

DESIGN	DATA		
Design Speed Min. Radius of Curve Max. Gradient Min. Stopping Sight Dist. Min. Passing Sight Dist.	50 km/h 140 m 4.3% 65m 345m	2001 ADT 2021 ADT Design Hourly Volume Max. Super Elevation	483 VPD 718 VPD 20 VPD 4%

N.T.S.

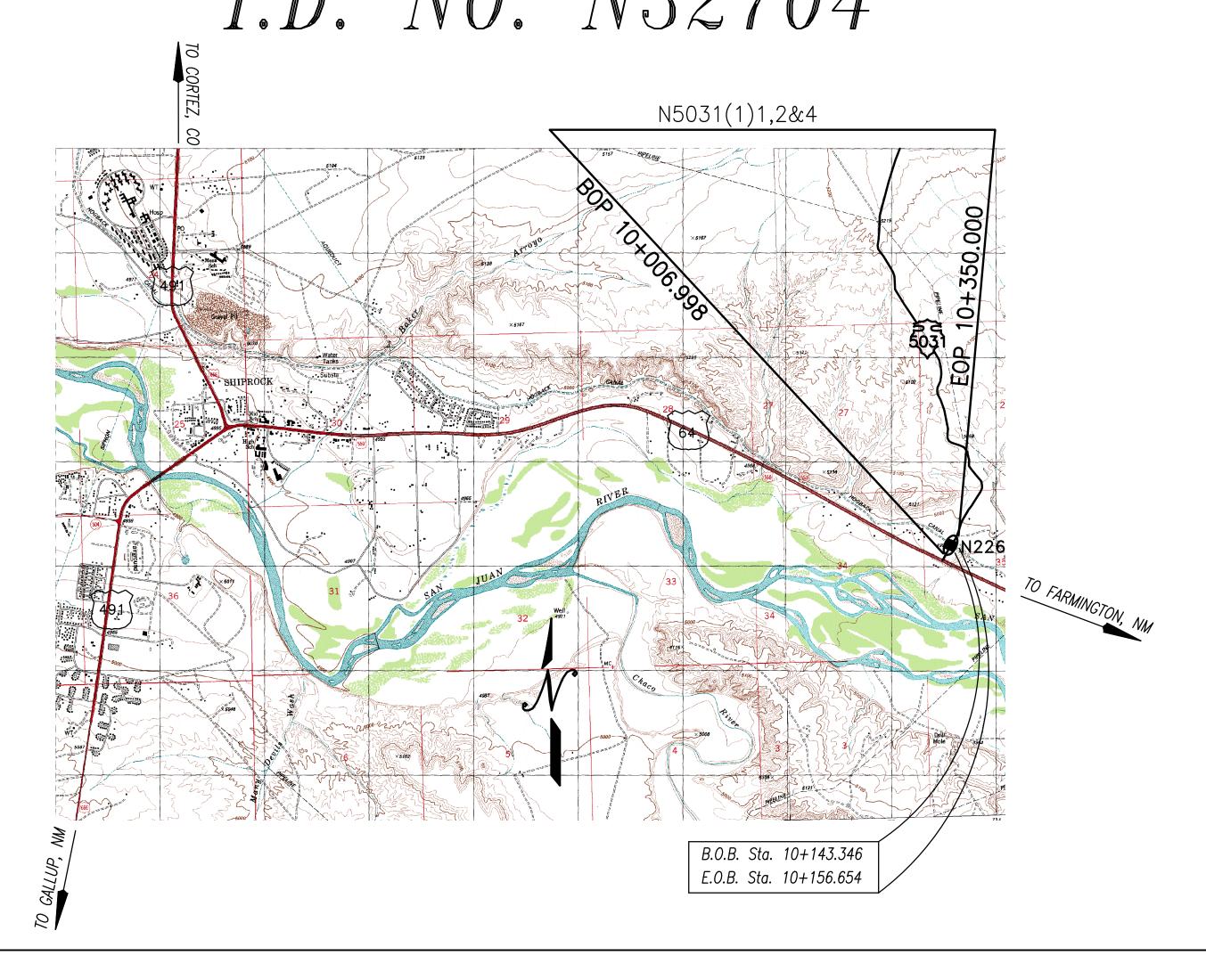
RIGHT	OF WAY	
Left Corridor Width	STA 10+005.657 TO STA 10+133.492 STA 10+133.492 TO STA 10+182.668 STA 10+182.668 TO STA 10+181.105 STA 10+181.105 TAPER FROM TO STA 10+350.000	7.000 m 23.000 m 13.690 m 13.690 m 17.027 m
Right Corridor Width	STA 10+005.657 TAPER FROM TO STA 10+129.867 STA 10+129.867 TO STA 10+350.000	8.617 m 6.452 m 23.000 m

LENGTH OF	PROJEC	Т
STATION TO STATION	LIN. METERS	KILOMETERS
B.O.P. Station 10+006.969 B.O.B. Station 10+143.346 E.O.B. Station 10+156.654 E.O.P. Station 10+350.000	136.377 13.308 193.346	0.136 0.014 0.193
TOTAL	343.031	0.343



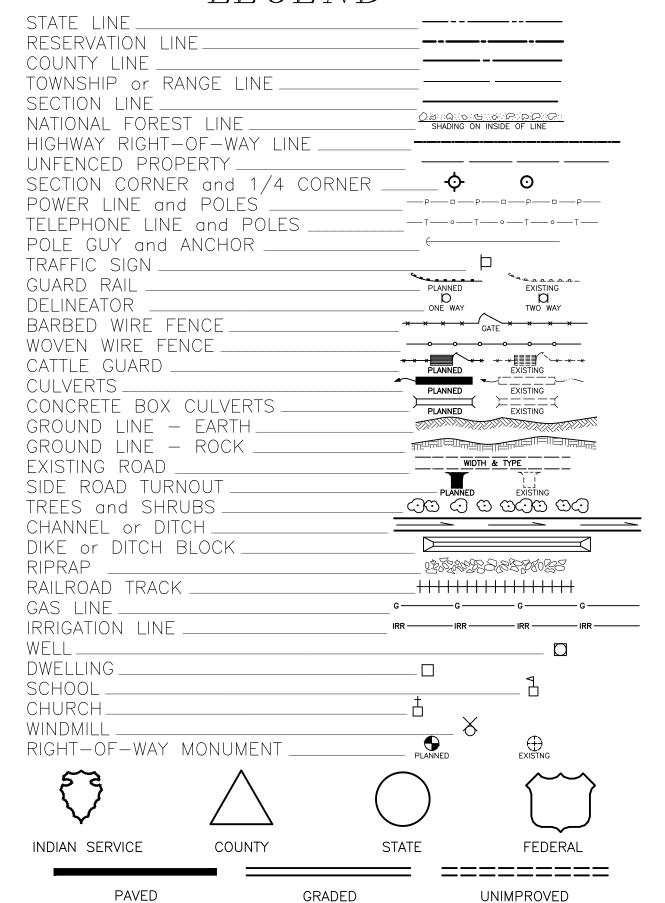


PROJ. N5031(1)1,2&4
HOGBACK IRRIGATION CANAL
I.D. NO. N32704



1	INDEX TO SHEETS						
SHT. NO.	. DESCRIPTION						
1	TITLE SHEET						
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22	STANDARD PIPE INSTALLATION TURNOUT & ROUTE SIGN DETAILS						
23	PIPE CULVERT DETAILS						
24	DELINEATOR AND ROW MARKER DETAILS						
25	CHAIN LINK FENCING DETAILS						
26	BARBWIRE FENCING DETAILS						
27	PRECAST CONCRETE CATTLEGUARD DETAILS						
28	CATTLEGUARD WING-BRACE DETAILS						
29	CONCRETE DRAINAGE PAD DETAILS FOR PCCG						
30-31	STORMWATER POLLUTION EROSION/SEDIMENT CONTROL DETAILS						
32	TEMPORARY TRAFFIC CONTROL & ALTERNATE DETOUR LAYOUT						

# LEGEND



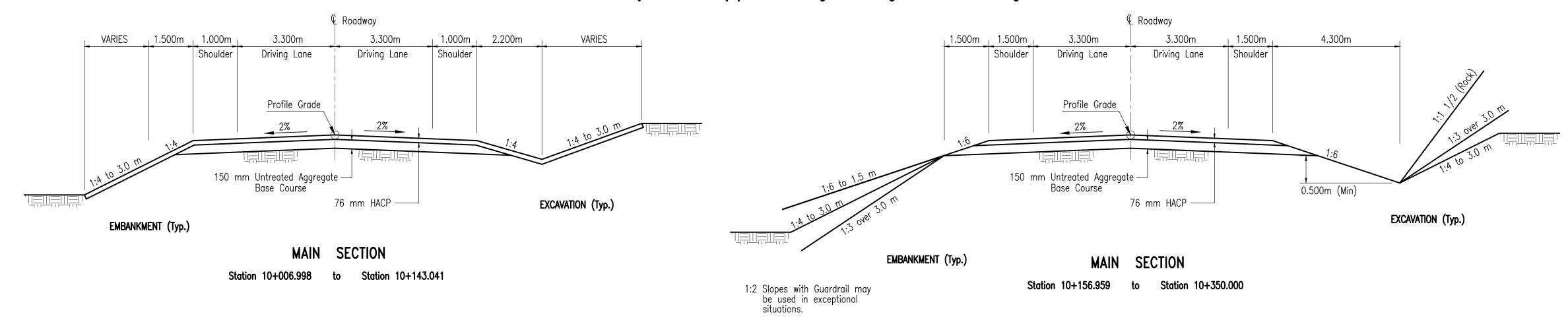
# U. S. DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE \* DIVISION OF TRANSPORTATION

		0	
RECOMMENDED AP	PROVAL	APPROV	AL
AGENCY ROAD ENGINEER	DATE	1824	
DIVISION MANAGER	DATE	OF INDIAN	
PLANNING AND DESIGN CHIEF	DATE	REGIONAL DIRECTOR	DATE

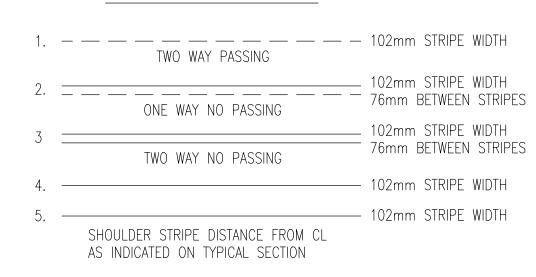
REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	NM	Navajo	N5031	N5031(1)1,2&4	2	32

### TYPICAL ROAD SECTIONS

# Project N5031(1)1,2&4 Hogback Irrigation Canal Bridge



# STRIPING DETAILS



# STRIPING NOTES

- Details (1) thru (4) are yellow, detail (5) is white.
   Payment will be made for meters only.
   Striping shall conform to the current edition of the manual on Uniform Traffic Control Devices for Streets and Highways and
- Section 634, Traffic Markings of FP-03.

