) TO VERY LIGHT GRAY AND HQ RODS. (N8) WITH CLAYSTONE INTERBEDS RANGING FROM PALE RED (5R 6/2) TO MODERATE RED (5R 4/6). SANDSTONE IS FINE TO MEDIUM WATER TESTING EQUIPMENT; 40 99 GRAINED, SUBANGULAR TO SUBROUNDED, THINLY BEDDED, SLIGHTLY NO WATER TESTS REQUIRED. (W3) TO MOSTLY MODERATELY (W5) WEATHERED, AND MODERATELY DRILLER; HARD (H4). MODERATELY FRACTURED (FD5), RECOVERED J. HAYDEN. PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 31.1 TO 44.6 FEET..INTENSELY FRACTURED (FD7), RECOVERED PREDOMINATELY DRILL FLUID: IN LENGTHS FROM 0.1 TO 0.33 FOOT FROM 44.6 TO 47.9 FEET. 50 - | 100 | 78 | . NO DRILL FLUID FROM 0.0 TO 31.1 CLAYSTONE INTERBEDS ENCOUNTERED FROM 34.6 TO 34.9 FT., 40.3 FT., USED WATER AS DRILL FLUID TO 41.3 FT., UPPER CONTACT IS CONFORMABLE AND GRADATIONAL FROM 31.1 TO 62.3 FT. WHILE LOWER CONTACT IS CONFORMABLE AND SHARP. THE LOWER 100 | 100 | CONTACT IS LOCATED AT THE BASE OF A GRAVEL TO PEBBLE DRILL FLUID RETURN: CONGLOMERATE FROM 44.6 TO 47.9 FT. THIS CONGLOMERATE IS INTERVAL (FT.) % RETURN CLAST SUPPORTED (90%) WITH A CLAY MATRIX. CLASTS COMPOSED 60 100 93 0.0 – 31.1 FA-NA 5540.4 62.3 OF INTENSELY (W7) WEATHERED SANDSTONE; LARGEST CLAST 0.15 FT 31.1 - 62.3 95 47.9 TO 62.0 FT. SANDSTONE WITH MINOR INTERBEDDED CLAY: DRILL FLUID RETURN COLOR: COLOR RANGES FROM VERY LIGHT GRAY (N8) TO DARK GRAY (N3), INTERVAL (FT.) COLOR FINE TO MEDIUM GRAINED, SUBANGULAR TO SUBROUNDED, LAMINAR 0.0 - 31.1 FA-NA TO THINLY BEDDED. SLIGHTLY WEATHERED (W3). HARD (H3) 70 — 31.1 - 62.3 REDDISH BROWN MODERATELY FRACTURED (FD5). RECOVERED PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 47.9 TO 52.3 FEET. DRILLING METHODS: SLIGHTLY TO VERY SLIGHTLY FRACTURED (FD2), RECOVERED IN INTERVAL METHOD/ LENGTHS FROM 1.0 TO MORE THAN 3 FEET FROM 52.3 TO 62.3 (FT.) BARREL SIZE FEET. UPPER AND LOWER CONTACTS ARE CONFORMABLE AND SHARP. 0.0 - 31.1 7-1/2 IN. FA 31.1 – 62.3 HQ CORE 62.0 TO 62.3 FT. SILTY CLAYSTONE: COLOR IS PALE OLIVE (10Y 6/2). MODERATELY TO THICKLY BEDDED, DRILLING CONDITIONS AND DRILLER'S SLIGHTLY WEATHERED TO FRESH (W2) BUT VERY SOFT (H7). UPPER CONTACT IS CONFORMABLE AND SHARP WHILE LOWER CONTACT IS 0.0 - 4.0 FT. AUGERED SMOOTH; 4.0 TO 31.1 AUGERED MOSTLY SMOOTH: AT 90— 31.1 FT. AUGER REFUSAL. PROBABLE REASON FOR CORE LOSS: INTERVAL (FT.) AMOUNT INTERPRETATION CASING RECORD (FA): 31.1 - 37.3 O.8 WASHED OUT CLAY THROUGHOUT CASING CASING INTERVAL. 37.3 - 42.3 0.05 WASHED OUT CLAY THROUGHOUT SIZE DEPTH DRILLED 42.3 - 47.3 0.35 WASHED OUT CLAY THROUGHOUT 7-1/2 IN. 0.0-31.1 0.0-31.1 100 — 7-1/2 IN. 31.1 31.1-62.3 DEPTH TO WATER DURING DRILLING; NOT DETERMINED. HOLE COMPLETION; BACKFILLED WITH EXCAVATED MATERIAL. FA = 7-1/2 IN. FLIGHT AUGER REASON FOR HOLE TERMINATION: NA = NOT APPLICABLEHOLE TREMINATED AT THE DISCRETION OF OFFICE OF NAVAJO HOPI INDIAN RELOCATION ON SITE REPRESENTATIVE (O.N.H.I.R.). ESTIMATED DRILLING TIME:

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DRILL HOLE DH-4-RPB

10-HOUR SHIFTS

SET-UP AND DRILLING

GEOLOGIC LOG OF DRILL HOLE DH-5-RPB SHEET 1 OF 1 FEATURE: RIO PUERCO BRIDGE PROJECT: O.N.H.I.R. STATE: ARIZONA COORDINATES: N. 11237.58 E. 10256.43 GROUND ELEVATION: 5631.36 LOCATION: SOUTH BANK OF THE RIO PUERCO BEGUN: 8/19/93 FINISHED: 8/21/93 TOTAL DEPTH: 95.8 ANGLE FROM HORIZONTAL AND BEARING: 90° DEPTH AND ELEV. OF WATER DEPTH TO BEDROCK: 46.0 HOLE LOGGED BY: R. LUNG LEVEL AND DATE MEASURED: SEE NOTES REVIEWED BY: CLASSIFICATION AND NOTES PHYSICAL CONDITION PURPOSE OF HOLE: 0.0 TO 33.1 FT. QUATERNARY ALLUVIUM (QaL): DETERMINE FOUNDATION CONDITIONS AT POORLY GRADED. SUBANGULAR TO SUBROUNDED SAND WITH 100 MINOR AMOUNTS OF FINE GRAVEL AND SILT. INTERBEDED WITH PROPOSED PIER NO. 5 (STA. 30+05.3, SANDY LEAN CLAY BEDS AND SEAMS. DESCRIPTIONS BASED ON OFFSET 10 FT. LT.), PERFORM VISUAL AND LAB ANALYSIS OF SPT SAMPLES OR DRILLING STANDARD PENETRATION TESTING 100 SP 3 4 17 CONDITIONS AND VISUAL ANALYSIS OF CUTTINGS IN AUGERED (SPTs), COLLECT SOIL SAMPLES FOR LABORATORY ANALYSIS AND DETERMINE DEPTH TO ROCK. 0.0 TO 4.5 FT. POORLY GRADED, SUBANGULAR TO SUBROUNDED QaL DRILL SITE AND SET-UP: SAND WITH A TRACE OF FINE GRAVEL. DESCRIPTIONS BASED ON SITE LOCATED ON ORIGINAL GROUND AT VISUAL ANALYSIS OF CUTTINGS IN AUGERED INTERVALS. s(CL) 14 7 31 STA. 30+05, 10.0 LT. ABOUT 10 FEET SOUTH OF THE SOUTH BANK OF THE 4.5 TO 5.5 FT. SILTY SAND (SM): ABOUT 75% COARSE TO FINE. SUBANGULAR TO SUBROUNDED RIVER CHANNEL. SAND; ABOUT 25% FINES WITH LOW PLASTICITY. LOW TOUGHNESS, CL 22 8 29 RAPID DILATANCY, NO DRY STRENGTH; MAX. SIZE COARSE SAND; DRILL EQIUPMENT: CME 1250 TRACK MOUNTED DRILL RIG LIGHT BROWN; STRONG REACTION WITH HCI. WITH AUTOMATIC PENETRATION TEST SP-SM 21 8 14 HAMMER: 5 FT. LONG 7-1/2 INCH LAB DATA; 51% SAND; 49% FINES; 0% GRAVEL; PI HP; LL NT; MC HOLLOW-STEM FLIGHT AUGERS; 9-INCH 4.2%, POORLY GRADED SAND (SP/GP). * * * CARBIDE TIPPED BIT; 1-3/8 IN. I.D. STANDARD SPLIT-SPOON SAMPLER; 5 5.5 TO 9.5 FT. POORLY GRADED, SUBANGULAR TO SUBROUNDED FT. LONG HQ CORE BARREL WITH SPLI CL-CH * * SAND WITH MINOR AMOUNTS OF FINE GRAVEL AND SILT. TUBE INNER BARREL; SURFACE SET, DESCRIPTION BASED ON VISUAL ANALYSIS OF CUTTINGS IN DIAMOND BIT; AND HQ RODS. AUGERED INTERVALS. * * * 9.5 TO 10.5 FT. POORLY GRADED SAND (SP): ABOUT 100% WATER TESTING EQUIPMENT: MEDIUM TO FINE, SUBANGULAR TO SUBROUNDED SAND, TRACE OF NO WATER TESTS REQUIRED. 100 | 100 | NONPLASTIC FINES; MAX. SIZE MEDIUM GRAINED SAND; LIGHT BROWN; NO REACTION WITH HCI. DRILLER; J. HAYDEN. 100 | 100 LAB DATA; 94% SAND; 6% FINES; 0% GRAVEL; PI NP; LL NT; MC DRILL FLUID: 3.4%, POORLY GRADED SAND (SP-SM). NO DRILL FLUID FROM 0.0 TO 46.0 FT., USED WATER AS DRILL FLUID 10.5 TO 14.5 FT. POORLY GRADED, SUBANGULAR TO FROM 46.0 TO 95.8 FT. SUBROUNDED SAND WITH MINOR AMOUNTS OF FINE GRAVEL. DESCRIPTION BASED ON VISUAL ANALYSIS OF CUTTINGS IN DRILL FLUID RETURN: 100 100 AUGERED INTERVALS. INTERVAL (FT.) % RETURN 0.0 - 46.0 FA-NA 14.5 TO 15.5 FT. POORLY GRADED SAND (SP): ABOUT 100% 46.0 - 50.8 90 MEDIUM TO FINE, SUBANGULAR TO SUBROUNDED SAND, TRACE OF *50.8 – 95.8 95* NONPLASTIC FINES; MAX. SIZE MEDIUM GRAINED SAND; LIGHT DRILL FLUID RETURN COLOR: 95 95 BROWN; NO REACTION WITH HCI (CLAY LENSE PRESENT IN INTERVAL (FT.) COLOR 0.0 - 46.0 FA-NA LAB DATA: 74% SAND: 26% FINES: 0% GRAVEL: PI NP: LL NT: MC 46.0 - 95.8 REDDISH BROWN 3.9%, SILTY SAND (SM). DRILLING METHODS: INTERVAL METHOD/ (FT.) BARREL SIZE 15.5 TO 19.5 FT. POORLY GRADED, SUBANGULAR TO 100 | 100 0.0 - 46.0 7-1/2 IN. FA; SPTs AT 5 SUBROUNDED SAND WITH MINOR AMOUNTS OF FINE GRAVEL. FT. INTFRVALS DESCRIPTIONS BASED ON VISUAL ANALYSIS OF CUTTINGS IN 46.0 - 95.8 HQ CORE 100 100 AUGERED INTERVALS. DRILLING CONDITIONS AND DRILLER'S 19.5 TO 20.5 FT. SANDY LEAN CLAY (CL): COMMENTS: ABOUT 50% FINES WITH HIGH PLASTICTY, HIGH TOUGHNESS. NO 100 100 3 0.0 - 33.1 FT. AUGERED SMOOTH; DILATANCY, HIGH DRY STRENGTH; ABOUT 40% MEDIUM GRAINED. 33.1 TO 46.0 FT. AUGERED SUNROUNDED TO ROUNDED SAND; MAX. SIZE MEDIUM GRAINED BOTTOM OF HOLE PREDOMINATELY SMOOTH; AT 45.0 FT. SAND; MODERATE BROWN; WEAK REACTION WITH HCI. AUGERED VERY HARD, PROBABLY FIRST SIGNIFICANT SANDSTONE LENSE AT 46.0 FT. AUGER REFUSAL. COMMENTS: CASINS RECORD (FA): FA = 7-1/2 IN. FLIGHT AUGER PI = PLASTICITY INDEXCASING CASING INTERVAL. LL = LIQUID LIMITMC = MOISTURE CONTENT SIZE DEPTH DRILLED NP = NON PLASTICNT = NOT TESTED7-1/2 IN. 0-46.0 0-46.0 SPT = STANDARD PENETRATION TESTING NA = NOT APPLICABLE 7-1/2 IN. 46.0 46.0-95.8 DEPTH TO WATER DURING DRILLING; * MOIST CONT (INP) = 34.5 TO 34.9 12.3 * SPT = 34.5 TO 34.9 94/0.4 NOT DETERMINED. 39.5 TO 40.0 11.4 *39.5 TO 40.0 76/0.5* 44.5 TO 44.9 13.1 44.5 TO 44.9 74/0.4 HOLE COMPLETION; BACKFILLED WITH EXCAVATED MATERIAL. * % CORE RECOVERY = 34.0 TO 34.9 100 * USCS = 34.0 TO 34.9 CL/CH REASON FOR HOLE TERMINATION: 44.0 TO 44.9 100 44.0 TO 44.9 CL/CH HOLE TERMINATED AT THE DISCRETION OFFICE OF NAVAJO HOPI INDIAN RELOCATION ON SITE REPRESENTATIVE (O.N.H.I.R.) ESTIMATED DRILLING TIME: 10-HOUR SHIFTS SET-UP AND DRILLING

NOTE: Subsurface investigation information has been obtained for designer use only. The Government shall not be responsible for any misinterpretation or assumptions made by the Contractor based on the subsurface investigation information presented on these plans.

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DRILL HOLE DH-5-RPB

GEOLOGIC LOG OF DRILL HOLE DH-5-RPB

FEATURE: RIO PUERCO BRIDGE
LOCATION: SOUTH BANK OF THE RIO PUERCO
BEGUN: 8/19/93 FINISHED: 8/21/93
DEPTH AND ELEV. OF WATER
LEVEL AND DATE MEASURED: SEE NOTES

G12

PROJECT: O.N.H.I.R. COORDINATES: N. 11237.58 E. 10256.43 TOTAL DEPTH: 95.8 DEPTH TO BEDROCK: 46.0 STATE: ARIZONA GROUND ELEVATION: 5631.36 ANGLE FROM HORIZONTAL AND BEARING: 90° HOLE LOGGED BY: R. LUNG REVIEWED BY:

G12

SHEET 1 OF 1

CLASSIFICATION AND PHYSICAL CONDITION

LAB DATA; 19% SAND; 81% FINES; 0% GRAVEL; PI 18%; LL 32%; MC 14.7%; LEAN CLAY WITH SAND (CL)s.

20.5 TO 24.5 FT. CLAY WITH A TRACE OF FINE GRAINED SAND SILT. DESCRIPTION BASED ON VISUAL ANALYSIS OF CUTTINGS IN AUGERED INTERVALS.

24.5 TO 25.5 FT. LEAN CLAY (CL):

ABOUT 90% FINES WITH HIGH PLASTICITY, HIGH TOUGHNESS. NO
DILATANCY, HIGH DRY STRENGTH; ABOUT 10% FINE. SUBANGULAR
TO SUBROUNDED SAND; MAX. SIZE MEDIUM SAND; MODERATE
BROWN; NO TO STRONG REACTION WITH HCI.