  4. The 1:3 striping segment to gap ratio (3.048m segment, 9.144m gap) is recommended.

# GENERAL NOTES

- 1. ALL WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR CONSTRUCTION OF ROADS AND BRIDGES ON FEDERAL HIGHWAY PROJECTS (FP-03), AND THE SUPPLEMENTAL SPECIFICATIONS FOR THIS PROJECT.
- 2. ALL PERMANENT AND TEMPORARY ROADSIDE SIGNS SHALL BE PLACED IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) FOR STREETS AND HIGHWAYS (LATEST EDITION AND AMENDMENTS) AND IN ACCORDANCE WITH THE DETAILS ON THESE PLANS.
- 3. THE DETAILS SHOWN FOR TEMPORARY TRAFFIC CONTROL REFLECTS GENERAL REQUIREMENTS FOR THIS PROJECT. THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND SUBMITTING A TRAFFIC CONTROL PLAN (TCP)
  IN ACCORDANCE WITH THESE DETAILS, THE CONTRACTOR'S CONSTRUCTION SEQUENCING PLAN, MUTCD, AND THE SUPPLEMENTAL SPECIFICATIONS FOR SECTION 635, TEMPORARY TRAFFIC CONTROL. THE CONTRACTOR SHALL SUBMIT A SEPARATE TRAFFIC CONTROL PLAN, FOR REVIEW AND APPROVAL TO THE APPROPRIATE STATE HIGHWAY AGENCY( WITH A COPY SENT TO THE COR) FOR WORK WITHIN THE STATE HIGHWAY RIGHT OF WAY.
- 4. THE DESIGN FEATURES INCLUDING HORIZONTAL AND VERTICAL ALIGNMENTS, TYPICAL SECTIONS, TURNOUTS, AND OTHER DESIGN DETAILS SHOWN ON THESE DESIGN PLANS SHALL NOT BE ALTERED OR MODIFIED IN ANYWAY DURING CONSTRUCTION WITHOUT THE EXPRESSED WRITTEN DIRECTION AND APPROVAL OF THE REGIONAL ROAD ENGINEER AND/OR CONTRACTING OFFICER. DRAINAGE STRUCTURES SHALL BE INSTALLED AS SHOWN ON THE PLANS WITH ONLY MINOR CORRECTIONS IN LOCATION, SKEW AND/OR ELEVATIONS AS NEEDED TO FIT FIELD CONDITIONS.
- 5. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY AND EXPENSE FOR DISPOSAL OF TRASH AND/OR CONSTRUCTION DEBRIS IN ACCORDANCE WITH SECTIONS 107 AND 203 OF THE FP-03. THIS WORK SHALL BE INCIDENTAL OBLIGATIONS OF THE CONTRACTOR.
- 6. THE CONTRACTOR SHALL READ AND MAKE CAREFUL EXAMINATION OF THE PLANS, SPECIFICATION, QUANTITIES, MATERIALS AND VISIT THE SITE OF THE PROPOSED CONSTRUCTION TO BECOME FAMILIAR WITH THE SITE CONDITIONS AND LIMITATIONS BEFORE MAKING A PROPOSAL. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY AND ALL ERRORS RESULTING FROM THE FAILURE TO MAKE SUCH AN EXAMINATION. ANY INFORMATION DERIVED FROM THE MAPS, PLANS, SPECIFICATIONS, PROFILES, DRAWINGS OR THE ENGINEER, WILL NOT RELIEVE THE CONTRACTOR FROM ANY RISK OR FROM FULFILLING THE TERMS OF THE CONTRACT.
- 7. NO WORK SHALL BE PERFORMED OUTSIDE OF THE DESIGNATED CONSTRUCTION LIMITS WITHOUT APPROVAL. IN NO CASE SHALL ANY WORK BE PERFORMED OUTSIDE THE DESIGNATED RIGHT-OF-WAY LIMITS WITHOUT WRITTEN APPROVAL FROM THE REGIONAL DIVISION MANAGER.
- 8. THE QUANTITIES SHOWN ARE FOR ESTIMATING PURPOSES ONLY AND TO COMPARE AND CANVAS BIDS. ACTUAL PAY QUANTITIES WILL BE DETERMINED IN THE FIELD FOR AUTHORIZED CHANGES THAT AFFECT THE QUANTITIES. EARTHWORK QUANTITIES SHALL BE BASED ON FINAL CROSS SECTION SURVEY DATA SUBMITTED TO THE AOTR/COR IN AN APPROVED FORMAT. ANY OVER-RUN OR UNDER-RUN OF QUANTITIES SHALL BE SUBJECT TO FAR 52.211–18.
- 9. ALL TURNOUTS AND DRIVEWAYS SHALL EITHER BE CONSTRUCTED, REBUILT AND/OR RESHAPED OR REMOVED UP TO THE R/W LIMITS AS SHOWN ON THESE PLANS AND PAVED TO THE R/W LIMITS AS REQUIRED. THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE APPROPRIATE EARTHWORK AND PAVING BID ITEMS SHOWN IN THE BID SCHEDULE.
- 10. STRUCTURAL EXCAVATION AND BACKFILL OF CULVERTS AND OTHER DRAINAGE STRUCTURES SHALL BE CONSIDERED INCIDENTAL TO INSTALLATION OF THE STRUCTURE. EXCESS MATERIAL REMOVED MAY BE USED TO REBUILD TURNOUTS OR PLACED ALONG ROADWAY SHOULDERS IN AREAS DESIGNATED AND AS DIRECTED BY THE COR/AOTR.
- 11. ALL FURROW DITCHES AND DRAINAGE DITCHES SHALL BE STAKED AND GRADED TO DRAIN UP TO THE RIGHT-OF-WAY LIMITS. DITCH BLOCKS, DIKES AND DITCHES MAY BE ADDED AT LOCATIONS DESIGNATED BY
  THE COR/AOTR AND/OR AS SHOWN ON THESE PLANS. ALL DITCH BLOCKS, DIKES AND FURROW DITCHES SHALL BE PAID FOR UNDER THE APPROPRIATE BID ITEMS FOR THIS WORK AS SHOWN IN THE BID SCHEDULE.
- 12. IMMEDIATELY PRIOR TO PLACING AGGREGATE BASE COURSE, THE TOP 150 mm OF FINISHED SUBGRADE SHALL BE CHECKED FOR COMPACTION AND GRADE. IF COMPACTION DOES NOT MEET THE MINIMUM SPECIFIED COMPACTION REQUIREMENTS, THE SUBGRADE SHALL BE WATERED AND/OR SCARIFIED AS NEEDED AND RE-COMPACTED TO THE REQUIRED DENSITY. IN NO CASE SHALL ANY EMBANKMENT OR SURFACING MATERIAL BE PLACED ON FROZEN, MUDDY OR UNSTABLE NATURAL GROUND. THIS WORK SHALL BE CONSIDERED AN INCIDENTAL OBLIGATION OF THE CONTRACTOR.
- 13. THE EARTHWORK TABLE SHOWN IS TO ASSIST THE CONTRACTOR IN ESTABLISHING A BID UNDER THE EARTHWORK ITEMS SHOWN IN THE BID SCHEDULE. ANY BORROW MATERIAL CALLED FOR ON THE PLANS SHALL BE TAKEN FROM CONTRACTOR IDENTIFIED SOURCES OUTSIDE THE RIGHT-OF-WAY LIMITS. IT IS THE SOLE RESPONSIBILITY AND EXPENSE OF THE CONTRACTOR TO PROVIDE ANY NECESSARY BORROW MATERIAL FOR THIS PROJECT INCLUDING ALL NECESSARY PERMITS. ALL EXCAVATION, WASTE AND EMBANKMENT MATERIAL SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE EARTHWORK BID ITEMS SHOWN. ANY WASTE MATERIAL SHOWN ON THESE PLANS SHALL BE USED AS NECESSARY TO CONSTRUCT TURNOUTS AND DITCH BLOCKS AND BE PLACED AS EMBANKMENT ALONG THE SHOULDERS IN AREAS AS DIRECTED BY THE COR/AOTR.
- 14. THE LOCATION OF UTILITIES SHOWN ON SHEET 3 & 4 ARE APPROXIMATE AND ARE ONLY TO ASSIST THE CONTRACTOR IN COMPLETING THE WORK. THE CONTRACTOR SHALL VERIFY ALL UTILITIES WITH THE OWNERS PRIOR TO CONSTRUCTION. ANY DAMAGE TO ANY UTILITIES THROUGH THE NEGLIGENCE OF THE CONTRACTOR SHALL BE RESTORED TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.
- 15. THE CONTRACTOR SHALL REMOVE AND CLEAN AND STOCKPILE ALL EXISTING CULVERTS, CATTLEGUARD AND FENCING MATERIALS AS CALLED FOR ON THESE PLANS AND SECTIONS 203 AND 607. ALL SALVAGEABLE MATERIALS AS DETERMINED BY THE COR/AOTR SHALL BE TAKEN TO THE SHIPROCK AGENCY MAINTENANCE YARD AND STOCKPILED. ANY MATERIALS DETERMINED TO BE UNUSABLE BY THE COR/AOTR SHALL BE DISPOSED OF BY THE CONTRACTOR IN ACCORDANCE WITH SECTIONS 107 AND 203. THIS WORK SHALL BE INCLUDED IN THE APPROPRIATE UNIT PRICE BID ITEMS FOR SECTIONS 203 AND/OR 607 (WHERE APPLICABLE).