LAB DATA; 25% SAND; 75% FINES; 0% GRAVEL; PI 16%; LL 30%; MC 22.8%, LEAN CLAY WITH SAND (CL).

25.5 TO 27.0 FT. CLAY WITH A TRACE OF FINE GRAINED SAND.
DESCRIPTION BASED ON VISUAL ANALYSIS OF CUTTINGS IN
AUGFRED INTERVALS.

27.0 TO 29.5 FT. POORLY GRADED SUBANGULAR TO SUBROUNDED SAND WITH MINOR AMOUNTS OF FINE GRAVEL. DESCRIPTIONS BASED ON VISUAL ANALYSIS OF CUTTINGS IN AUGERED INTERVALS.

29.5 TO 30.5 FT. POORLY GRADED SAND WITH CLAY (SP-SM):
ABOUT 90% COARSE TO FINE, SUBANGULAR TO SUBROUNDED
SAND; ABOUT 10% FINES WITH MEDIUM PLASTICITY, MEDIUM
TOUGHNESS, NO DILATANCY, MEDIUM DRY STRENGTH; MAX. SIZE
COARSE SAND; LIGHT BROWN; NO TO WEAK REACTION WITH HCI.

LAB DATA; 67% SAND; 13% FINES; 0% GRAVEL; PI NP; LL NT; MC 21.6%, SILTY SAND (SM).

30.5 TO 33.1 FT. POORLY GRADED, SUBANGULAR TO SUBROUNDED SAND WITH MINOR AMOUNTS OF FINE GRAVEL. DESCRIPTIONS BASED ON VISUAL ANALYSIS OF CUTTINGS IN AUGERED INTERVALS.

33.1 TO 95.6 FT. TRIASSIC CHINLE FORMATION (TrC):

33.1 TO 44.0 FT. SANDY CLAYSTONE: DESCRIPTION BASED ON VISUAL ANALYSIS OF DRILLING CONDITIONS AND CUTTINGS IN AUGERED INTERVALS. COLOR RANGES FROM PALE RED (5R 6/2) TO MODERATE RED (5R 4/6). SANDY CLAYSTONE IS THINLY TO MODERATELY BEDDED AND INTENSELY (W7) TO MODERATELY (W5) WEATHERED WITH DEPTH (APPROXIMATELY 10 TO 12 FEET). VERY SOFT (M7) BREAKING WITH LIGHT MANUAL PRESSURE. THE UPPER CONTACT IS UNCONFORMABLE AND SHARP WHILE LOWER CONTACT IS CONFORMABLE AND GRADATIONAL. MINOR SANDSTONE LENSES RANGING FROM 0.1 TO 0.5 FT. ARE ENCOUNTERED FROM 2.0 TO 5.0 FT. ABOVE THE LOWER CONTACT.

34.5 TO 34.9 FT. CLAYSTONE VISUALLY CLASSIFIED AS A LEAN TO FAT CLAY CL/CM; ABOUT 85% FINES WITH HIGH PLASTICITY, HIGH TOUGHNESS. NO DILATANCY, HIGH DRY STRENGTH; ABOUT 15% FINE. SUBANGULAR TO SUBROUNDED SAND; MAX. SIZE FINE SAND; MODERATE RED; NO REACTION WITH HCI.

LAB DATA; 45% SAND; 55% FINES; 0% GRAVEL; PI 10%; LL 30%; MC 12.3%, LEAN CLAY TO CLAYEY SAND CL/SC.

34.9 TO 39.5 FT. SANDY CLAYSTONE; DESCRIPTION BASED ON DRILLING CONDITIONS AND CUTTINGS.

39.5 TO 40.0 FT. CLAYSTONE VISUALLY CLASSIFIED AS A LEAN TO FAT CLAY CL/CH; ABOUT 85% FINES WITH HIGH PLASTICITY, HIGH TOUGHNESS. NO DILATANCY, HIGH DRY STRENGTH; ABOUT 15% FINE. SUBANGULAR TO SUBROUNDED SAND; MAX. SIZE FINE SAND; MODERATE RED; NO REACTION WITH HCI.

LAB DATA; 38% SAND; 62% FINES; 0% GRAVEL; PI 7%; LL 29%; MC 11.4%, SANDY CLAYEY SILT s(CL-ML).

40.0 TO 44.0 FT. SANDY CLAYSTONE; DESCRIPTION BASED ON DRILLING CONDITIONS AND CUTTINGS.

CLASSIFICATION AND PHYSICAL CONDITION

44.0 TO 44.9 FT. CLAYSTONE VISUALLY CLASSIFIED AS A SANDY LEAN TO FAT CLAY: s(CL-CH); ABOUT 60% FINES WITH HIGH PLASTICITY, HIGH TOUGHNESS, NO DILATANCY, HIGH DRY STRENGTH; ABOUT 40% MEDIUM GRAINED. SUBROUNDED TO ROUNDED SAND; MAX. SIZE MEDIUM GRAINED SAND; MODERATE RED; WEAK REACTION WITH HCI.

LAB DATA; 73% SAND; 27% FINES; 0% GRAVEL; PI NP; LL NT; MC 13.1%, SANDY SILT SM.

44.9 TO 77.3 FT. SANDSTONE WITH INTERBEDDED CLAYSTONE: COLOR RANGES FROM GRAYISH PINK (5R 6/2) TO VERY LIGHT GRAY (N8) WITH CLAYSTONE INTERBEDS RANGING FROM PALE RED (5R 6/2) TO MODERATE RED (5R 4/2). FINE TO MEDIUM GRAINED. THINLY BEDDED, SLIGHTLY (W3) TO PREDOMINATELY MODERATELY (W5) WEATHERED; HARD (H3) TO PREDOMINATELY MODERATELY HARD (H4). MODERATELY FRACTURED (FD3), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 46.0 TO 46.6 FEET. INTENSELY FRACTURED (FD7), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.1 TO 0.33 FOOT FROM 46.6 TO 51.6 FEET. MODERATELY FRACTURED (FD5). RECOVERED PREDOMINATELY IN LENGTHS FROM 0.33 TO 1.0 FOOT FROM 51.6 TO 75.1 FEET INTENSELY FRACTURED (FD7), RECOVERED PREDOMINATELY IN LENGTHS FROM 0.1 TO 0.33 FOOT FROM 75.1 TO 77.3 FEET. CLAYSTONE INTERBEDS ENCOUNTERED FROM; 47.3 TO 47.6 FEET., 51.6 TO 54.0 FT., 55.6 TO 56.1 FT., 57.6 TO 58.0 FT., 61.9 TO 62.4 FT., 65.4 TO 66.6 FT., 67.6 TO 69.4 FT., AND 73.4 TO 75.1 FT., FROM 48.6 TO 51.6 FT., GRAVEL TO PEBBLE CONGLOMERATE. CLAST SUPPORTED (90%) WITH A CLAY MATRIX. CLASTS COMPOSED OF INTENSELY WEATHERED SANDSTONE. CONGLOMERATE HAS A STRONG REACTION WITH HCI. UPPER CONTACT IS CONFORMABLE AND GRADATIONAL WHILE LOWER CONTACT IS CONFORMABLE AND SHARP. THE LOWER CONTACT IS INDICATED BY A GRAVEL TO PEBBLE CONGLOMERATE FROM 75.8 TO 77.3 FT. THIS CONGLOMERATE IS CLAST SUPPORTED (90%) WITH A CLAY MATRIX, CLASTS COMPOSED OF INTENSELY (W7) WEATHERED SANDSTONE; LARGEST CLAST 0.15 FT.

77.3 TO 91.5 FT. SANDSTONE WITH MINOR INTERBEDDED CLAY:
COLOR RANGES FROM VERY LIGHT GRAY (N8) TO DARK GRAY
(N3). FINE TO MEDIUM GRAINED, SUBANGULAR TO SUBROUNDED.
LAMINAR TO THINLY BEDDED; SLIGHTLY WEATHERED (W3);
MODERATELY HARD (H4) TO HARD (H3). SLIGHTLY TO VERY
SLIGHTLY FRACTURED (FD2), RECOVERED IN LENGTHS FROM 1.0
TO MORE THAN 3 FEET FROM 77.3 TO 90.3 FEET. INTENSELY
FRACTURED (FD7), RECOVERED PREDOMINATELY IN LENGTHS FROM
0.1 TO 0.33 FOOT FROM 90.3 TO 91.5 FEET. UPPER CONTACT IS
CONFORMABLE AND SHARP, LOWER IS CONFORMABLE AND SHARP.

91.5 TO 95.8 FT. SILTY CLAYSTONE:

COLOR IS PALE OLIVE (10Y 6/2). MODERATELY TO THICKLY

BEDDED. SLIGHTLY WEATHERED TO FRESH (W2) BUT VERY SOFT

(H7). SLIGHTLY FRACTURED (FD3). RECOVERED PREDOMINATELY IN

LENGTHS FROM 1 TO 3 FEET FROM 91.5 TO 95.8 FEET. UPPER

CONTACT IS CONFORMABLE AND SHARP WHILE LOWER CONTACT IS

PROBABLE REASON FOR CORE LOSS:

UNKNOWN.

INTERVAL (FT.) AMOUNT INTERPRETATION

55.8 - 60.8 0.2 WASHED OUT CLAY THROUGHOUT

65.8 - 70.8 0.1 WASHED OUT CLAY THROUGHOUT

70.8 - 75.8 0.25 WASHED OUT CLAY THROUGHOUT

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DRILL HOLE DH-5-RPB

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE – DIVISION OF TRANSPORTATION

RIO PUERCO BRIDGE BORING LOGS - SHEET 2 OF 2

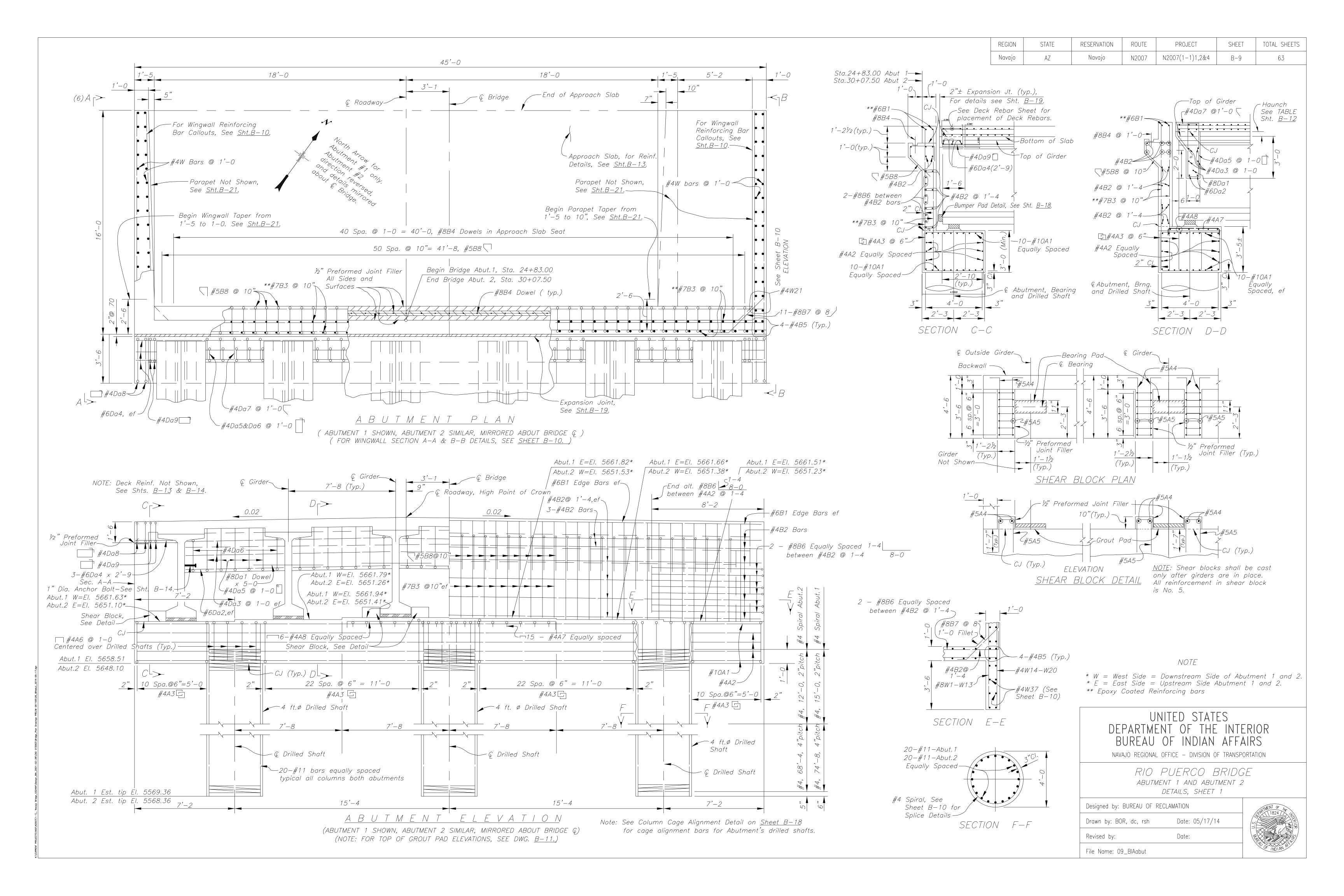
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Drawn by: BOR, rsh, dc