- 16. THE CONTRACTOR SHALL BE REQUIRED TO REPAIR ALL DENTED, BENT OR OTHERWISE DAMAGED PIPE ENDS OR PIPE END SECTIONS. THIS WORK SHALL MEET THE APPROVAL OF THE CONTRACTING OFFICER, AND IS EXPECTED TO INCLUDE STRAIGHTENING OF DENTED/BENTED CULVERT ENDS, WELDING OF CUTS/TEARS IN THE EXISTING CULVERTS, ETC. NO SEPARATE PAYMENT FOR THIS WORK WILL BE MADE. THE CONTRACTOR SHALL CONSIDER THESE REPAIRS INCIDENTAL TO THE CONTRACT.
- 17. THE ROADWAY TYPICAL SECTION SHOWN IS THE BASIC TEMPLATE TO WHICH THE PROJECT IS TO BE STAKED AND BUILT. THERE WILL HOWEVER, BE LOCATIONS WHERE, DUE TO EXISTING GROUND CONDITIONS, TURNOUTS, CULVERTS OR OTHER STRUCTURES, ETC., THE SHOWN TYPICAL SECTION CAN NOT BE CONSTRUCTED. THE FINAL CONSTRUCTED ROAD SECTION SHALL BE BASED ON THE COMPUTERIZED STAKING NOTES AS ADJUSTED TO FIT FIELD CONDITIONS.
- 18. THE CONTRACTOR SHALL BE REQUIRED TO SAW CUT THE EXISTING ASPHALT PAVEMENT WHERE OLD ASPHALT IS TO TIE INTO THE NEW ASPHALT PAVEMENT. THE CONTRACTOR SHALL BE REQUIRED TO TAPER THE NEW ASPHALTIC CONCRETE SURFACING TO MATCH EXISTING PAVEMENT SECTION AT TIE-IN POINTS AND TO PROVIDE FOR A SMOOTH TRANSITION AS DIRECTED BY THE COR/AOTR THIS WORK SHALL BE INCIDENTAL TO THE PAVING ITEMS SHOWN.
- 19. AT THE COMPLETION OF THIS PROJECT, THE CONTRACTOR SHALL RESHAPE THE IRRIGATION CHANNEL TO MATCH ADJACENT CHANNEL SHAPE. THE COR/AOTR SHALL INVITE THE NAVAJO NATION DEPARTMENT OF WATER RESOURCES & SHIPROCK IRRIGATION OFFICE TO THE FINAL INSPECTION
- 20. IN NO CASE SHALL ANY CALLED FOR DRAINAGE STRUCTURE(S) BE INSTALLED BELOW THE NATURAL FLOW LINE OF THE WASH OR DITCH. THE CONTRACTOR AND COR/AOTR SHALL ADJUST THE LENGTH OF PIPE(S) TO MATCH FIELD CONDITIONS AND SUBMIT A REVISED PIPE LIST TO THE PLANNING & DESIGN BRANCH CHIEF FOR REVIEW AND APPROVAL BEFORE THE CONTRACTOR ORDERS THE MATERIAL.
- 21. EXISTING CATTLE GUARDS, CULVERTS, AND ROADSIDE SIGNS, OR OTHER IMPROVEMENTS NEGLIGENTLY DAMAGED BY THE CONTRACTOR, DURING CONSTRUCTION, SHALL BE RESTORED IN EQUAL OR BETTER CONDITION AT THE CONTRACTOR'S EXPENSE.
- 22. REMOVAL AND REATTACHMENT OF BARBED-WIRE FENCING AT ALL MAJOR DRAINAGE PIPES AND CATTLEGUARDS SHALL BE CONSIDERED INCIDENTAL OBLIGATIONS OF THE CONTRACTOR, FOR WORK REQUIRING REMOVAL AND/OR EXTENSION OF EXISTING STRUCTURES.
- 23. THE CONTRACTOR SHALL REMOVE ALL EXISTING ROADSIDE SIGNS AT THE TIME NEW PERMANENT ROADSIDE SIGNS ARE INSTALLED OR IF THEY ARE IN THE WAY OF CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY THE COR/AOTR AT LEAST THREE (3) WORKING DAYS IN ADVANCE OF SUCH SIGN REMOVAL.
- 24. CONSTRUCTION SURVEY STAKING SHALL BE IN ACCORDANCE WITH SECTION 152 OF THE FP-03. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE GOVERNMENT FURNISHED REFERENCE AND CONTROL POINTS DURING CONSTRUCTION. THE COST OF ANY GOVERNMENT RE-STAKING DUE TO THE NEGLIGENCE OF THE CONTRACTOR SHALL BE DEDUCTED FROM THE CONTRACTOR'S PROGRESS PAYMENTS.
- 25. AT ALL DRAINAGE PIPE REPLACEMENTS AND IN-PLACE PIPE CLEANING LOCATIONS, THE CONTRACTOR SHALL CLEAN AND REGRADE, RESHAPE THE INLET AND OUTLET CHANNELS TO THE RIGHT-OF-WAY LINE AS DIRECTED BY THE CONTRACTING OFFICER. THIS WORK SHALL BE INCIDENTAL TO BID ITEM 60202-0510.
- 26. ALL RIGHT-OF-WAY REFERENCE MARKERS SHALL BE LABELED IN THE METRIC AND ENGLISH UNITS OF MEASURE.
- 27. DUE TO NARROW RIGHT-OF-WAY WIDTHS, THE COMPLETE TURNOUT RADIUS CAN NOT BE CONSTRUCTED AT NUMEROUS TURNOUTS. AT THESE LOCATIONS THE CONTRACTOR SHALL CONSTRUCT THE PLANNED RADIUS, BUT END THE RADIUS AT THE RIGHT-OF-WAY LIMIT. THE ACTUAL TURNOUT WIDTH (AT THE RIGHT-OF-WAY LINE) WILL BE WIDER THAN THE WIDTH GIVEN ON THE PLANS.
- 28. THE EXISTING MAIL BOXES LOCATED ALONG THE ROADWAY PRISM SHALL BE REMOVE AND REINSTALLED OUTSIDE OF THE RIGHT-OF-WAY LIMIT. THIS WORK SHALL BE INCIDENTAL TO BID ITEM 20304-0000.
- 29. AT THE COMPLETION OF THE PROJECT, THE CONTRACTOR SHALL INSPECT THE INTERIOR OF ALL NEWLY INSTALLED CULVERTS AND EXISTING DRAINAGE PIPES SPECIFIED IN THE PLANS TO BE CLEANED. THESE CULVERTS SHALL BE FREE OF SILT AND OTHER DEBRIS PRIOR TO THE FINAL PROJECT INSPECTION. THE CULVERTS SHALL BE MAINTAINED IN A "CLEAN" CONDITION UNTIL THE PROJECT IS ACCEPTED BY THE CONTRACTING OFFICER.

NOTE: THE BRIDGE SHALL BE CONSTRUCTED DURING THE WINTER MONTHS WHEN THE CANAL IS SHUT OFF. THE CANAL SHUT OFF DATES VARY DEPENDING ON THE WEATHER BUT ARE APPROXIMATELY FROM NOVEMBER 15 TO MARCH 31, EACH YEAR. BRIDGE CONSTRUCTION SHALL BE COMPLETED IN ONE SHUT OFF SEASON. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY WEATHER PROTECTION THAT MAY BE NECESSARY TO COMPLETE CONSTRUCTION DURING THE WINTER, INCLUDING EARTHWORK AND COMPACTION. THIS INCLUDES PROTECTION FROM FREEZING AND MOISTURE.

			150 mm-Mainline	AND CENEDAL MOTES
DESCRIPTION	GRADE	UNIT WT.	APPLICATION	TYPICAL ROADWAY SECTION
DASIS UF	ESTIMATED Q	UANTITIES		TYDICAL DOADWAY CECTION
DACIC OF	ESTIMATED O	LIANITITIES		NAVAJO REGIONAL OFFICE — DIVISION OF TRANSPORTATION
				BUREAU OF INDIAN AFFAIRS
				I DEPARTMENT OF THE INTERIOR
				I UNITED STATES

TEM NUMBER	DESCRIPTION	GRADE	UNIT WT.	APPLICATION	TYPICAL ROADWAY S	
30101-2000	Aggregate Base Grading D	D	2,244 kg/m <sup>3</sup>	150 mm—Mainline 102 mm Turnouts	AND GENERAL NO	<u>TE</u>
40201-0500	Hot Asphaltic Conc. Pvmt. Marshall Test Class B Grading B	В	2,404 kg/m <sup>3</sup>	Two 38 mm lifts — Mainline, One 51 mm lift — Turnouts.	Designed by: HRC	4
40930-0600	Asphalt Cement Grade PG 58-28	PG 58-28	0.9806 L/kg	5.5% by wt. of total weight of mixture	Drawn by: TAY, cdh Date: 08/08/11	<u> </u>
41101-5000	Prime Coat Grade MC-70	MC-70	1.056 L/m <sup>2</sup>	1.36 L/m <sup>2</sup>	Checked by: EV, cdh Date: 10/27/11	」 \
41201-1000	Asphalt Emulsion Tack Coat Grade SS-1	SS-1	1.000 L/m <sup>2</sup>	0.23 L/m <sup>2</sup>	File Name: 02_Rdtyp	



### BRIDGE AND ROADWAY ESTIMATED QUANTITIES

t	BRIDGE AND ROADWAY ESTIMATED C	QUANIII	IF2	
ITEM	DESCRIPTION	QUANTITY	UNIT	AS BUIL
10901-0000	Extra and Miscellaneous Work Authorized Under Section 109.02(m)	All Req'd	Lump Sum	
15101-0000	Mobilization	All Req'd	Lump Sum	
15201-0000	Construction Survey and Staking	All Req'd	Lump Sum	
15301-0020	Contractor Quality Control	4800	Man-Hour	
15701-0000	Soil Erosion Control	All Req'd	Lump Sum	
15708-1000	Soil Erosion Control, Temporary Straw Mulch	0.72	Hectare	
20102-0000	Clearing and Grubbing	All Req'd	Lump Sum	
20304-1000	Removal of Structures and Obstructions	All Req'd	Lump Sum	
20401-0000	Roadway Excavation	222	Cubic Meter	
20403-0000	Unclassified Borrow	1460	Cubic Meter	
20601-0000	Development of Water Supply	0.58	Mega Liter	
20801-0000	Structure Excavation	46	Cubic Meter	
20803-0000	Structure Backfill	21	Cubic Meter	
30101-2000	Aggregate Base Grading D	1230	Metric Ton	
40201-0500	Hot Asphalt Concrete Pavement Marshall Test Class B , Grading B	610	Metric Ton	
40930-0600	Asphalt Cement Grade, PG 58-28	32	Metric Ton	
41101-5000	Prime Coat, Grade MC-70	7.63	Metric Ton	
41201-1000	Asphalt Emulsion, Tack Coat, Grade SS-1	0.76	Metric Ton	
55101-1200	HP 250 x 62 Steel Piles, In Place	140	Meter	
55115-1000	Preboring	130	Meter	
55201-0200	Structural Concrete Class A(AE)	59	Cubic Meter	
55301-0500	Precast, Prestressed Concrete Members, Voided Slab (SIII-48)	9	Each	
55401-1000	Reinforcing Steel	3255	Kilogram	
55401-2000	Reinforcing Steel, Epoxy Coated	4032	Kilogram	
55601-0900	Bridge Rail, Steel	26.7	Meter	
60202-0510	711 mm Span, 508 mm Rise Corrugated Steel Pipe Arch	65	Meter	
60211-0910	End Section for 711 mm Span, 508 mm Rise Corrugated Steel Pipe Arch	8	Each	
61701-4100	Guardrail System SGR04b, Type PDE02 w/ CRT End Treatment	74	Meter	
61701-5000	Guardrail System SGR04b, Type PDE02 w/ ET-PLUS End Treatment	99	Meter	
61902-1300	Swing Gates	4	Each	
61903-0310	Cattle Guard, 4900 mm with Type II Gate	2	Each	
61903-0711	Cattle Guard, 7190 mm without gate	1	Each	
61903–1011	Cattle Guard, 9480 mm without gate	1	Each	
61921-2000	Remove and Replace Fence With Chain Link	670	Meter	
62101-0000	Right of Way Monument	14	Each	
62102-0000	Reference Marker	14	Each	
62510-1000	Seeding, Dry Method	0.72	Hectare	
63302-0003	Sign Installation, 1 Post and Hardware: 4.10 kg/m	1.20	Square Meter	
63302-0010	Sign Installation, 2 Posts and Hardware: 2.98 kg/m	0.92	Square Meter	
63308-2000	Object Marker, Glass Fiber, Type 2	2	Each	
63308-3000	Object Marker, Type 3 w/1 Post and Hardware 2.98kg/m	4	Each	
63309-0020	Delineators, Glass Fiber, Type 1b	11	Each	
63401-1510	Pavement Markings, Type H, Solid Yellow	686	Meter	
63401-1520	Pavement Markings, Type H, Solid White	571	Meter	
63501-0000	Temporary Traffic Control	All Req'd	Lump Sum	
63506-0500	Temporary Traffic Control, Flagger	320	Man-Hour	
		l	ı	<u> </u>

# ITEM 20401-0000: EARTHWORK QUANTITIES

11Em 20101 000				
STATION - STATION	CUT (m³)	* FILL (m³)	BORROW (m³)	WASTE (m³)
10+006.998 to 10+143.346	65.65	271.56	205.91	
10+156.654 to 10+350.000	155.47	1409.29	1253.82	
TOTAL	221.12	1680.85	1459.73	
	STATION - STATION  10+006.998 to 10+143.346  10+156.654 to 10+350.000	STATION – STATION CUT (m³)  10+006.998 to 10+143.346 65.65  10+156.654 to 10+350.000 155.47	STATION - STATION       CUT (m³)       * FILL (m³)         10+006.998 to 10+143.346       65.65       271.56         10+156.654 to 10+350.000       155.47       1409.29	STATION - STATION         CUT (m³)         * FILL (m³)         BORROW (m³)           10+006.998 to 10+143.346         65.65         271.56         205.91           10+156.654 to 10+350.000         155.47         1409.29         1253.82

<sup>\*</sup> Assumed 25% fill factor

STATION — STATION	CUT (m³)	* FILL (m³)	BORROW (m³)	WASTE (m³)
10+006.998 to 10+143.346	65.65	271.56	205.91	
10+156.654 to 10+350.000	155.47	1409.29	1253.82	
TOTAL	221.12	1680.85	1459.73	

# ITEM 61903-0310: 2-UNIT CATTLE GUARD AND TURNOUT PAVING LOCATIONS

STATION	LOCATION	TURNOUT SIZE (m)	REMARKS
10+219.328	Rt.	4.50 x 18.2m	Construct New Turn Out to R/W W/Type 2 Gate
10+271.718	Rt.	4.50 x 21.0m	Construct New Turn Out @ 30° Skew to R/W W/Type 2 Gate

## ITEM 61903-0710: 3-UNIT CATTLE GUARD W/ TYPE II GATE AND TURNOUT PAVING LOCATIONS

STATION	LOCATION	TURNOUT SIZE (m)	REMARKS
10+336.969	Lt.	7.00 x 18.2m	Construct New Turn Out to R/W, W/Type 2 Gate

### ITEM 61903-1011: 4-UNIT CATTLE GUARD

STATION	LOCATION	REMARKS
10+350.000	CL	Without Gate

### TURNOUT LOCATIONS: GRADE AND PAVE ONLY

STATION	LOCATION	SIZE (m)	REMARKS
10+049.127	Rt.	4.50 x 8.000m	Construct New Turn Out to R/W W/1—Swing Gate
10+108.824	Rt.	4.50 x 8.000m	Construct New Turn Out to R/W W/1—Swing Gate
10+083.212	Lt.	4.50 x 8.000m	Construct New Turn Out to R/W W/1—Swing Gate
10+098.967	Lt.	4.50 x 8.000m	Construct New Turn Out to R/W W/1—Swing Gate
10+134.661	Rt.	4.50 x 23.000m	Construct New Turn Out to R/W
10+137.912	Lt.	4.50 x 23.000m	Construct New Turn Out to R/W
10+161.991	Rt.	4.50 x 23.000m	Construct New Turn Out to R/W
10+163.508	Lt.	4.50 x 23.000m	Construct New Turn Out to R/W

### **EXISTING UTILITY CROSSING**

STATION	LOC.	OWNER	DESCRIPTION	SKEW	REMARKS
10+000 - 10+128.009	Left	NTUA	Over Head Power		To Remain in Place
10+06.515	Ģ	NTUA	Over Head Power	90°	To Remain in Place
10+010.935	Ą	NTUA	Over Head Power	59°	To Remain in Place
10+023.506	Ą	NTUA	Over Head Power	76°	To Remain in Place
10+070.566	Ą	NTUA	Over Head Power	90°	To Remain in Place

## ITEM 63401-1510: Pavement Markings, Type "H". Solid Yellow

·/ [- · · · · · · · · · · · · · · · · · ·			
Station	Location	Length	Unit
10+006.969-10+350.000	Centerline, Rt.	343.031	m
10+006.969-10+350.000	Centerline, Lt.	343.031	m
	ΤΩΤΔΙ	686 062	m

# 62101-0000 & 62102-0000: R/W MONUMENTS AND REFERENCE MARKERS

STATION	OFFSET (m)	LOCATION	REQUIRED
10+006.969	8	Left & Right	2
10+090.560	8	Left & Right	2
10+109.441	8	Left & Right	2
10+130.000	8	Left & Right	2
10+130.000	23	Left & Right	2
10+300.000	23	Left & Right	2
10+350.000	23	Left & Right	2
TOTAL REQ'D :			14

# ITEM 60202-0510: STEEL PIPE ARCH

	_	
STATION	SIZE	REMARKS
10+210.000, CL	1-711 mm x 508mm x 22.82m	150° SKEW
10+219.328, Rt.	1-711 mm x 508mm x 11.73m	TURNOUT
10+271.718, Rt.	1-711 mm x 508mm x 16.12m	TURNOUT
10+336.969, Lt.	1-711 mm x 508mm x 13.55m	TURNOUT
SUBTOTAL (m) :	64.22m	

### ITEM 20304-1000: REMOVAL OF STRUCTURES AND OBSTRUCTIONS

<u> </u>	<del></del>	
STATION	LOCATION	DESCRIPTION
10+145 to 10+155	Centerline	Remove existing bridge and related foundations
10+219.328	Right	Remove and salvage existing 305mm culvert

### ITEM 61701-4100: GUARDRAIL SYSTEM SGR04(b), Type PDE02 with CRT END TREATMENT

STATION	LOCATION	LENGTH (m)
10+130.458 to 10+131.241	RT.	11.688
10+132.302 to 10+134.969	LT.	11.688
10+138.816 to 10+142.159	RT.	12.642
10+140.763 to 10+144.105	LT.	12.642
10+155.895 to 10+159.237	RT.	12.642
10+157.841 to 10+161.184	LT.	12.642
TOTAL		73.944

ITEM 63401-1520: Pavement Markings,

Location

Lt. Shoulder

Rt. Shoulder

1- 7.00m TO

10- 4.50m TO's

ITEM 61701-5000: GUARDRAIL

SYSTEM SGR04(b), Type PDE02 with ET-PLUS END TREATMENT

TOTAL

TOTAL

Length

343

343

-15

-100

571

LOCATION

RT.

LT.

LT.

Unit

LENGTH (m)

15.240

15.240

34.290

34.290

99.060

Type "H", Solid White

STATION

10+115.217 to 10+130.458

10+117.061 to 10+132.302

10+168.847 to 10+203.139

10+168.847 to 10+203.139

10+006.969-10+350.000

10+006.969-10+350.000

# **BRIDGE GENERAL NOTES**

ROUTE

N5031

PROJECT

N5031(1)1,2&4

TOTAL SHEETS

32

SHEET

1. SPECIFICATIONS: Design; AASHTO Standard Specifications for Highway Bridges, 1996, 16th Ed. w/ Interims thru 2000 Construction; Standard Specifications for Construction of Roads and Bridges on Federal Highway Projects, FP-03 and Supplemental Specifications.