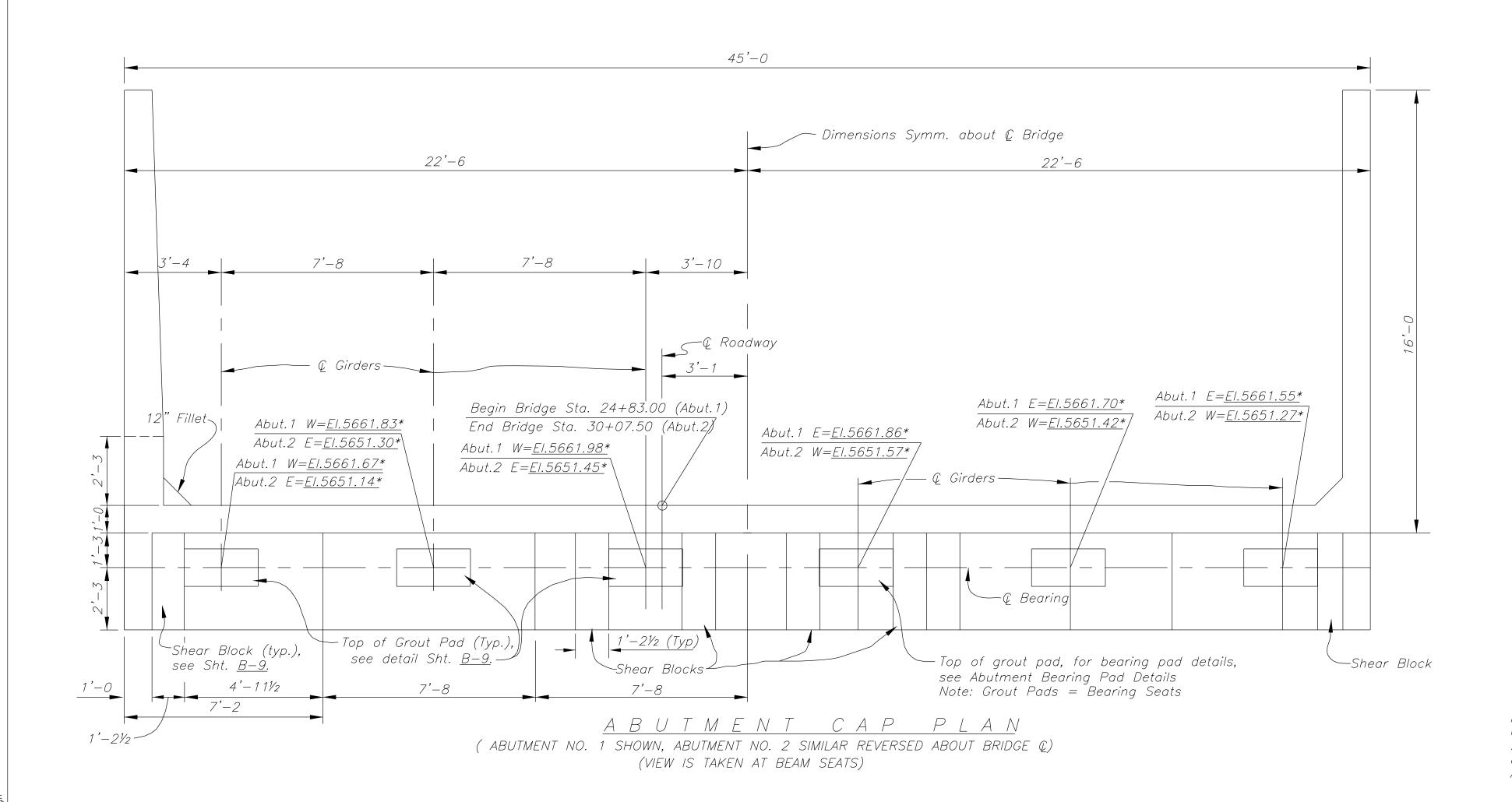
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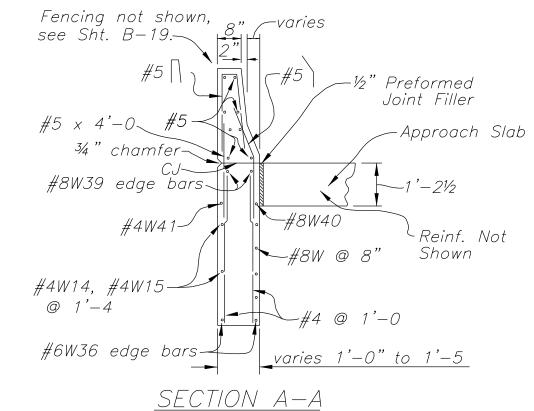
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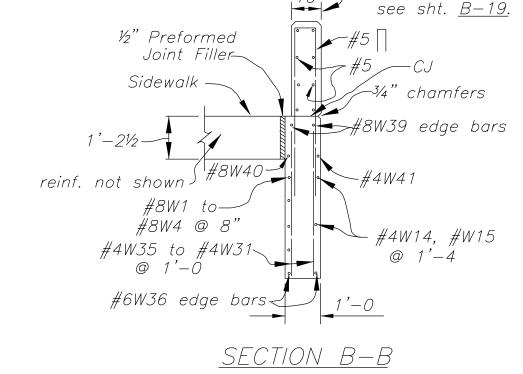








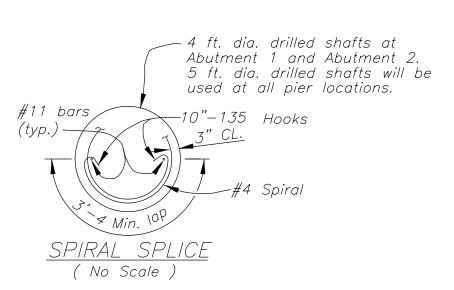
(West Side Wingwall, Abutment 1 and Abutment 2)



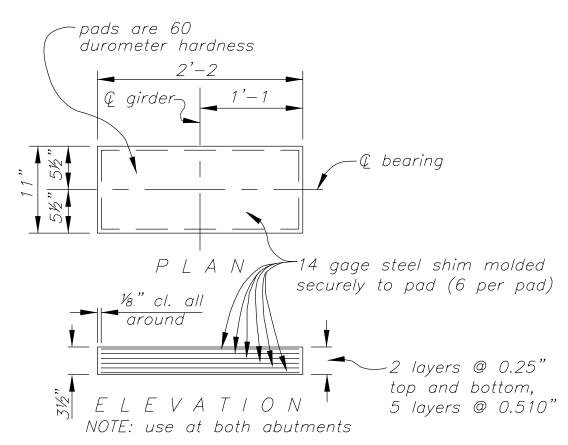
(East Side Wingwall, Abutment 1 and Abutment 2)

—Curved Fencing

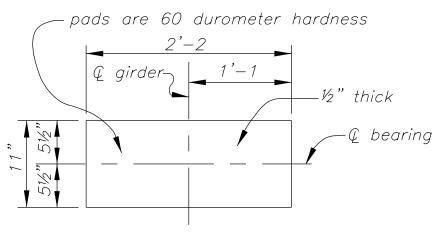
not shown



NOTE: Spiral reinf. shall conform to AASHTO M31, Grade 60 ($\frac{1}{2}$ " \emptyset) or AASHTO M32 (W20 smooth). Splices shall be as detailed. Hooks may be field bent and shall go around vert. #11 bars as shown. Add $1\frac{1}{2}$ turns of spiral and 135° hook with 10" tail at top and bottom of spiral length.



ABUTMENT BEARING PAD DETAIL (12 REQUIRED) No Scale



P L A N NOTE: use at all piers PIER BEARING PAD DETAIL (36 REQUIRED) No Scale

NOTE

* $W = West \ Side = Downstream \ side \ of \ abutment \ 1 \ and \ 2$ * $E = East \ Side = Upstream \ side \ of \ abutment \ 1 \ and \ 2$

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

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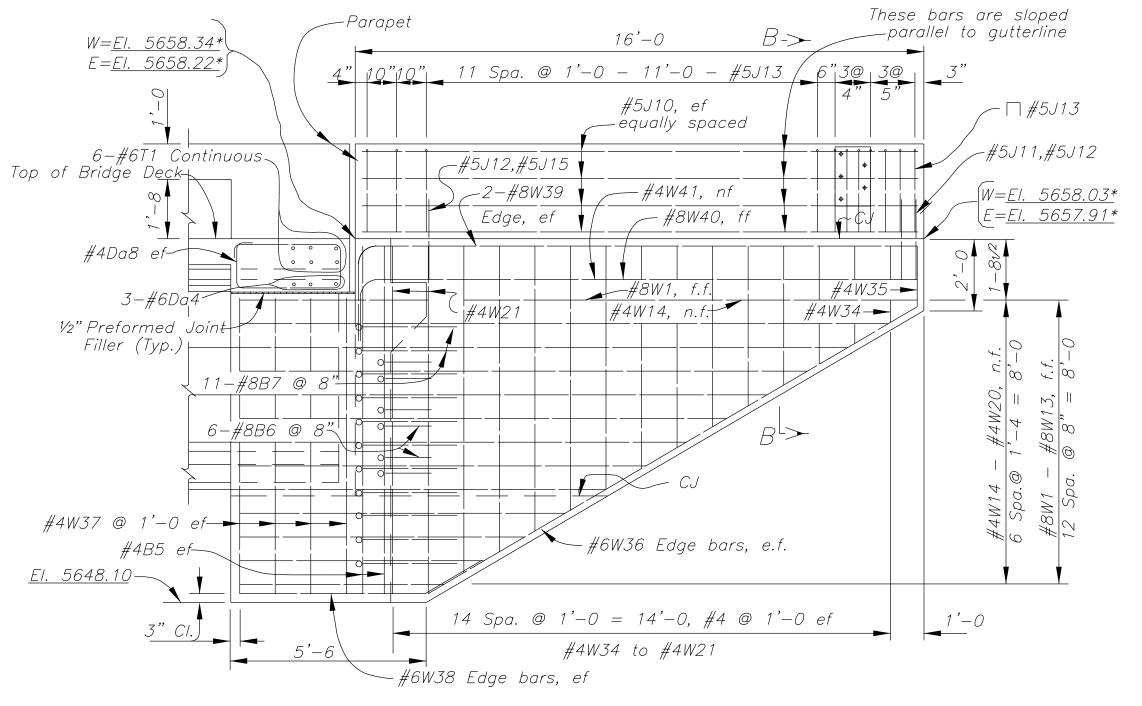
RIO PUERCO BRIDGE ABUTMENT 1 AND ABUTMENT 2 DETAILS, SHEET 2

Designed by: BUREAU OF	RECLAMATION
Drawn by: BOR, dc, rsh	Date: 01/17/14
Revised by:	Date: — —
File Name: 10_BIAwing	

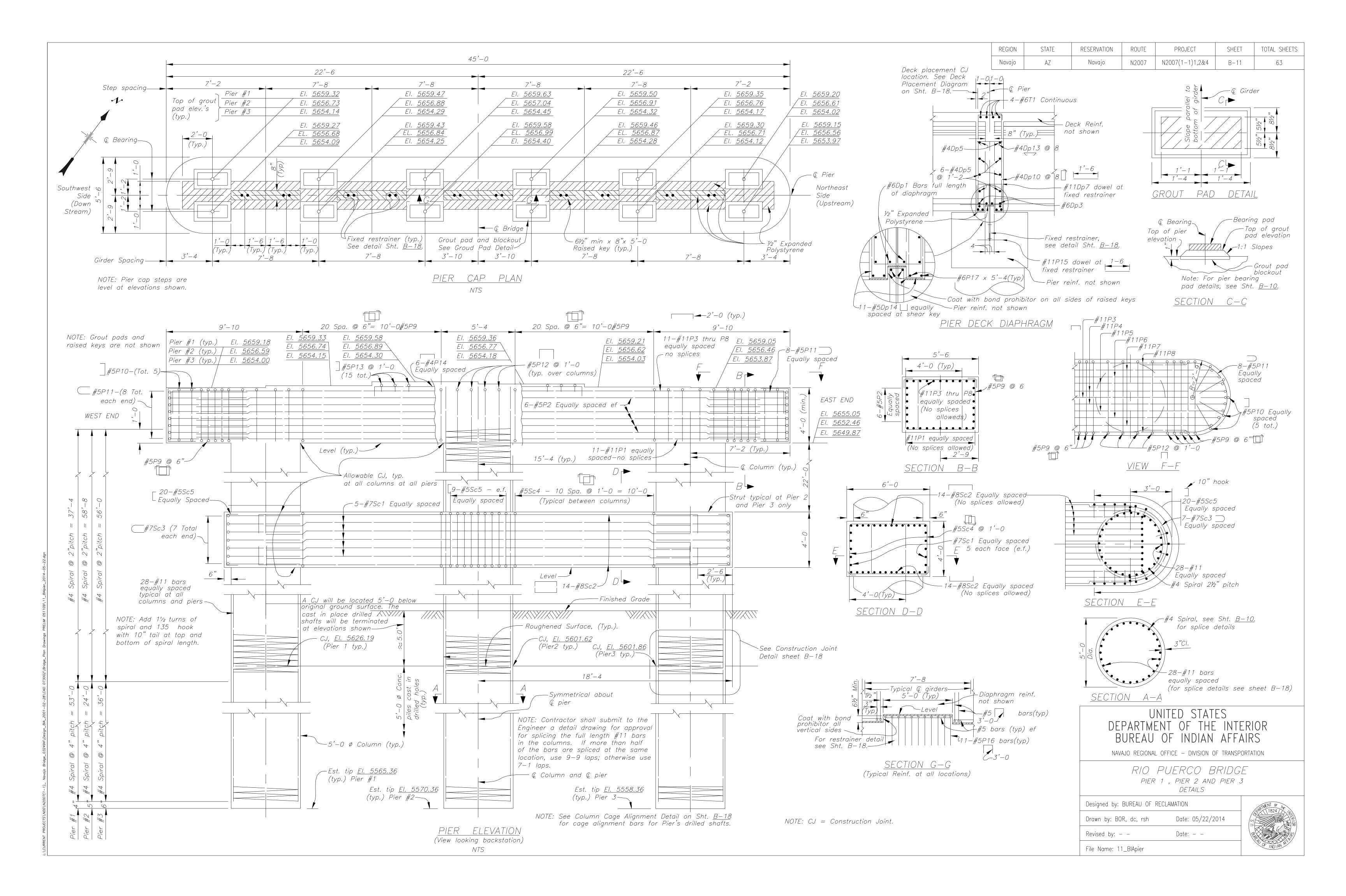


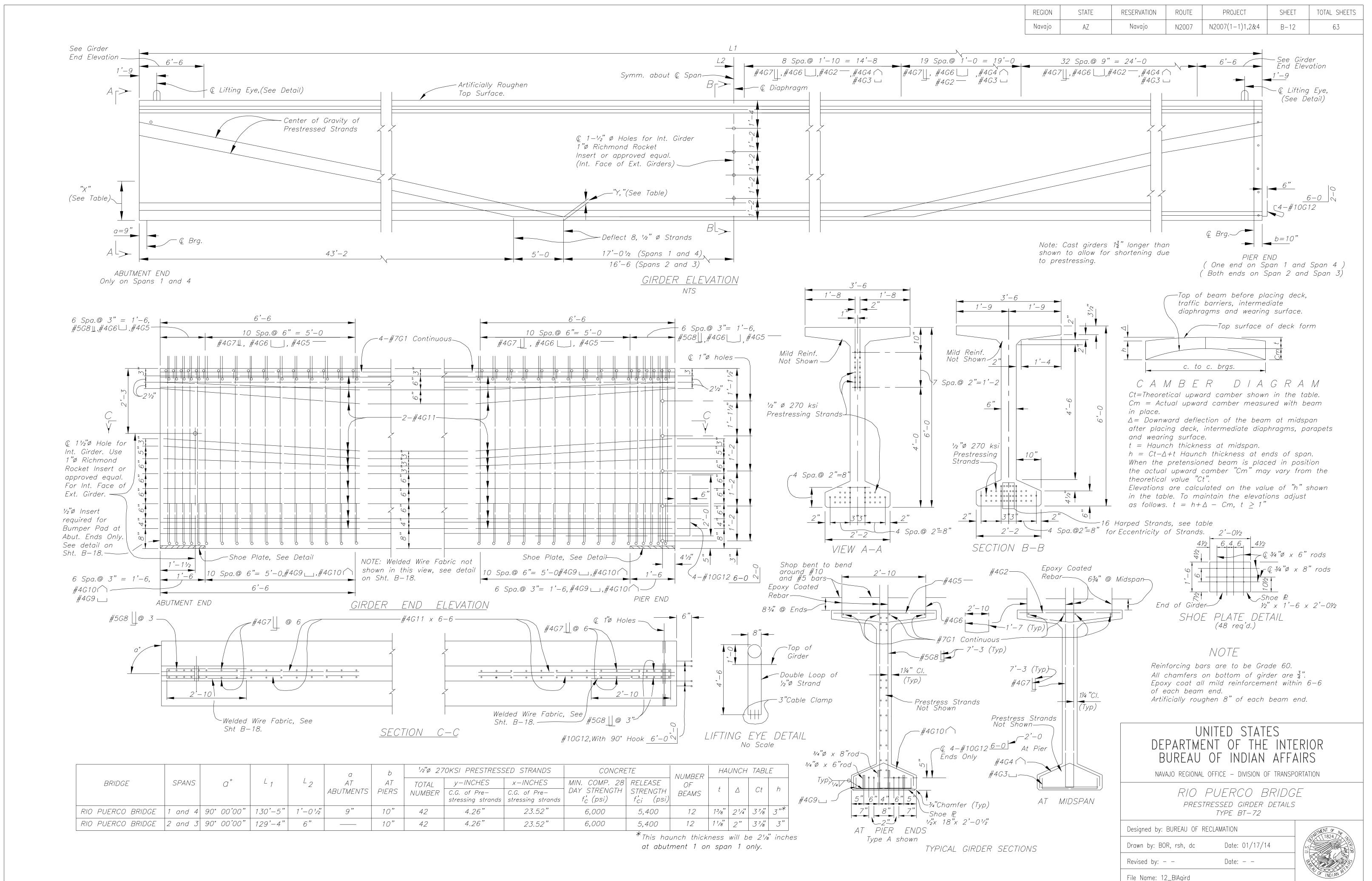
✓These bars are sloped parallel to gutterline ✓Parapet	
M= <u>E1. 5668.77</u>	
3" 3@ 3@ 6" 11 Spa. @ 1'-0 - 11'-0 - #5J13 10"10" 4" \\ \begin{align*} E = \begin{align*}	* -
#5J13 5" 4" A #5J10, ef equally spaced	
#5J11,#5J12 #5J13/ 6-#6T1 Continuous #5J12,#5J15 #5J13/ 6-#6T1 Continuous Deck	<
W = EI. 5669.03* $E = EI. 5668.91*$ $W = EI. 5668.91*$ $E = EI. 5668.91*$ $E = EI. 5668.91*$ $E = EI. 5668.91*$ $E = EI. 5668.91*$	
#8W1, f.f. #8W1, f.f.	
#4W21) #4W34 #4W14, n.f. #4W21) #3-#6Da4	
#4W35 Joint Filler (Type	p.)
0 ti 0 ti 0 8"	,
© # 1	
#6W36 Edge bars, e.f. #4W37 @ 1'-C	' et
EI. 5658	.5.1
	<u></u> ,
1'-0 14 Spa. @ 1'-0 = 14'-0, #4 @ 1'-0 ef	
#4W34 to #4W21 #6W38 Edge bars, ef	
// 01100 Eage Bars, er	