RESERVATION

2. UNITS: This project has been designed and drawn using the SI (metric) system of units.

STATE

Navajo

- 3. DESIGN LOADS: Dead Loads; Concrete = 23.56 kN/m<sup>3</sup>, Steel = 76.97 kN/m<sup>3</sup>, Paving Allowance = 1.20 kPa of roadway Surface, Backfill Earth Pressure = 4.7 kPa/m LIVE LOADS; MS 18 plus impact. Impact = 15/L + 38 where L = span length in meters. Maximum Impact factor = 0.30.
- 4. RATINGS: Inventory Rating = MS 19.1. Operating Rating = MS 31.9.
- 5. DESIGN & CONSTRUCTION: Superstructure designed using AASHTO Load Factor Design (LFD) and substructure (including Bearings) using Service Design. Material strengths are f'c = 27.6 MPa for reinforced concrete, Fy = 413.7 MPa for reinforcing steel and Fy = 248.2MPa for H-piling. Prestressed beams designed in accordance with current AASHTO design criteria. Material strengths are f'c = 41.4 MPa for prestressed concrete and Fs = 1861.6 MPa for prestressing steel. The slabs are "fixed" to the Abutments, but the abutment piles are free to flex, in order to take any temperature movement.
- 6. HOT ASPHALTIC CONCRETE PAVEMENT (HACP) OVERLAY: HACP shall conform to Section 402 of the FP-03. Water proofing membrane shall be the Polyguard 665 membrane manufactured by Polyguard Products, Inc., the GeoTac Membrane manufactured by Contech Construction Products, Inc., or an approved equal. A primer or adhesive shall be applied prior to water proof membrane as recommended by the manufacturer. See Deck Plan and Section for overlay placement details. Quantities for bridge overlay HACP shall be included in Item 40201-0500. Water proofing membrane, adhesive and sand blasting shall be considered incidental to Item 40201-0500.
- 7. CONCRETE: Concrete in precast, prestressed concrete voided slabs (S III-48) shall be Class P and shall have a F'ci = 34.50 MPa at release of prestressing strands and the minimum design strength indicated above at 28 days. Cast in place concrete in superstructure and substructure shall be Class A(AE) with the minimum design strength indicated above at 28 days. Cast in place concrete shall be made with Type II Cement. The air content for Class A(AE) concrete shall not be less than that specified in the FP-03. Chamfer exposed corners of all concrete 19 mm unless otherwise shown. The time limits specified in the FP-03 shall apply. If concrete cannot be discharged within the specified time limit, alternatives such as dry batching, a site batching plant conforming to the specifications or retardant admixtures shall be used. If required, such alternatives shall be discussed at pre-construction meeting. Approval of alternative methods shall be based on review of historical data for identical strength concrete placed at similarly remote locations. Historical data shall indicate conformance to the required specifications. The approach slab shall be given a sidewalk finish in accordance with Sections 552.14(2) of the FP-03. Exposed surfaces of the substructure down to 300 mm below the ground line shall be given a Class 2 rubbed finish as specified in Section 552.16 of the FP-03. All other surfaces of concrete shall be given a Class 1 ordinary finish.
- 8. REINFORCING STEEL: All plain reinforcing steel shall conform to AASHTO M31M, Grade 420, and epoxy coated reinforcing shall conform to AASHTO M284M and AASHTO M31M, Grade 420. All reinforcement in deck, or protruding into the deck shall be epoxy coated. The minimum cover of any reinforcing steel shall be 50 mm unless otherwise specified. Lengths of reinforcing steel bars shown include required splice lengths for splices shown. Any additional splices not shown in the plans shall be requested for approval by the Contractor and shall not be utilized until written approval is granted by the CO. Additional reinforcing steel quantities required for additional splices not shown in the plans shall not be paid for. Reinforcing steel sizes shown in these plans are in accordance with AASHTO M31M.
- 9. PRESTRESSING STEEL: Prestressing steel shall be 12.7 mm  $\phi$ , seven wire, low relaxation prestressing steel strands conforming to AASHTO M203M, Grade 1860.
- 10. PRESTRESSED SLABS: Prestressed slabs shall be manufactured as detailed in these plans. All concrete, reinforcing steel, prestressing steel, lifting devices, inserts, bearing plates, elastomeric bearing pads, and any other materials necessary for the fabrication of the prestressed beams shall be considered incidental to Item 55301-0500, Precast, Prestressed Concrete Structural Members, Voided Slabs (SIII-48). Apply a sidewalk finish to the top of the slabs as per Section 552.14(2) of the FP-03. Elastomeric bearing pads shall conform to AASHTO M251M and shall be 60 Durometer hardness.
- 11. STRUCTURAL STEEL: Diaphragm anchor bolts shall conform to AASHTO M164M.
- 12. STEEL PILES: Steel piles shall be HP 250 x 62 for abutments w/ heavy duty pile tips. Piles shall conform to AASHTO M270M, Grade 250. Pile shall be driven to three times the Applied Structural Load shown in the plans. Piles shall be driven utilizing the dynamic formula given in Section 551.06 (b) of the FP-03. The ultimate pile capacity (RU) shall be the Applied Structural Load multiplied by a factor of safety of 3. Piles shall be driven to at least minimum tip elevation required for lateral stability or to penetrate unsuitable strata or deeper to gain the required ultimate pile capacity (RU). Splicing shall be in accordance with Sections 551.08 and 551.09 of the FP-03 except that splice details shall be as shown in the plans. Piles shall be driven to the tolerances given in Section 551.08 of the FP-03. Axial alignment deviations shall be measured starting from the planned pile location at the cutoff elevation and shall not exceed the tolerance given in Section 551.08 of the FP-03. Assure correct pile placement and alignment (within applicable tolerances) by providing horizontal bracing between the crane and pile driving leads. The abutments are integral and designed to flex. Prebore all abutment piles to a depth of 3.000 m below bottom of abutment cap with a diameter of 457 mm. Backfill with Select Granular Backfill (FP-03 704.10) after placement of piles.
- 13. The Contractor shall remove, clean (if so directed) and stockpile all existing salvageable material, as indicated by the AOTR/COR and as called for on these plans under Item 203. Salvageable material shall be transported by the contractor to the Shiprock Agency maintenance yard and stockpiled. Any existing materials determined to be unsalvageable by the AOTR/COR shall be disposed of by the contractor in accordance with Sections 107 and 203 of the FP-03 and supplemental specifications. Any existing piling shall be removed to one meter below planned flowline, or lower, to accommodate new construction. All work involving salvageable and unsalvageable material shall be included in the appropriate unit price for Item 203, as applicable.

# ITEM 61921-2000: Remove and Replace Fence

		<u>.                                      </u>	
STATION TO STATION	LOCATION	LENGTH (m)	REMARKS
10+005 to 10+136	Left	146	Use chain link fence to replace existing fence
10+006 to 10+130	Right	124	Use chain link fence to replace existing fence
10+171 to 10+350	Left	196	Tie Fence to Cattleguard at Station 10+350
10+163 to 10+350	Right	204	Tie Fence to Cattleguard at Station 10+350
	TOTAL	670	

# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

# BRIDGE GENERAL NOTES, QUANTITIES AND TABLES

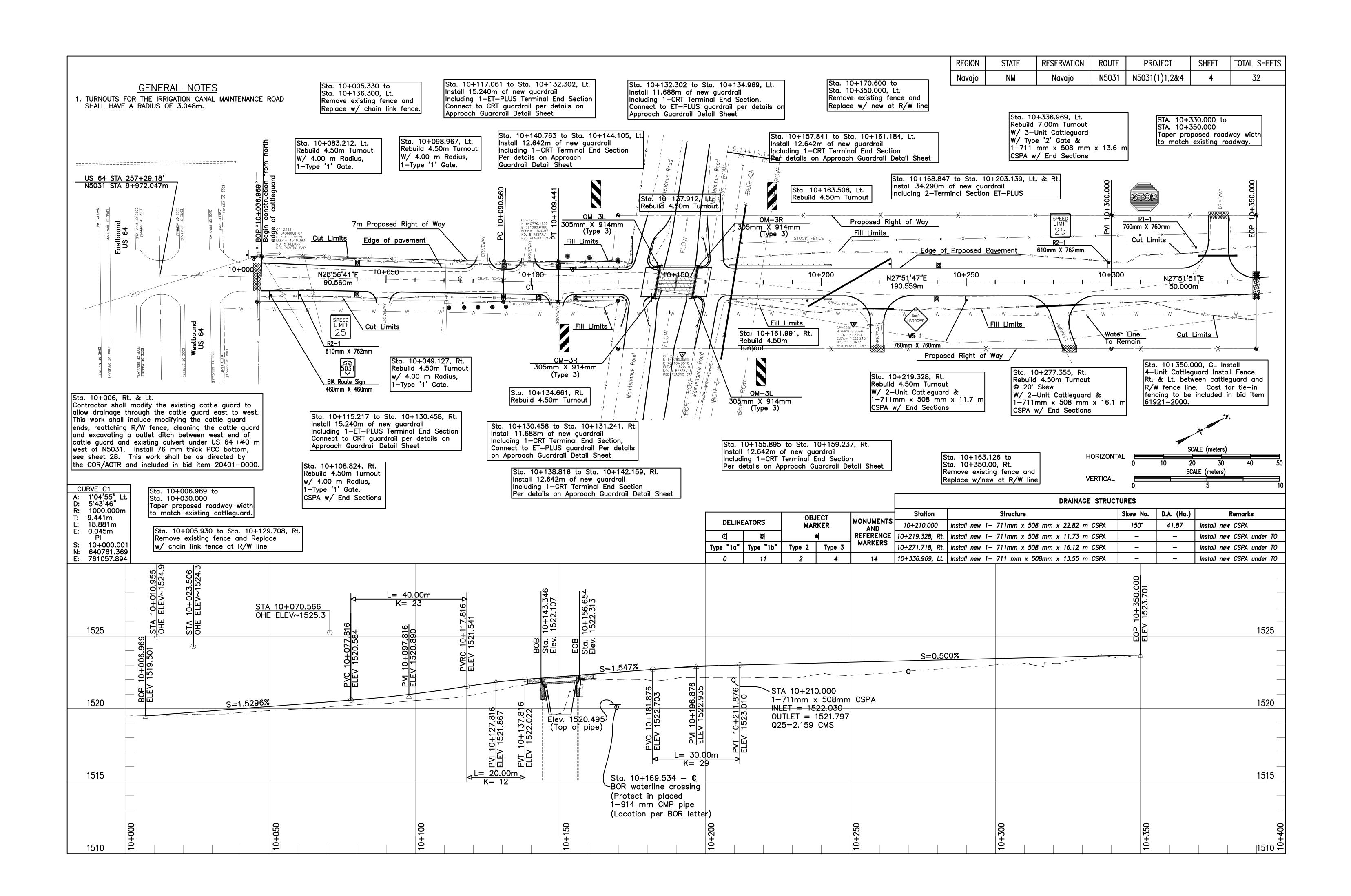
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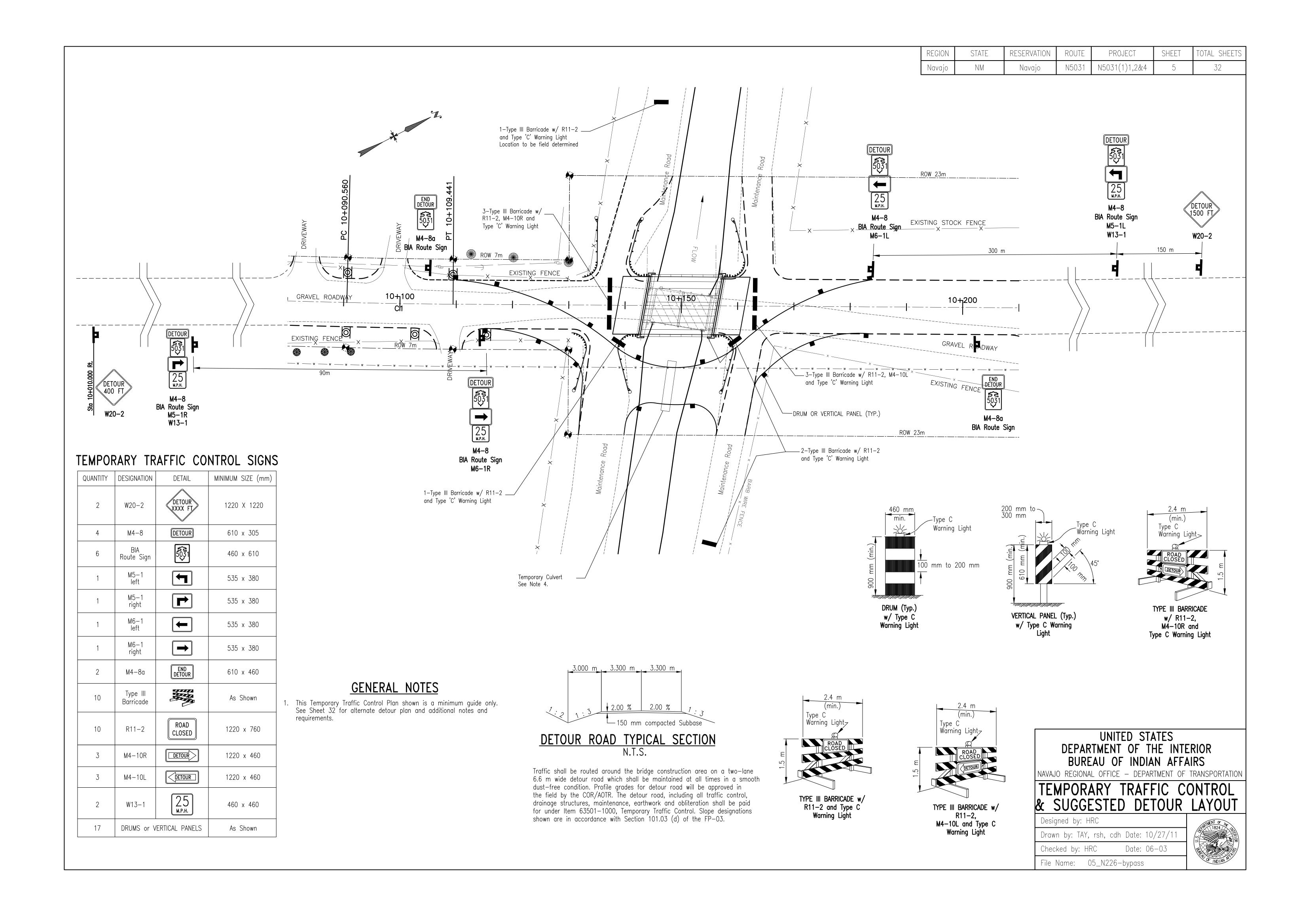
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Drawn by: cdh HRC	Date: 08/09/11

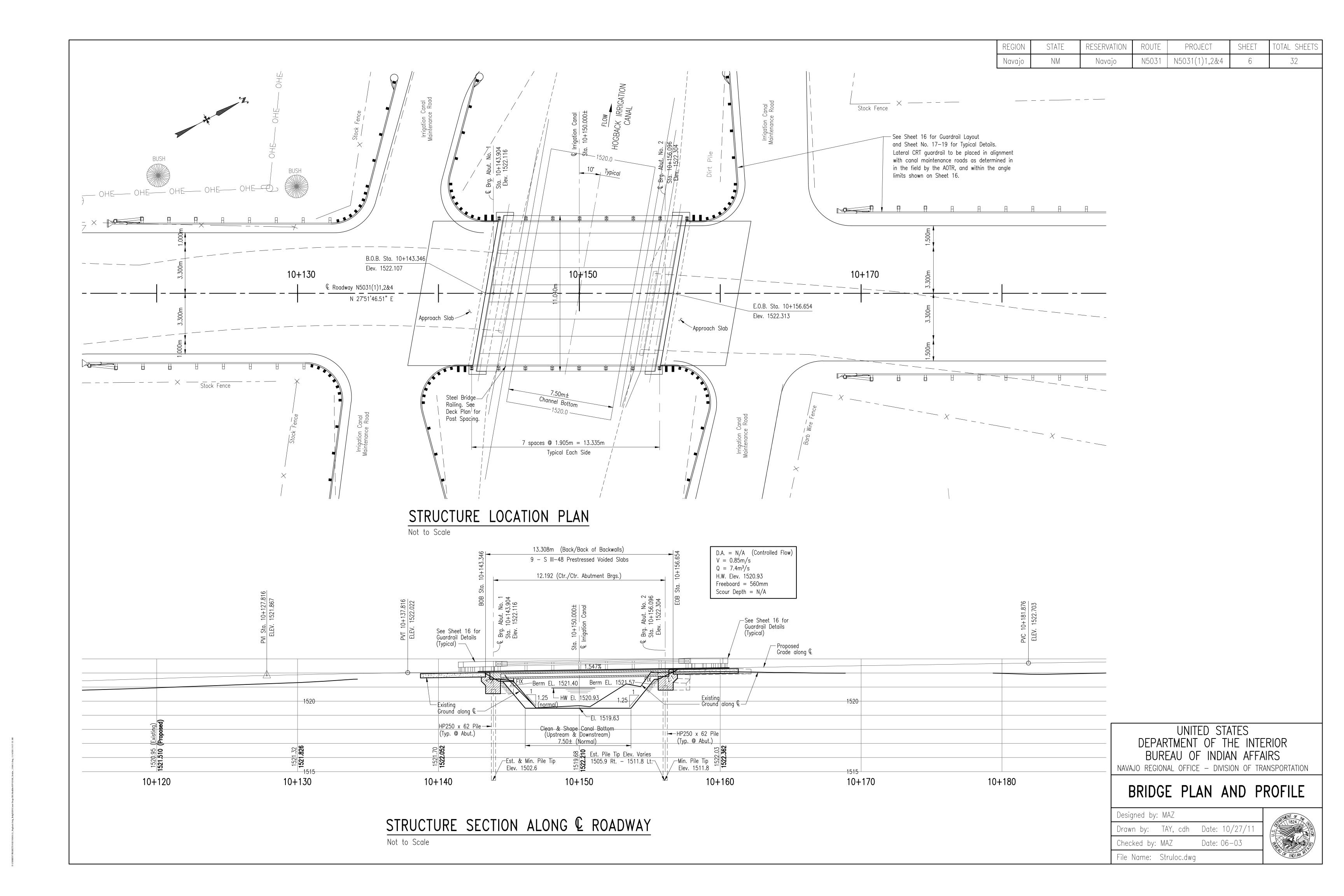
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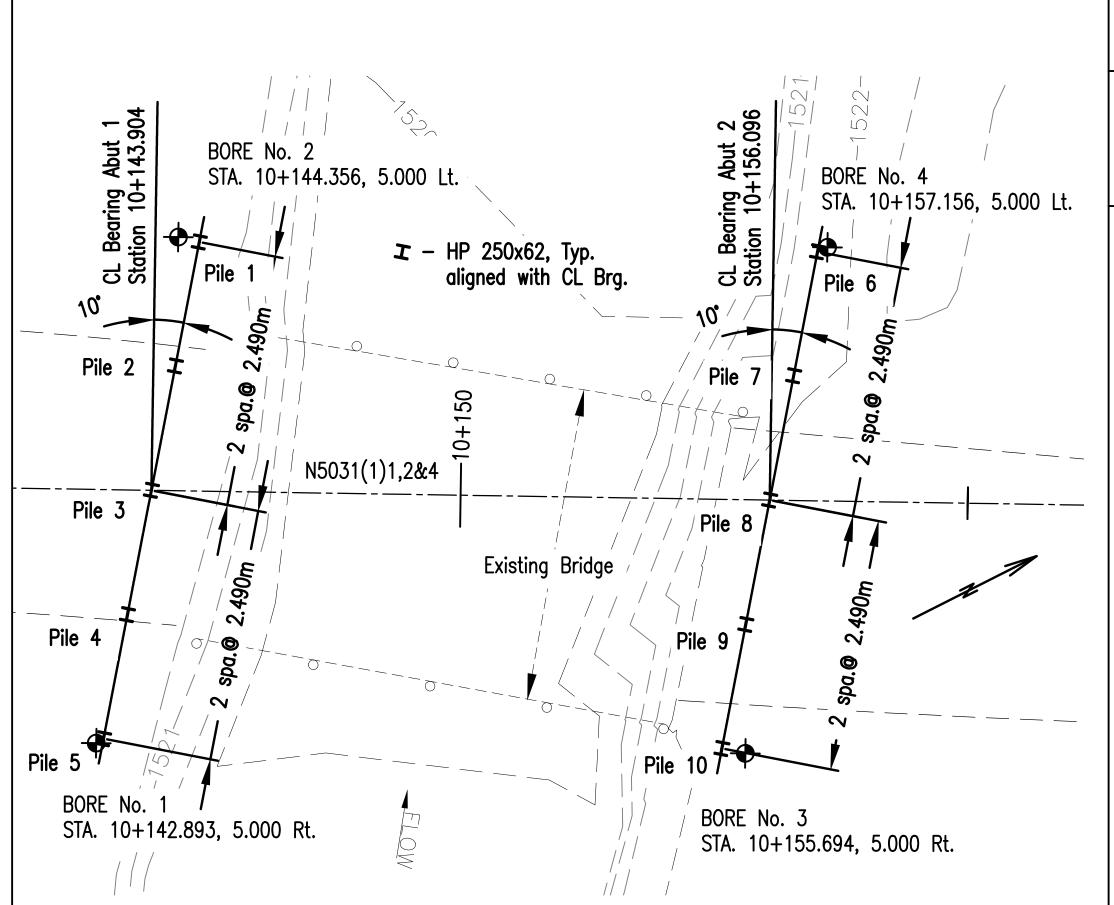
Checked by: cdh EV











BORING LOCATION PLAN

FOUNDATION INFORMATION							
		ABU	TMENT 1	ABUTMENT 2			
PILE TYPE		HP	250x62	HP 250x62			
NO. OF PILES			5	5			
APPLIED LOAD/PII	_E	438 kN 438 kN					
ULTIMATE CAP./PILE 13		1314 kN		1314 kN			
ABUTMENT 1:	PIL	E 1	PILE 2	PILE 3	PIL	E 4	PILE 5
PREBORE ELEV.	150	06.2	1505.7	1505.2	15	04.7	1504.2
MIN. TIP ELEV.	150	05.7	1505.2	1504.7	15	04.2	1503.7
ABUTMENT 2:	PIL	E 6	PILE 7	PILE 8	PIL	E 9	PILE 10
PREBORE ELEV.	15 <sup>-</sup>	14.0	1512.1	1510.2	15	08.3	1506.4
MIN. TIP ELEV.	15 <sup>-</sup>	13.5	1511.6	1509.7	15	07.8	155.9

ORILI	L RIG -	TYPE:	07-30- CME- ZE: HS		X core	BORING NO. 1  ELEVATION: 1522 FIELD ENGR: J. Sandova			<b>O. 1</b> ELEVATION: 1522 FIELD ENGR: J. Sandoval (Western Tech, Inc.
œ					/S/FT.				
WAIEK CONIENI	DRY DENSITY (LBS/CU. FT.)	SAMPLE TYPE	SAMPLE	R or N	С	ОЕРТН	nscs	GRAPHIC	SOIL DESCRIPTION
≥		G					CL-ML		CLAYEY SILT; gray to tan, firm to soft, damp to saturated
	NR	N		6					Groundwater encountered at 5 feet.
						2 —			
		N		0					
		11	<u>'///</u>	0					
						4 —			
		N		0					
	NR	N		8		6 —	GM		
							GIVI	4	COBBLES/GRAVEL; tan, medium dense, saturated, some clayey sand.
		N		40		8 —		4 4	
							CL-ML		
		N	///,	1			01 mi		SANDI SILI, tali, sort, saturatea.
		IN	////	'		10 —			
		N.	///	0			SC-SM		CLAYEY SAND; tan, loose to medium dense, saturated
		N	///	2					trace of gravel 44 to 48 feet.
						12 —			
		N		6					
		N		38		14 —			
							GM		
		N		50/3"				4	clayey sand.
						16 —		A A A	
		N		50/1"			•		SHALE/SANDSTONE; gray to tan, soft to hard, saturated to damp, start NX core with water at 16.763 meters,
		. •		-, '					2.972 meters 97% recovery, 2.489 meters 82% RQD start second core 19.354 meters, 2.912 meters 97% recovery, 2.896 meters 95% RQD, first core fractures
						18 —			1 to 34 inches, second core fractures 1 to 69 inches.
						20 —			
						22 —			

REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	NM	Navajo	N5031	N5031(1)1,2&4	7	31

DATE DRILLED: 07-29-2002  DRILL RIG TYPE: CME-75  BORING TYPE/SIZE: HSA/7"-NX core			ВО	BORING NO. 2  LOCATION: See Boring Location Diagram  ELEVATION: 1522					
	TYPE	_/ SIZ	re: HS						FIELD ENGR: J. Sandoval (Western Tech, Inc.)
WATER CONTENT (%)  DRY DENSITY	(LBS/cu. FT.)	SAMPLE TYPE	SAMPLE	R or N	/S/FT. C	ОЕРТН	nscs	GRAPHIC	SOIL DESCRIPTION
							CL-ML		CLAYEY SILT; tan, Ifirm to soft, damp to saturated.
		N		2		2 —			Groundwater encountered at 5 feet.
		N N		21		4 —	GM.		COBBLES/GRAVEL; tan, medium dense, saturated some clayey sand.
		N N		52 50/5"		6 8	SM		SILTY SAND; tan, loose to medium dense, saturated.
		N N		9		10 —			
		N		13		12 —			
		N N		43 50/2"		14 —	-		SHALE/SANDSTONE; gray, hard, saturated to damp, start NX core with water at 15.300 meters, 2.438 meters 80% recovery, 0.914 meters 30% RQD, start second core 18.348 meters, 2.616 meters 86% recovery, 2.540 meters 83% RQD, first core fractures 1 to 8 inches, second core
						16 —			fractures 3 to 76 inches.
						20 —			UNITED STATES  DEPARTMENT OF THE INTERIOR  BUREAU OF INDIAN AFFAIRS  NAVAJO REGIONAL OFFICE — DIVISION OF TRANSPORTATION
									SOIL PROFILE & BORING PLA (1 OF 2)

Designed by: MAZ Drawn by: cdh TAY Date: 08/09/11 Checked by: cdh MAZ Date: 08/09/11 File Name: Soilpro1.dwg



			07-31- CME-				ВО	RING N	0. 3
				5A/7"-N	X core				ELEVATION: 1522 FIELD ENGR: J. Sandoval (Western Tech, Inc.)
WATER CONTENT (%)	ISITY . FT.)	TYPE		BLOW R	S/FT.				SOIL DESCRIPTION
MATER CO	DRY DENSITY (LBS/CU. FT.)	SAMPLE TYPE	SAMPLE	or N	С	ОЕРТН	SOSN	GRAPHIC	
22.9		G R		1			CL-ML		CLAYEY SILT; gray to tan, firm to soft, damp to saturated. trace of gravel on surface.
22.5	3	N		0		2 —			Groundwater encountered at 5 feet.
		N	1//	0		4 —			
		N		0		4 —			
		N		40		6 —	GP	4 4 4	COBBLES/GRAVEL; tan, medium dense, saturated, some clayey sand.
	NR	N	1//	17		8 —	sc		CLAYEY SAND; tan, loose to medium dense, saturated.
		N		16		10 —			
		N		7					
		N		6		12			
		N	1///	50/3"		14 —	GP	4 4	GRAVEL; tan, medium dense, saturated some clayey sand.
		N	///	50/2"		16 —	•		SHALE/SANDSTONE; gray to tan, soft to hard, saturated to damp, start NX core with water at 14.630 meters, 1.956 meters 80% recovery, 1.473 meters 60% RQD, start second core 17.982 meters, 2.870 meters 94% recovery, 2.718 meters 89% RQD, start third core 21.030 meters, 0.610 meters, 100% recovery, 0.533 meters 87% RQD, first core fractutes 1 to 36 inches, second core fractures 2 to 46 inches, third core fractures 4 to 27 inches.
						18 —			
						20 —			
									Stopped at 21.640 meters

DATE DRILLED: 07-31-2002  DRILL RIG TYPE: CME-75  BORING TYPE/SIZE: HSA/7"-NX core			BORING NO. 4			ELEVATION: 1322			
CONTENT $(\%)$	DENSITY ====================================	SAMPLE TYPE		BLOW R or				O	FIELD ENGR: J. Sandoval (Western Tech, Inc
WATER	DRY DI (LBS/C	SAMPLI	SAMPLE	N		DEPTH	SOSN CL-ML	GRAPHIC	CLAYEY SILT; tan, firm to soft, damp to saturated.
8.3	116	R N		9		2 —			Groundwater encountered at 5 feet.
		N		1		4 —			
		N		4		6 —	SC GP		CLAYEY SAND; tan, loose, saturated.  COBBLES/GRAVEL; tan, medium dense, saturated, some clayey sand.
		N N		17 71		8		4 4 4	SHALE/SANDSTONE; gray to tan, soft to hard, saturated to damp, start NX core with water at 10.362 meters, 2.540 meters 83% recovery, 0787 meters 26% RQD, start second core at 13.411 meters, 3.048 meters 100% recovery, 2.896 meters 95% RQD, first core fractures 1 to 10 inches, second core fractures 1 to 34 inches.
		N	1//	50/4"		10 —			
						12 —			
						14 —			
						16 —			Stopped at 16.458 meters

REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	NM	Navajo	N5031	N5031(1)1,2&4	8	31

UNIFIED SOIL CLASSIFICATION SYSTEM

Soils are visually classified by the Unified Soil Classification system on the boring logs presented in this report. Grain—size analysis and Atterberg Limits Tests are often performed on selected samples to aid in classification. The classification system is briefly outlined in this chart. For a more detailed description of the system, see "The Unified Soil Classification System", Corp of Engineers, U.S. Army Technical Memorandum No. 3—357 (Revised April 1960) or ASTM Designation: D2487—66T.