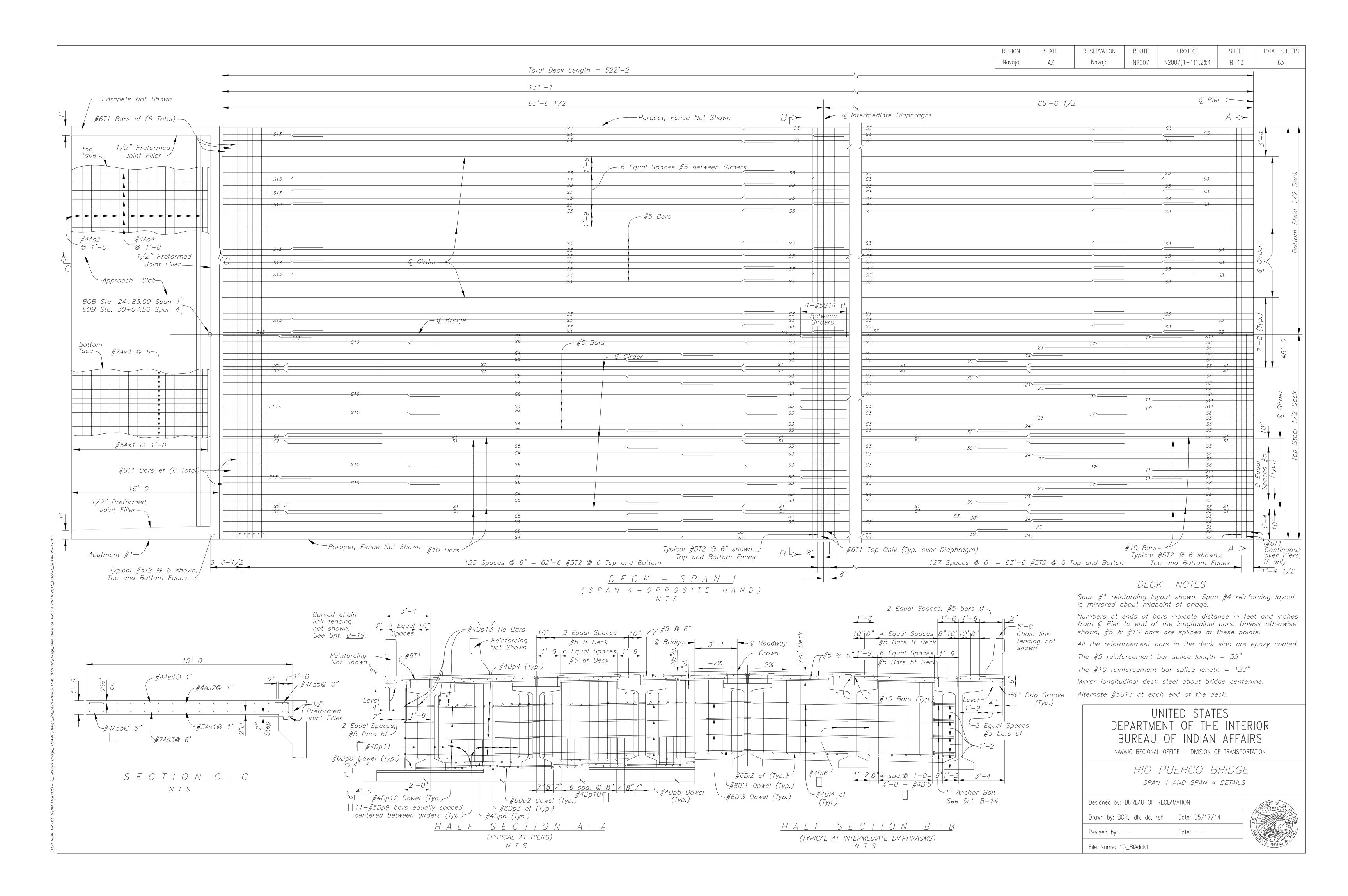


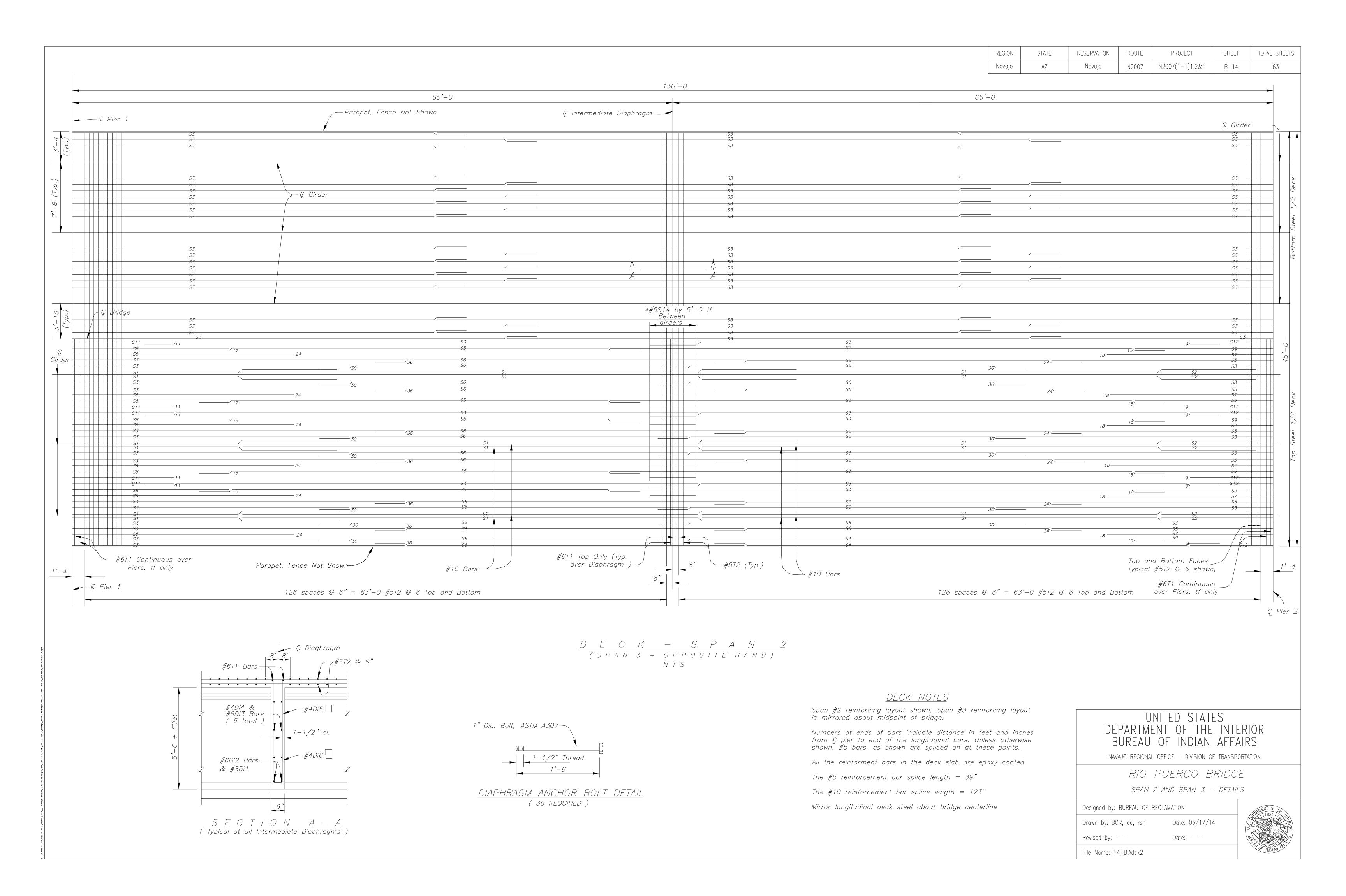
<u>ELEVATION</u> (WEST SIDE WINGWALL - ABUTMENT 2) (EAST SIDE WINGWALL SIMILAR)

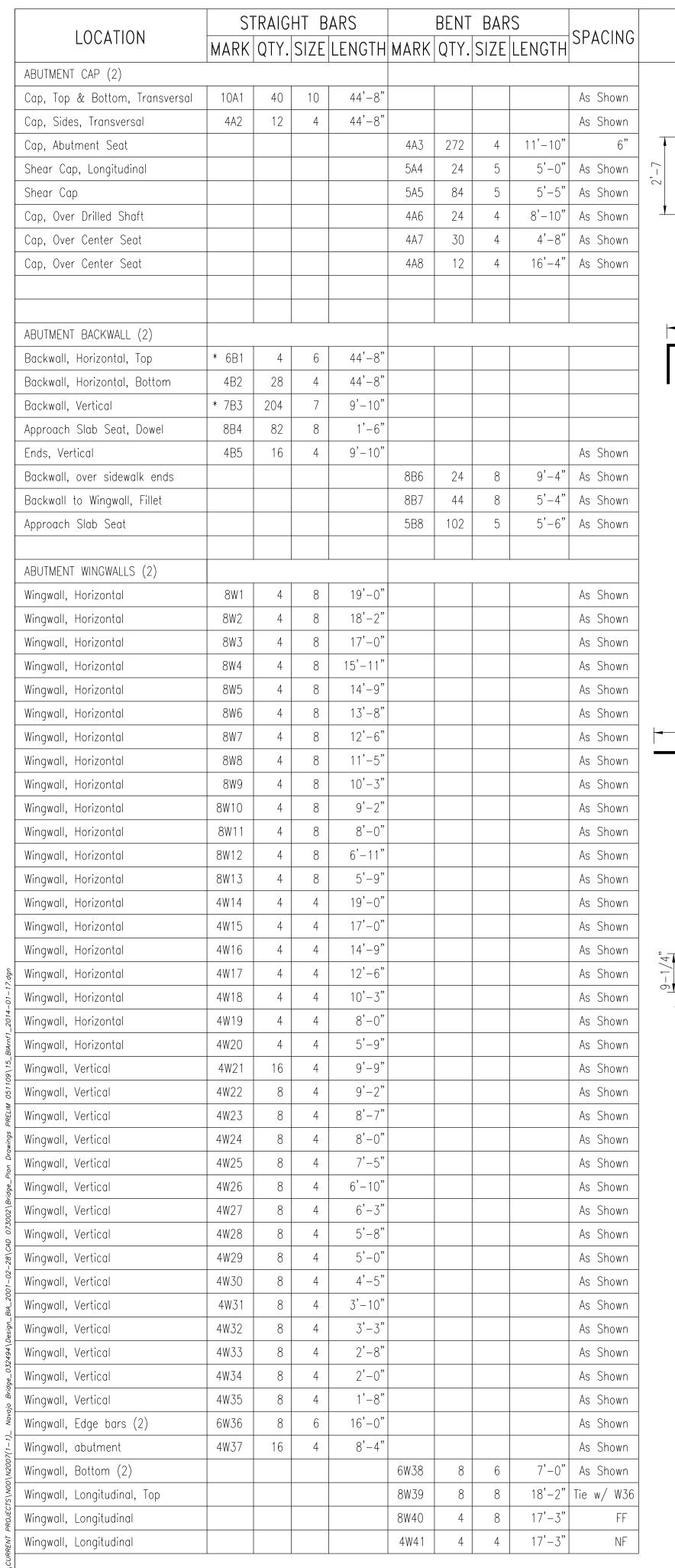


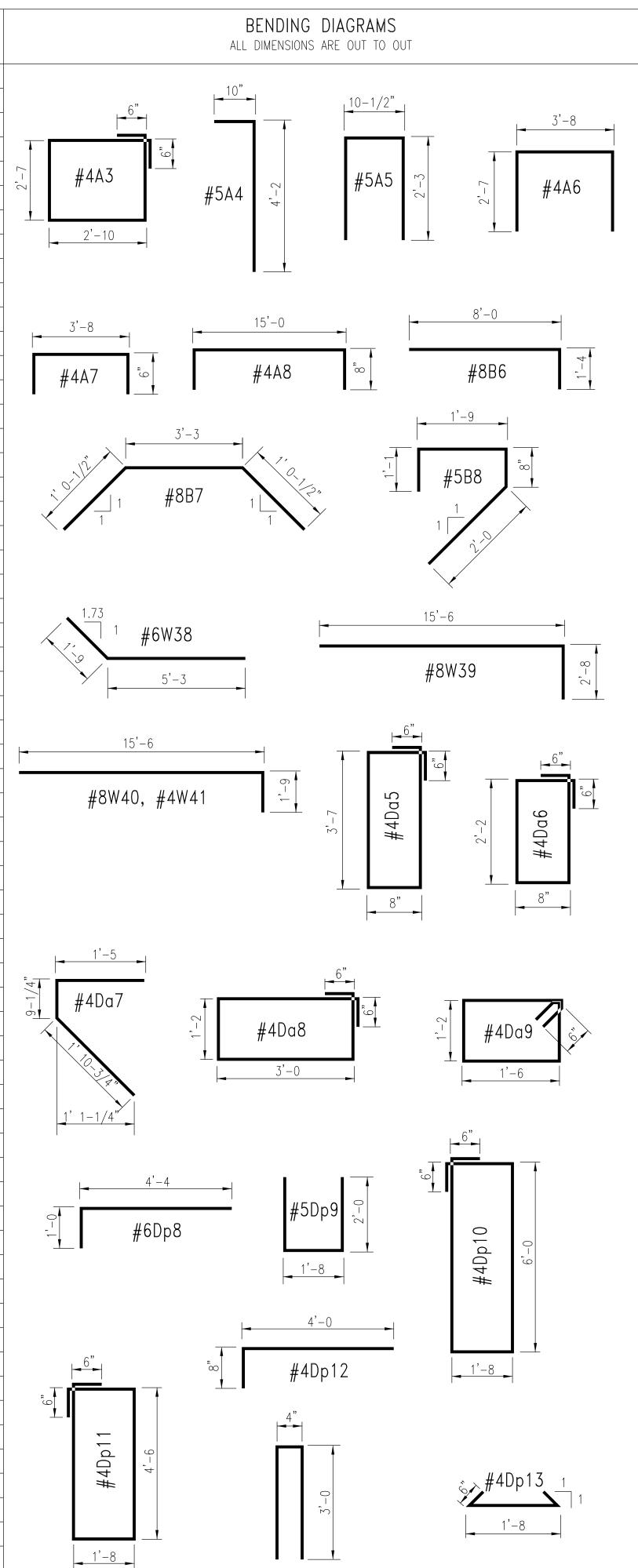


L:\CURRENT PROJECTS\NOO\N2007(1-1)_ Navajo Bridge_032494\Design_BIA_2001-02-28\CAD 073002\Bridge_PIan Drawings PRELIM 051109\12_BIAgird_201

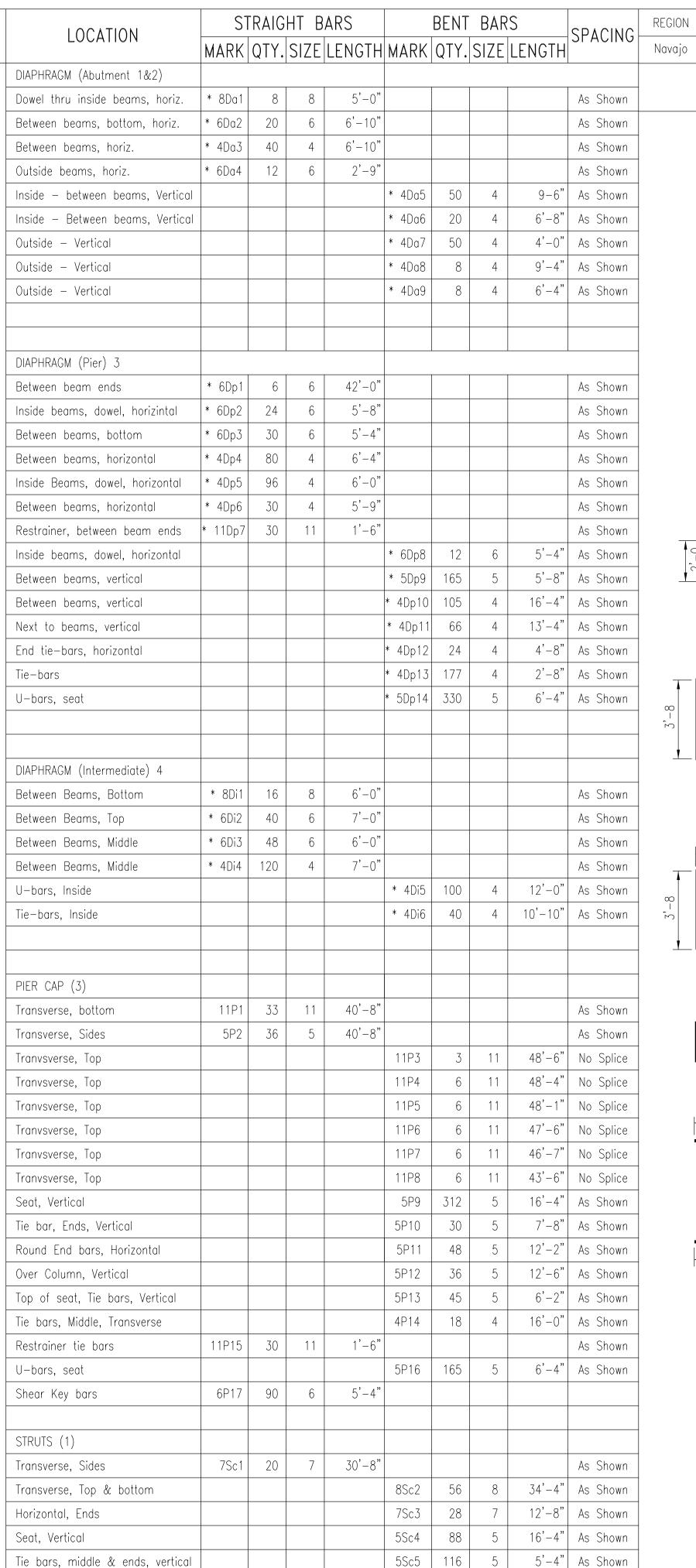


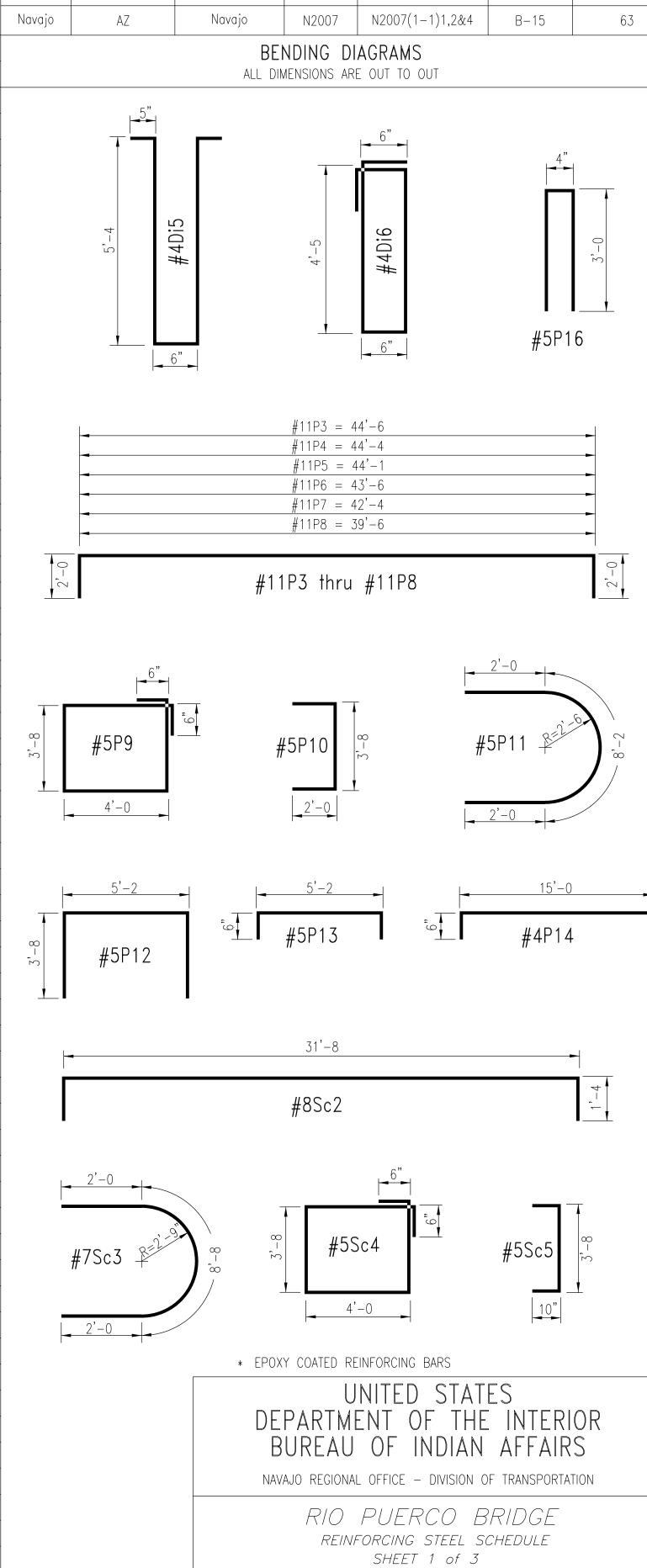






#5Dp14





Designed by: BUREAU OF RECLAMATION

Drawn by: BOR, dc, rsh

Revised by: - -

File Name: 15_BlArnf1

Date: 01/17/14

Date: - -

ROUTE

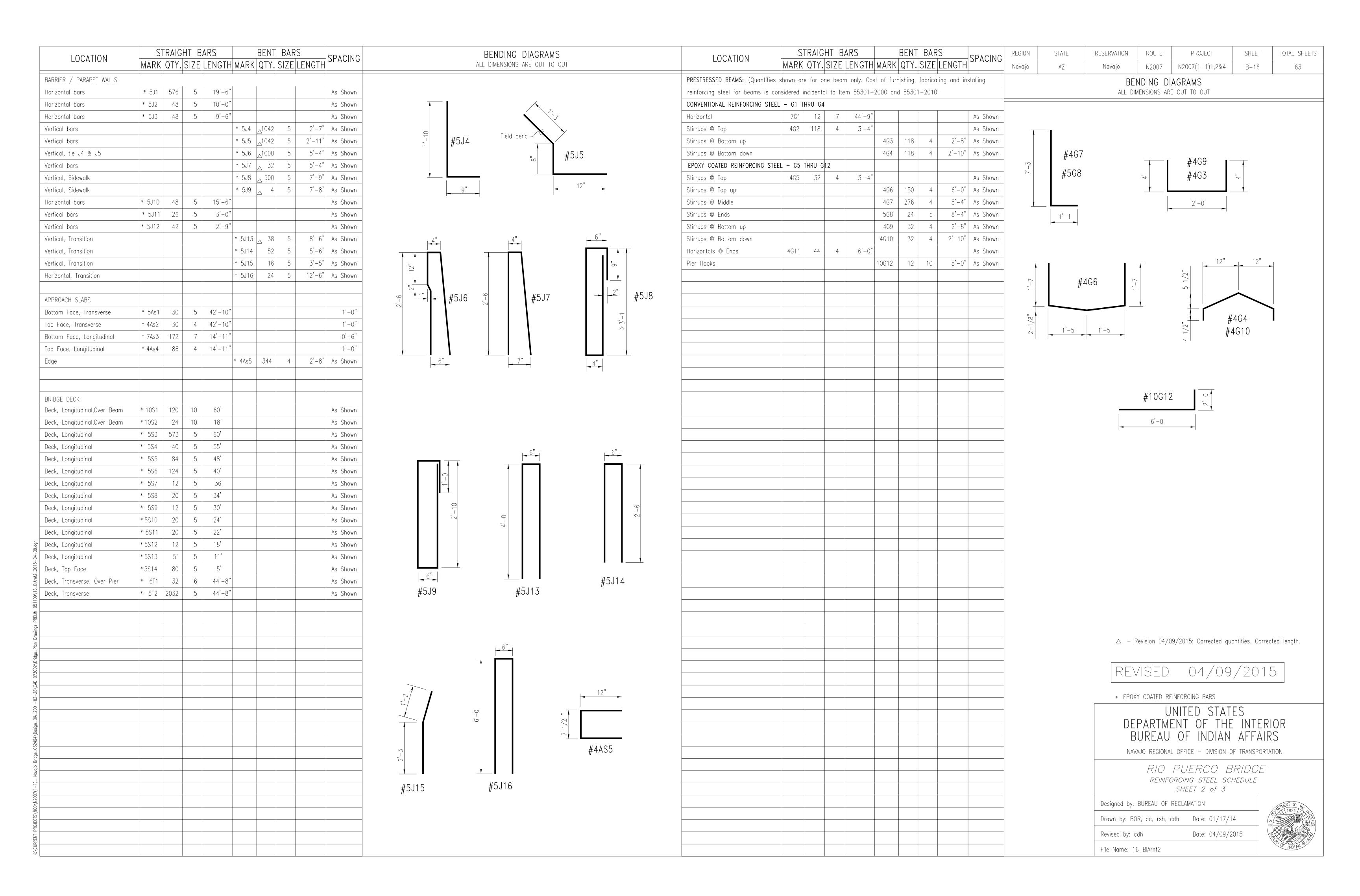
RESERVATION

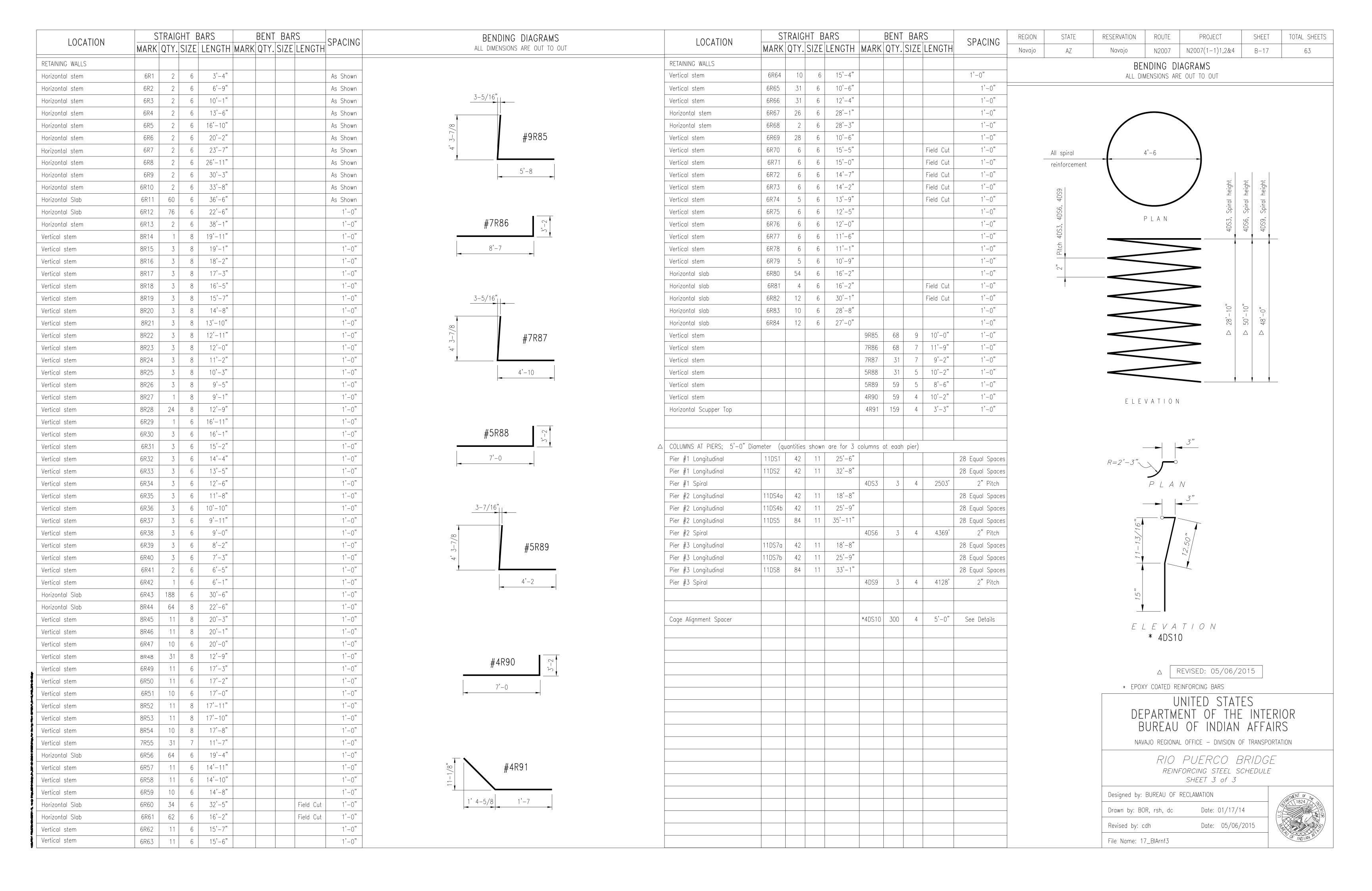
STATE

PROJECT

SHEET

TOTAL SHEETS



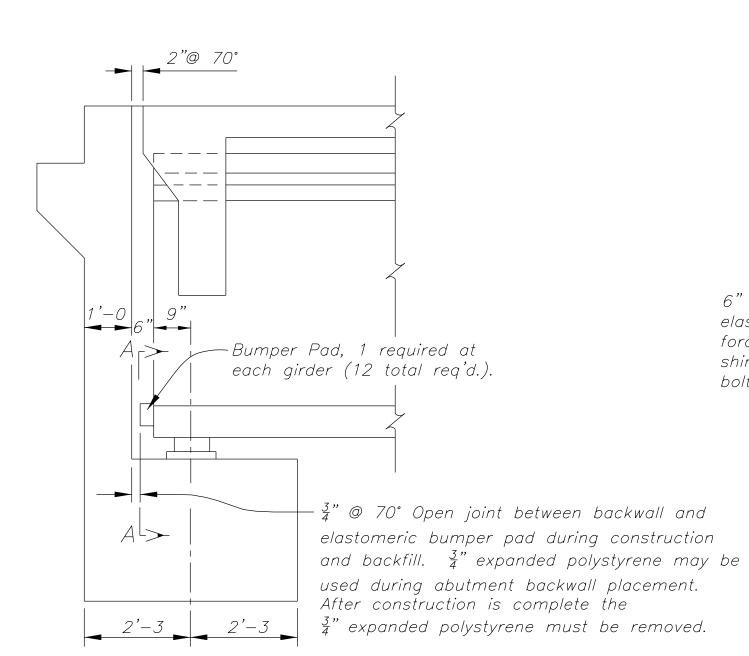


Note: #4 Alignment ties bars are to

be epoxy coated.

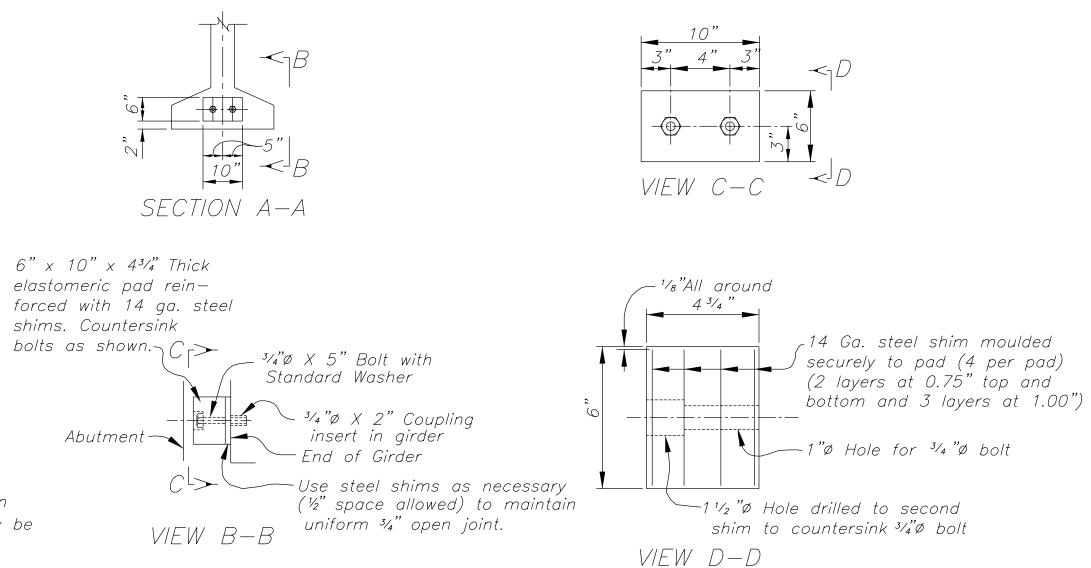
Heavy ties at intersections.

—Top of Casing



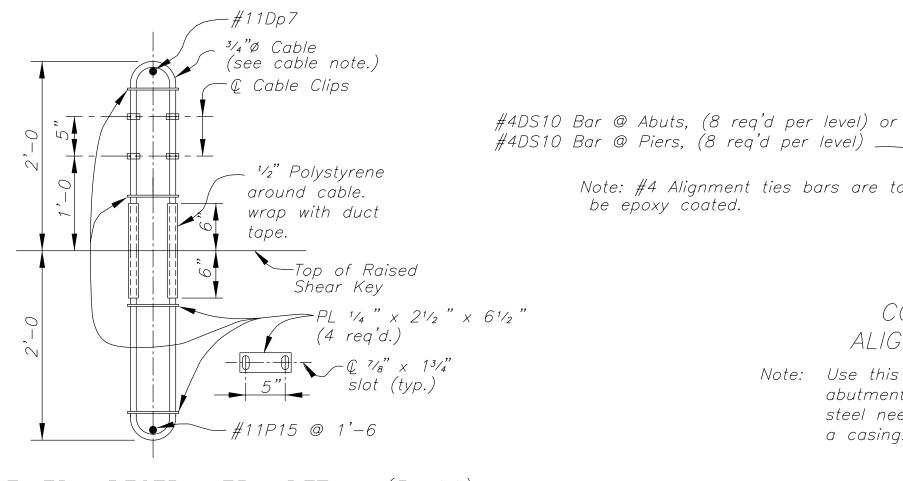
TYPICAL SECTION

AT ABUTMENT (B-9)



BUMPER PAD DETAILS (B-9)

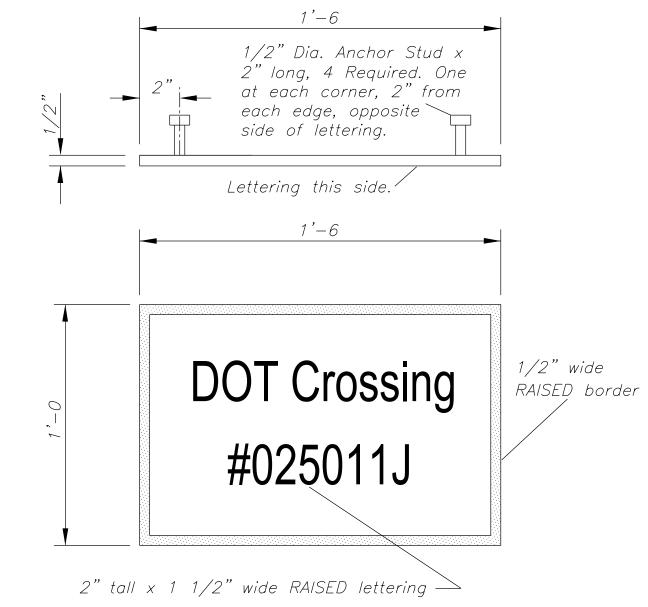
(No Scale)



CABLE NOTE: Restrainer cables shall be 3/4"ø min. preformed 6 x 19 galvanized wire rope and shall meet the requirement of AASHTO M30 with a min. breaking strength of 42 kips.

Note: The splice detail shown shall be used at the construction joint at top of drilled shaft for Piers 1, 2 & 3

a casing. RESTRAINER DETAIL (B-11)



COLUMN CAGE

ALIGNMENT DETAIL

abutments where column cage

steel needs to be aligned inside

Note: Use this detail at all piers and

DOT CROSSING NUMBER PLAQUE DETAILS

NOTE: DOT Crossing number plagues shall be made of bronze with raised polished lettering and raised polished border conforming to the dimensions shown above. Two (2) bronze plaques are required, one (1) for each end of the bridge. Bronze plaques shall be installed as shown on Sheets B-20 and B-21. Submit shop drawings for bronze plaques for review and approval no later than 45 days prior to fabrication. No ordering of materials or fabrication shall occur before written approval of the shop drawings is obtained by the Contractor. All work for furnishing, fabricating and installing bronze plaques shall be considered an incidental obligation of, and included under, Item 55201-0200, Structural Concrete.

REVISED 04/08/2015

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

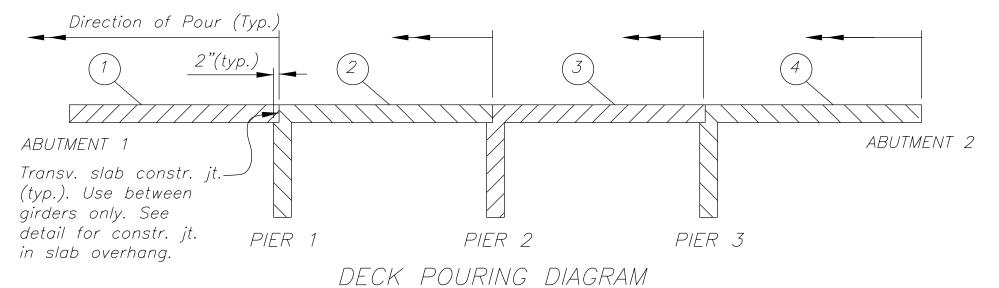
RIO PUERCO BRIDGE MISCELLANEOUS DETAILS

Designed by: BUREAU OF RECLAMATION

Drawn by: BOR, dc, rsh, cdh Date: 01/17/14

Date: 04/08/2015 Revised by: cdh

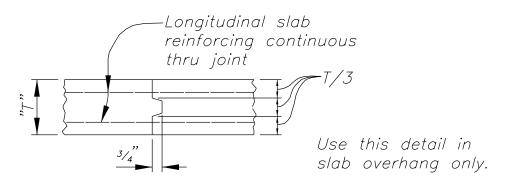
File Name: 18_BIAmisc



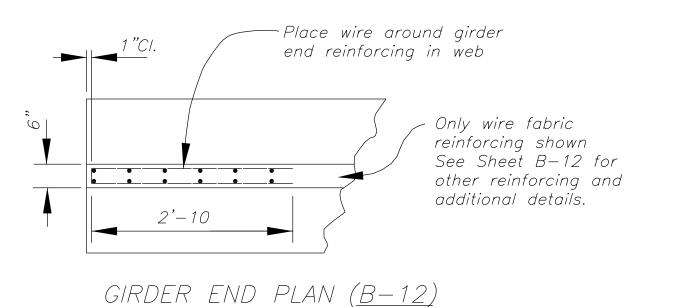
(No Scale)

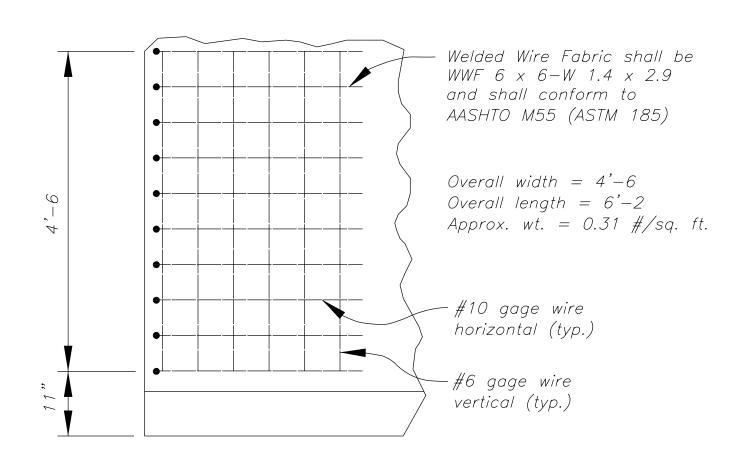
NOTE: Pour end diaphragms and intermediate diaphragms first. Then deck pours shall be made in numerial sequence and in the direction shown. Place pier diaphragms just before deck closures.

The purpose of this deck pouring diagram is to place all deck concrete on both sides of a pier diaphragm before casting the pier diaphragm. The contractor may submit an alternative sequence to the Engineer for approval provided that it accomplishes the same purpose.

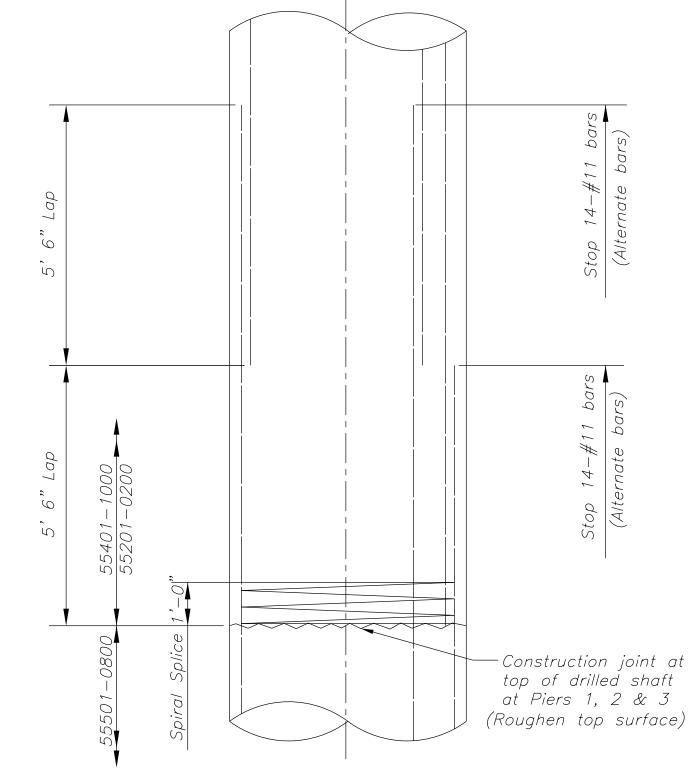


TRANSVERSE SLAB CONSTRUCTION JOINT (No Scale)



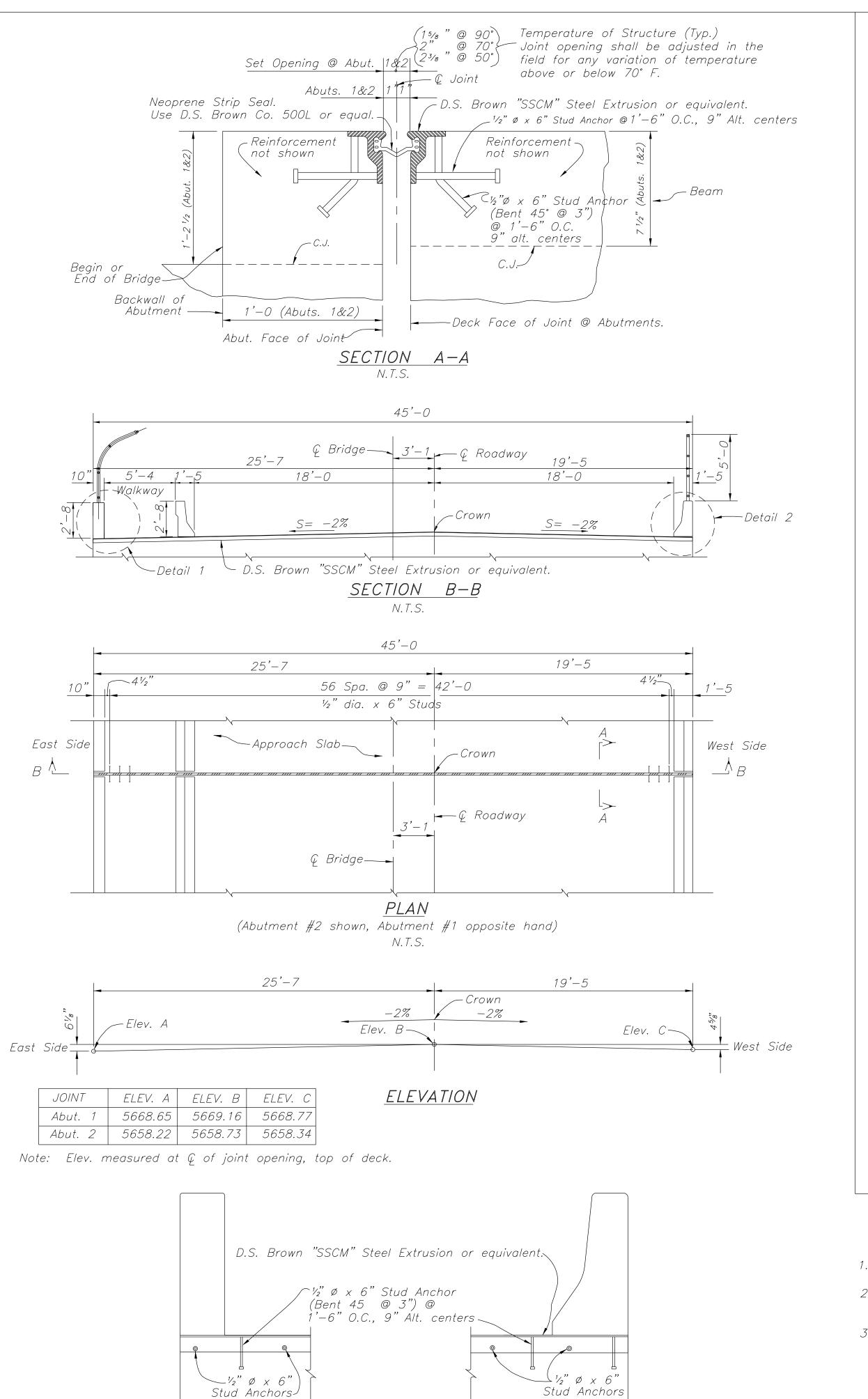


GIRDER END ELEVATION (B-12)



CONSTRUCTION JOINT DETAIL

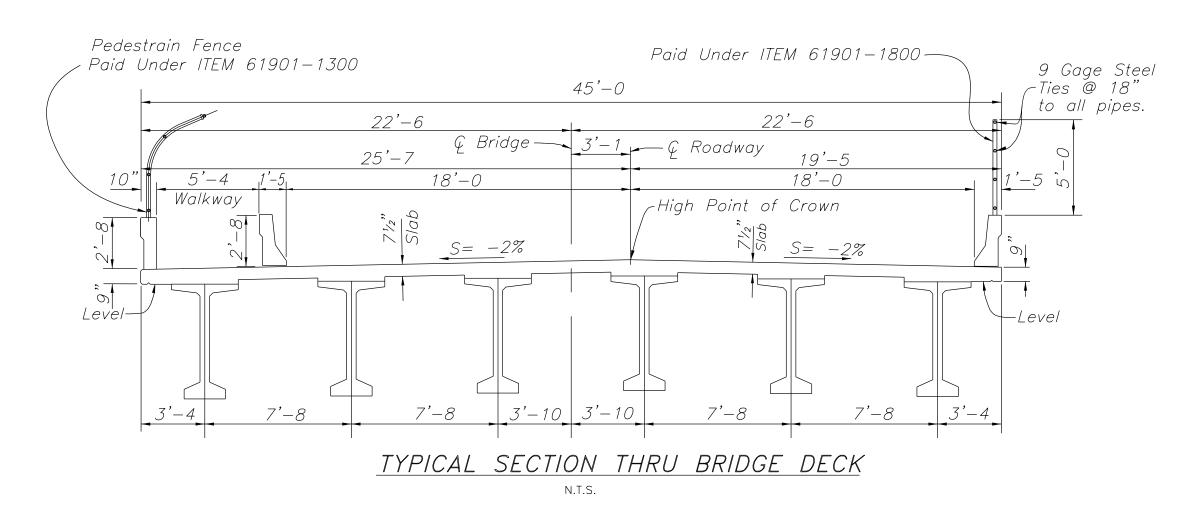


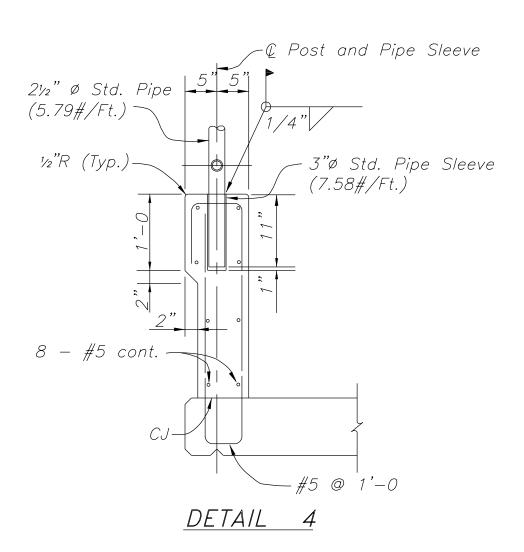


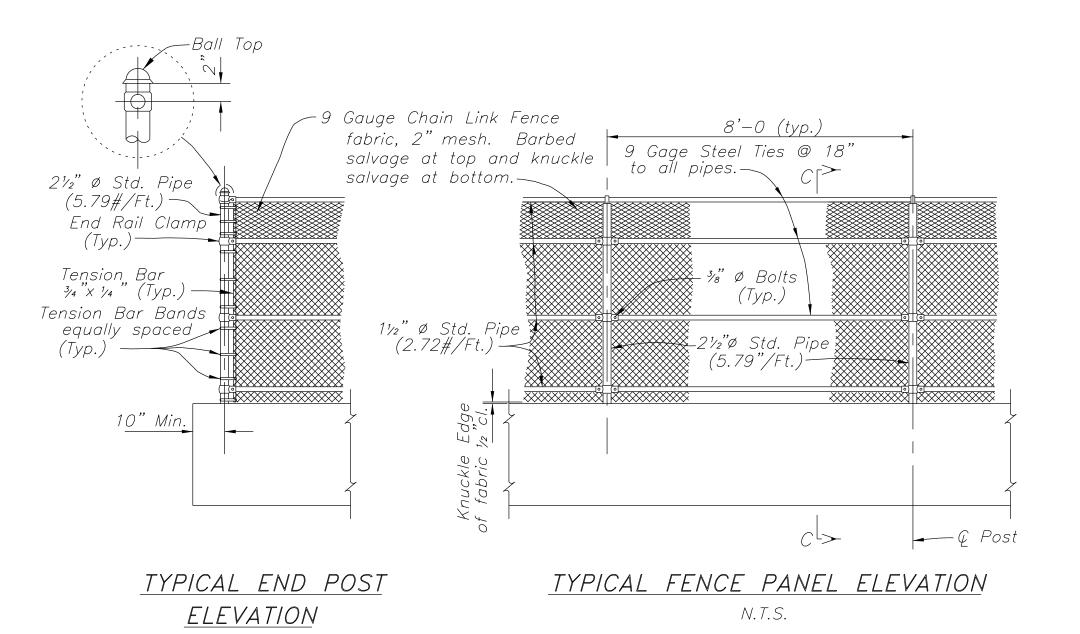
DETAIL 2

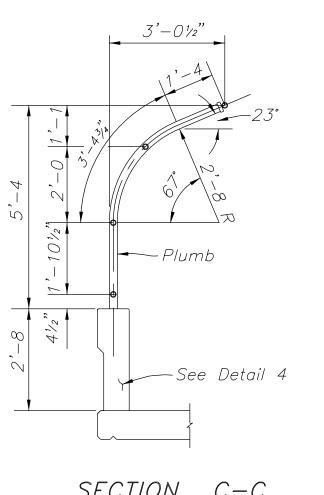
DETAIL

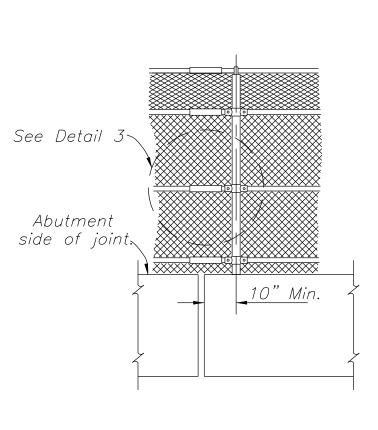
REGIONSTATERESERVATIONROUTEPROJECTSHEETTOTAL SHEETSNavajoAZNavajoN2007N2007(1-1)1,2&4B-1963









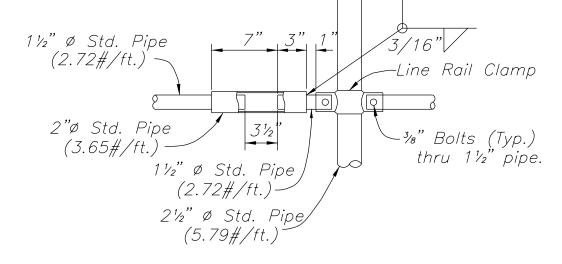


<u>SECTION</u> <u>C-C</u> (Typical Interior Post)

FENCE DETAIL AT
PARAPET EXPANSION JOINT

CHAIN LINK NOTES

- 1. Chain link fence fabric, posts, fittings and hardware shall conform to AASHTO M181 Type I or II. For Type 1, the wire fabric coating shall be Class A.
- 2. All galvanizing that has been damaged in handling, transportation or welding shall be repaired by the application of a paste compound of an approved zinc powder and flux.
- 3. All exposed edges shall be smooth.4. All bolt heads shall be to the inside.



DETAIL 3

EXPANSION JOINT GENERAL NOTES

- Contractor shall submit shop drawings for the joint showing all fabrication details and material specifications according to specification requirements.
 The joint shall be installed in accordance with the manufacture's instructions. A representative of the manufacture shall be present at the job site during installation.
- 3. Steel extrusions shall conform to AASHTO M270, Grade 36 or Grade 50W (ASTM A709, Grade 36 or Grade 50W). Welded anchors shall conform to AASHTO M169 (ASTM A108). The entire assembly consisting of extrusions and welded anchors shall be galvanized after fabrication. Neoprene strip seal shall conform to the physical properties prescribed in Table 1 of AASHTO 220 (ASTM D2628).

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

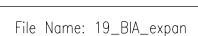
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

RIO PUERCO BRIDGE EXPANSION JOINT DETAILS AND PEDESTRIAN FENCING DETAILS

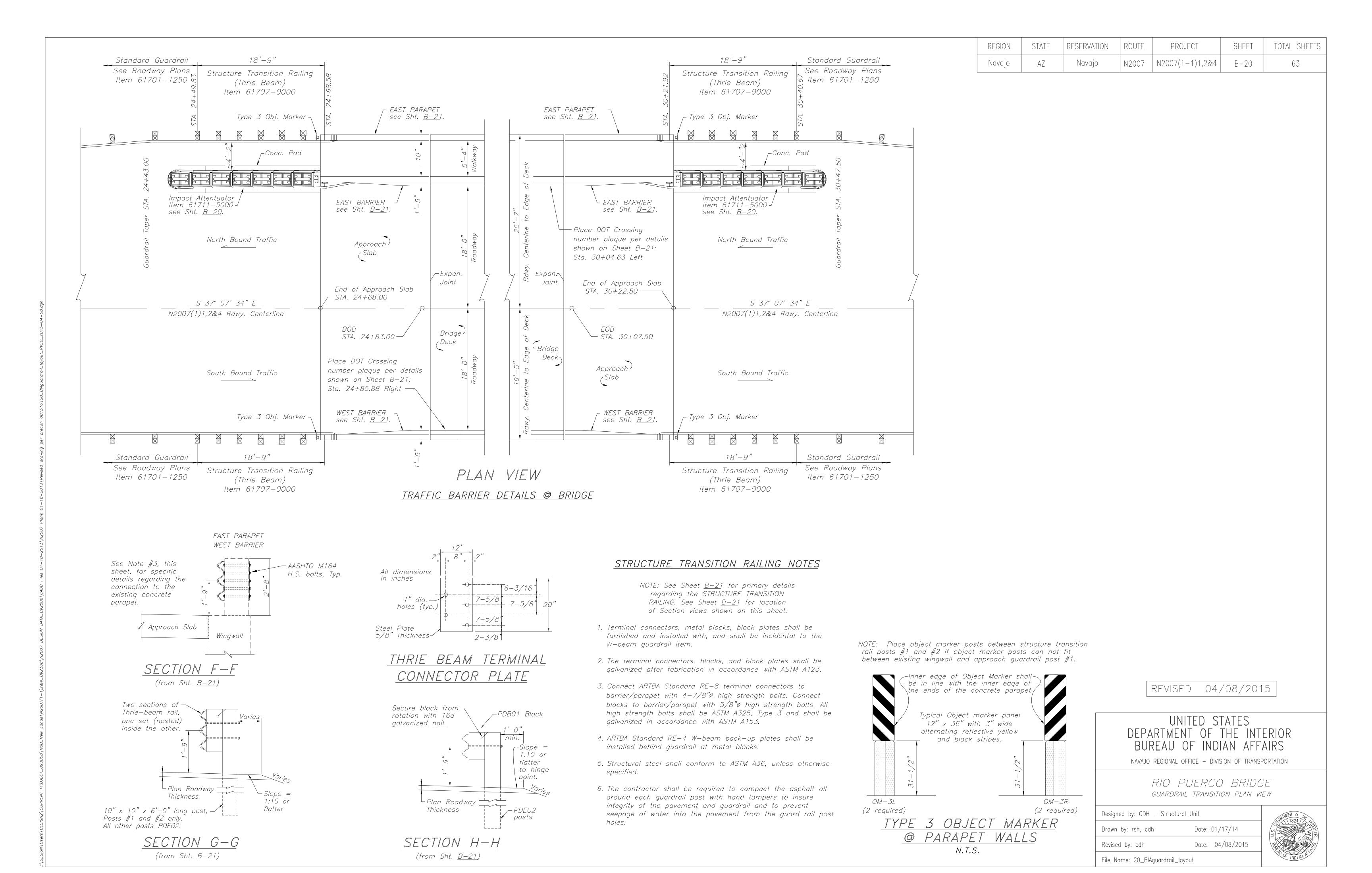
PEDESTRIAN FENCING DETAILS	
Designed by: BUREAU OF RECLAMATION	

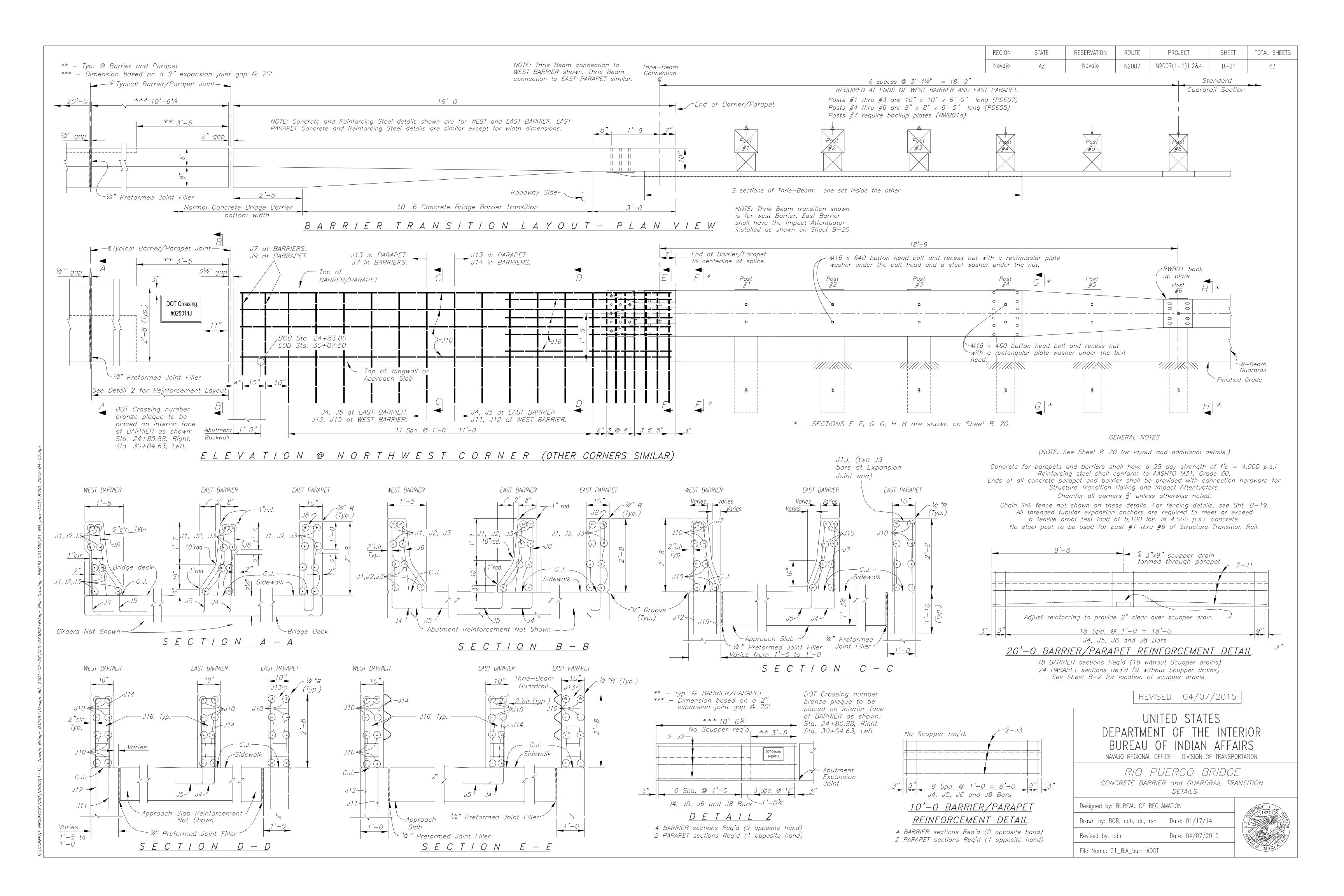
Drawn by: BOR, dc, rsh Date: 01/17/14

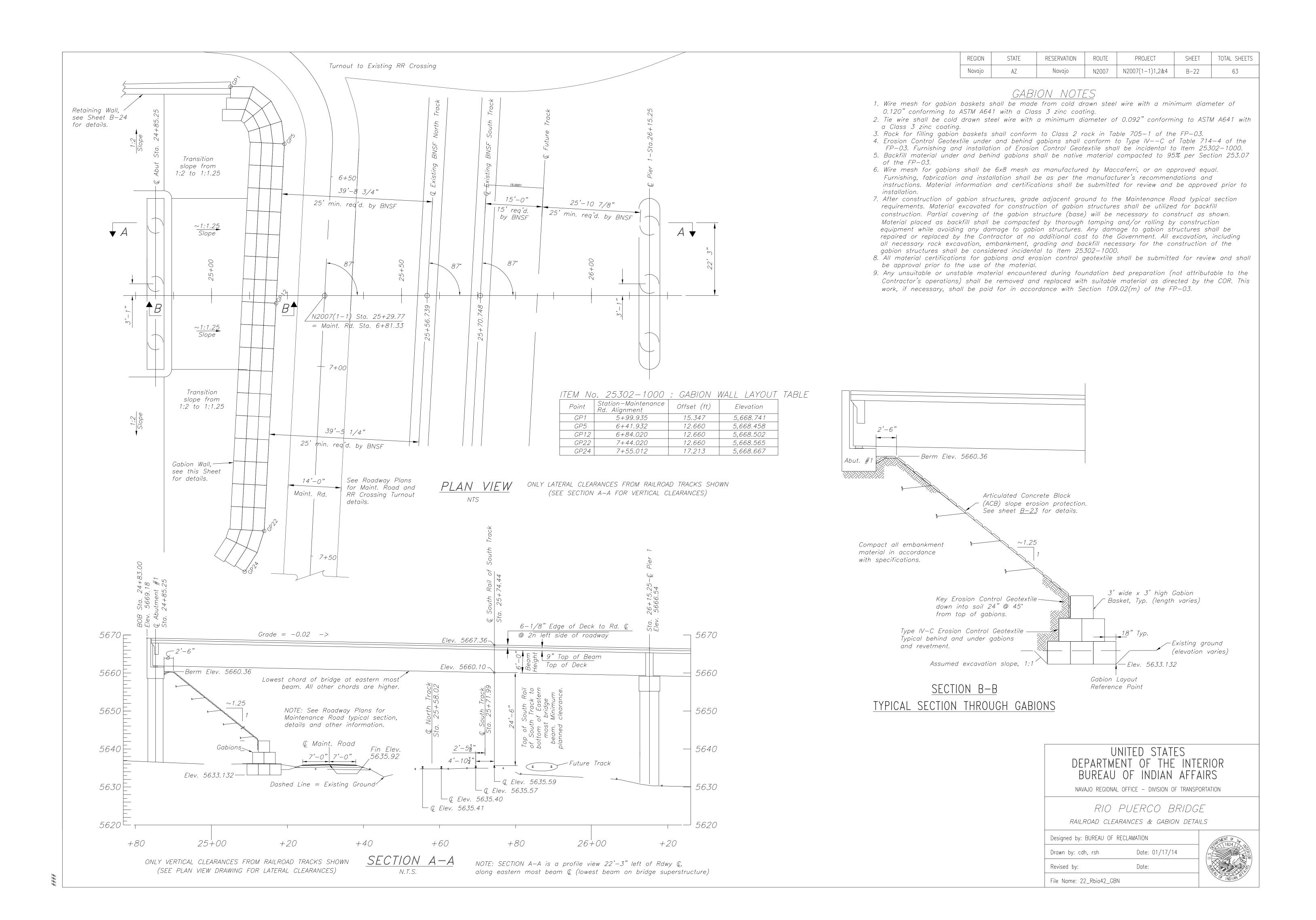
Revised by: - - Date: - -

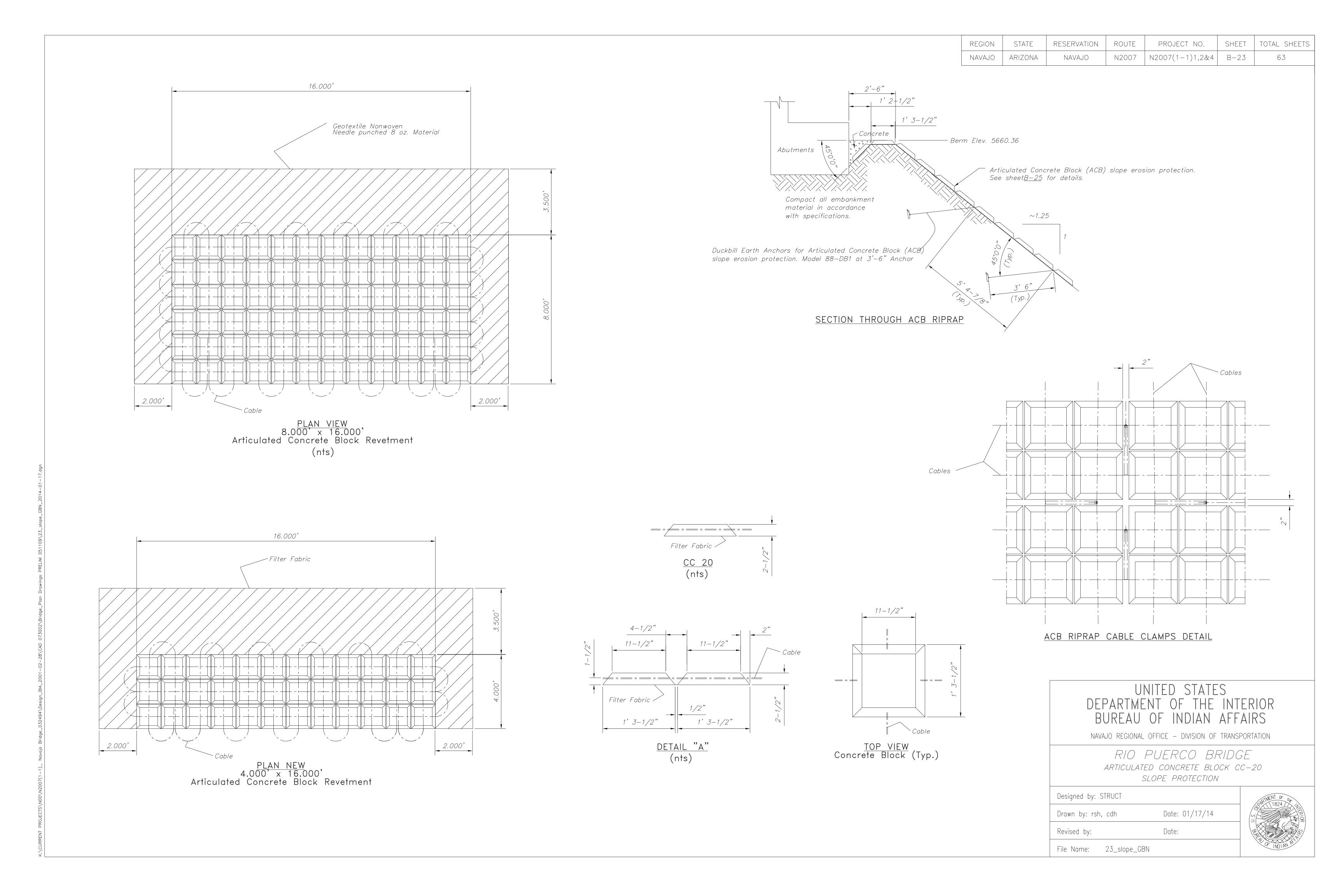


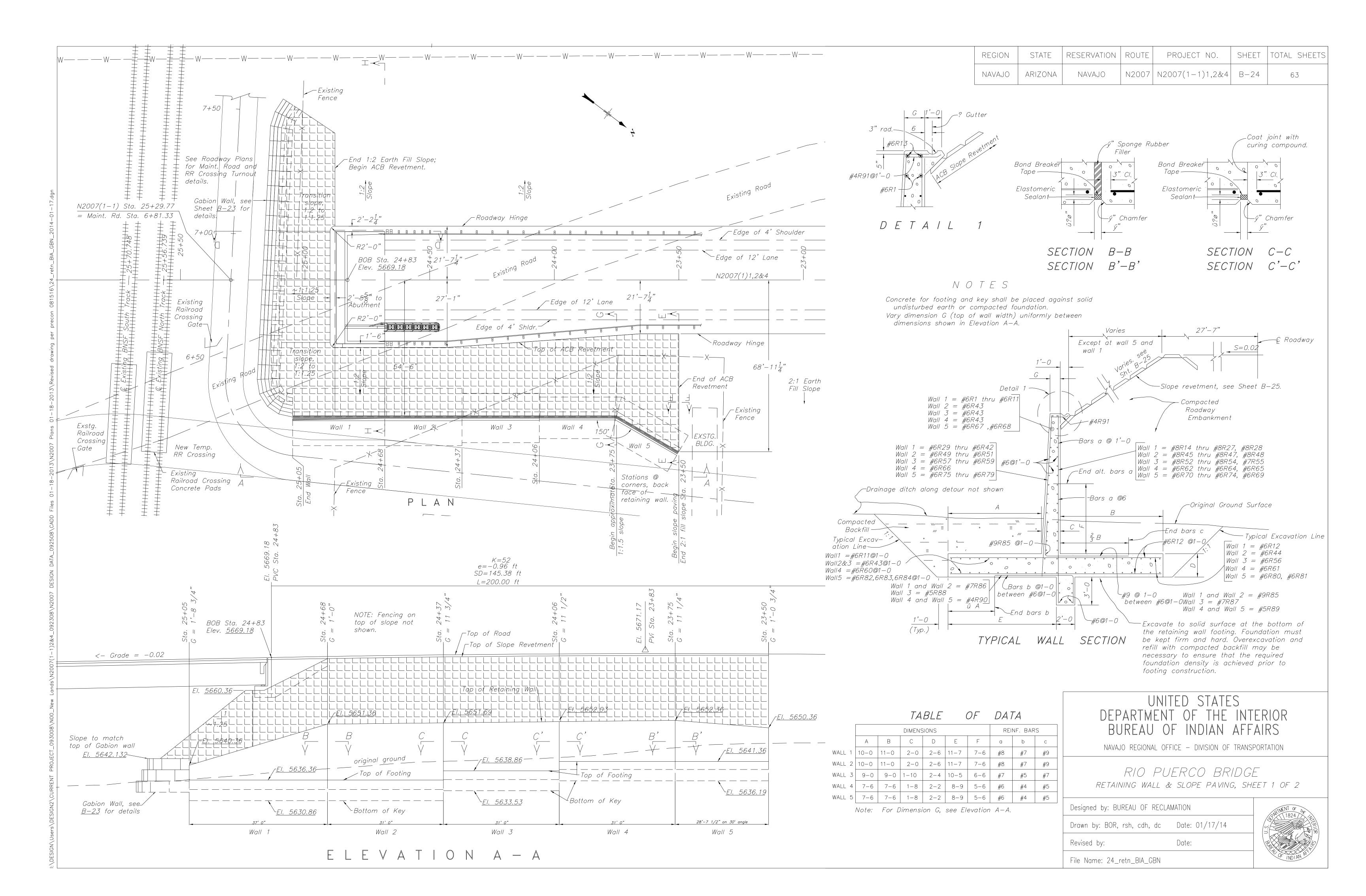


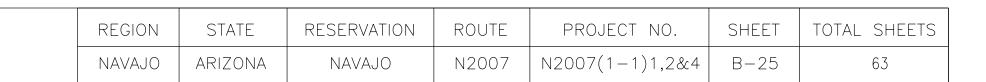












Date:

Revised by:

File Name: 25_slope_GBN