		MAJOR DIVISIONS		GRAPHIC SYMBOL	GROUP SYMBOL	TYPICAL NAMES		
	or ction ve)	CLEAN G	RAVELS	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	GW	Well graded gravel, gravel—sand mixtures, or sand—gravel—cobble mixtures.		
sieve)	GRAVELS (50% or less of coarse fraction passes No. 4 sieve)	(Less than 5% pass	es No. 200 sieve)		GP	Poorly graded gravels, gravel—sand mixtures, or sand—gravel—cobble mixtures.		
SOILS 5. 200	GRAVELS ss of coa passes No	GRAVEL WITH FINES (More than 12%	Limits plot below "A" line and hatched zone on plasticity chart.		GM	Silty gravel, gravel—sand—silt mixtures.		
COARSE—GRAINED SOILS (Less than 50% passes No. 200	GR less pas	passes No. 200 sieve)	Limits plot above "A" line and hatched zone on plasticity chart.		GC	Clayey gravels, gravel—sand—clay mixtures.		
SE-GR	than fraction sieve)	CLEAN S		SW	Well graded sands, gravelly sands.			
COAR than 5	(More the coarse fra	(Less than 5% pass		SP	Poorly graded sands, gravelly sands.			
(Less	82	SANDS WITH FINES	Limits plot below "A" line and hatched zone on plasticity chart.		SM	Silty sands, sand—silt mixtures.		
	5	(More than 12% passes No. 200 sieve)	Limits plot above "A" line and hatched zone on plasticity chart.		SC	Clayey sands, sand—clay mixtures.		
SOILS passes	TS t below and cone on chart.	SILTS OF LOW (Liquid Limit le			ML	Inorganic silts, clayey silts with slight plasticity.		
NED SOI ore pas ) sieve)	SILTS Limits plot below "A" line and hotched zone on plasticity chart.	SILTS OF HIGH (Liquid Limit m		МН	Inorganic silts, micaceous or diatomacecous silty soils, elastic silts.			
FINE—GRAINED (50% or more p No. 200 siev	CLAYS Limits plot above I "A" line and hatched zone on plasticity chart.	CLAYS OF LOW (Liquid Limit le		CL	Inorganic clays of low to medium plasticity, gravelly, sandy, silty and/or lean clays.			
FINI (50%)		CLAYS OF HIGH (Liquid Limit m		СН	Inorganic clays of high plasticity, fat clays, sandy clays of high plasticity.			
NOTE	NOTE: Coarse grained soils with between 5% and 12% passing the No. 200 sieve and fine grained soils with limits plotting in the hatched zone on the plasticity chart are to be denoted by double symbols.							

NOTE: Limited data is shown from the Geotechnical Investigation performed for this project. Upon request, a copy of the original Geotechnical Investigation report will be furnished. Use of the data contained here or in the original report is at the discretion of the user who is responsible for all interpretations or assumptions based on this data.

NOTE: HSA = Hollow Stem Auger (CME-75)
SS = Split Spoon Sampler with SPT value
T = Thin Walled Tube Sampler
B = Bulk Sample
N = SPT value
SPT = Standard Penetration Test (616 N hammer free falling 762 mm)

SAMPLE TYPE: G = Grab Sample R = Ring Sample N = Split—barrel Sample

# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

SOIL PROFILE & BORING PLAN
(2 OF 2)

Designed by: MAZ

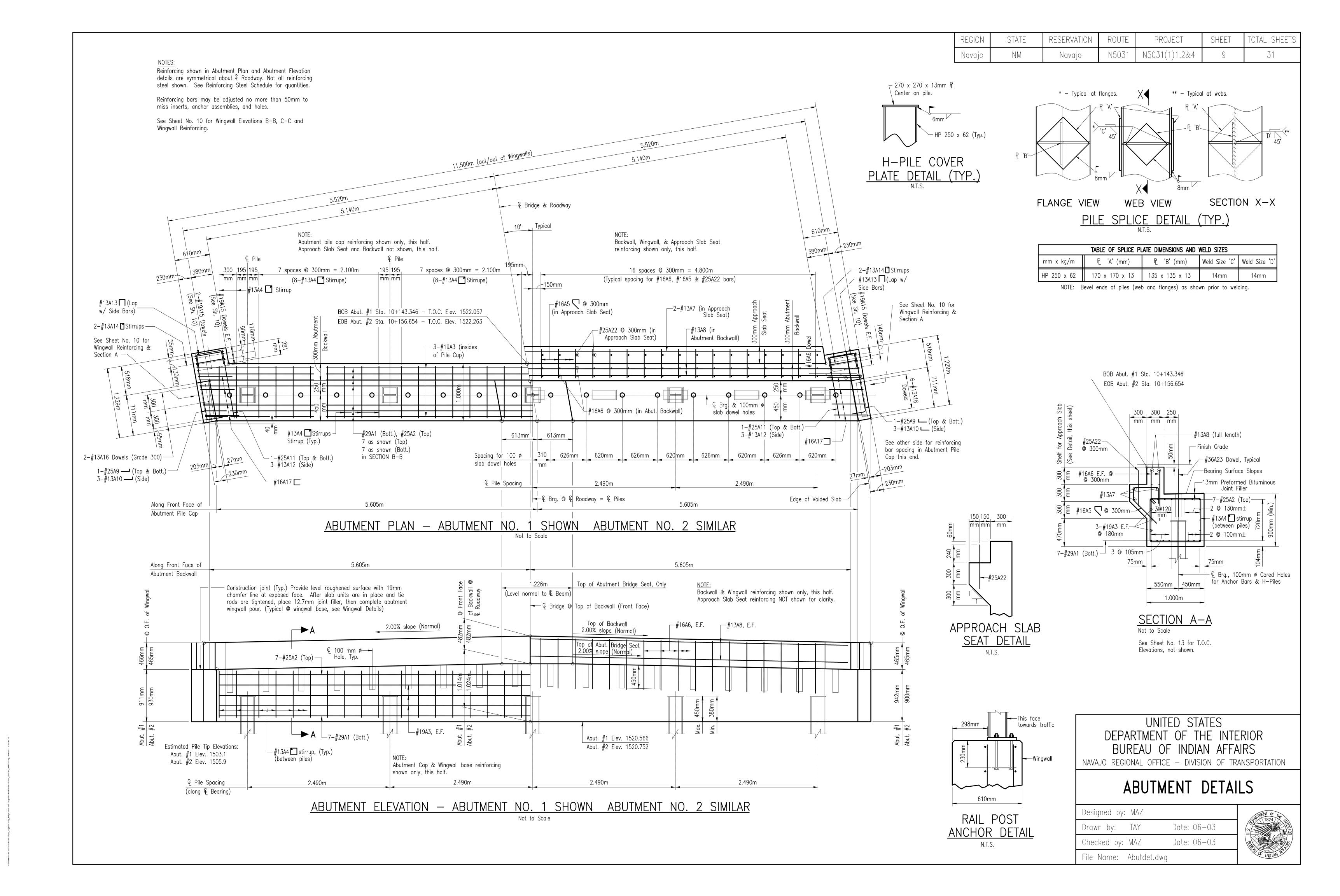
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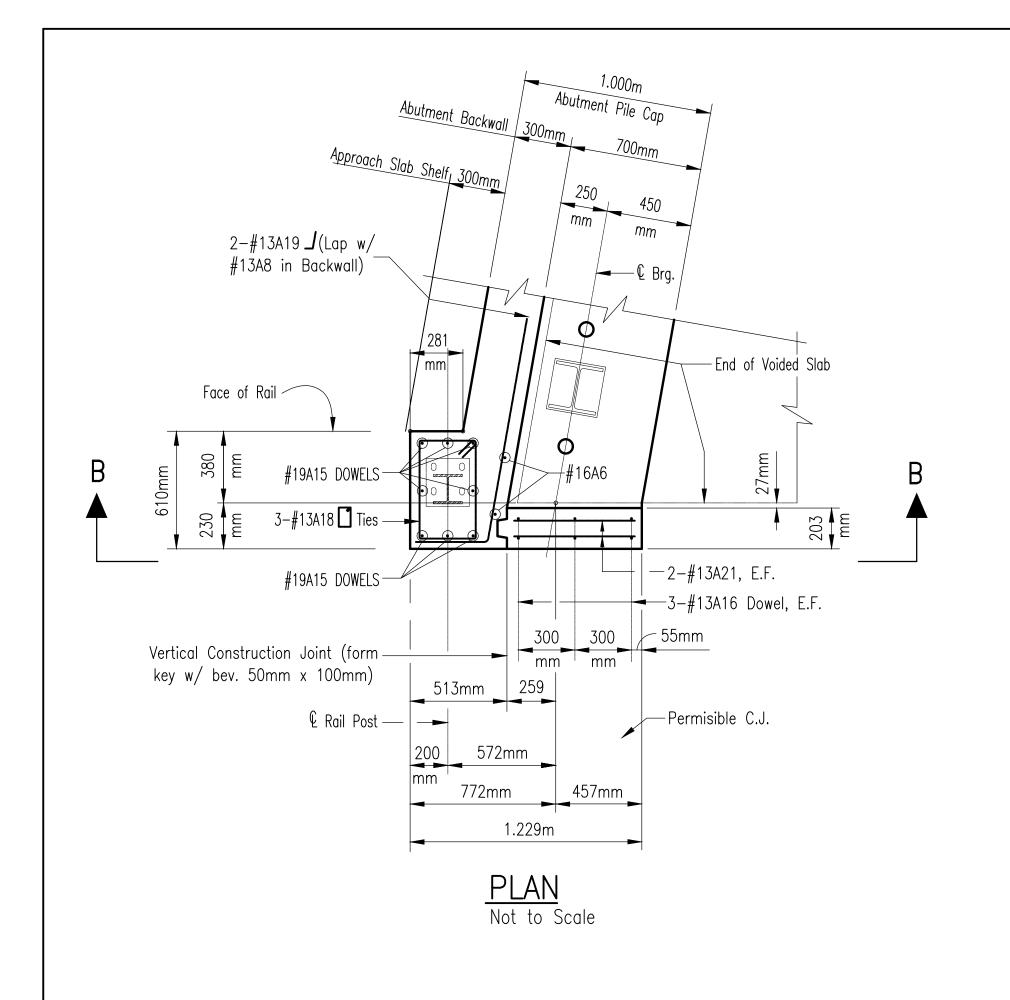
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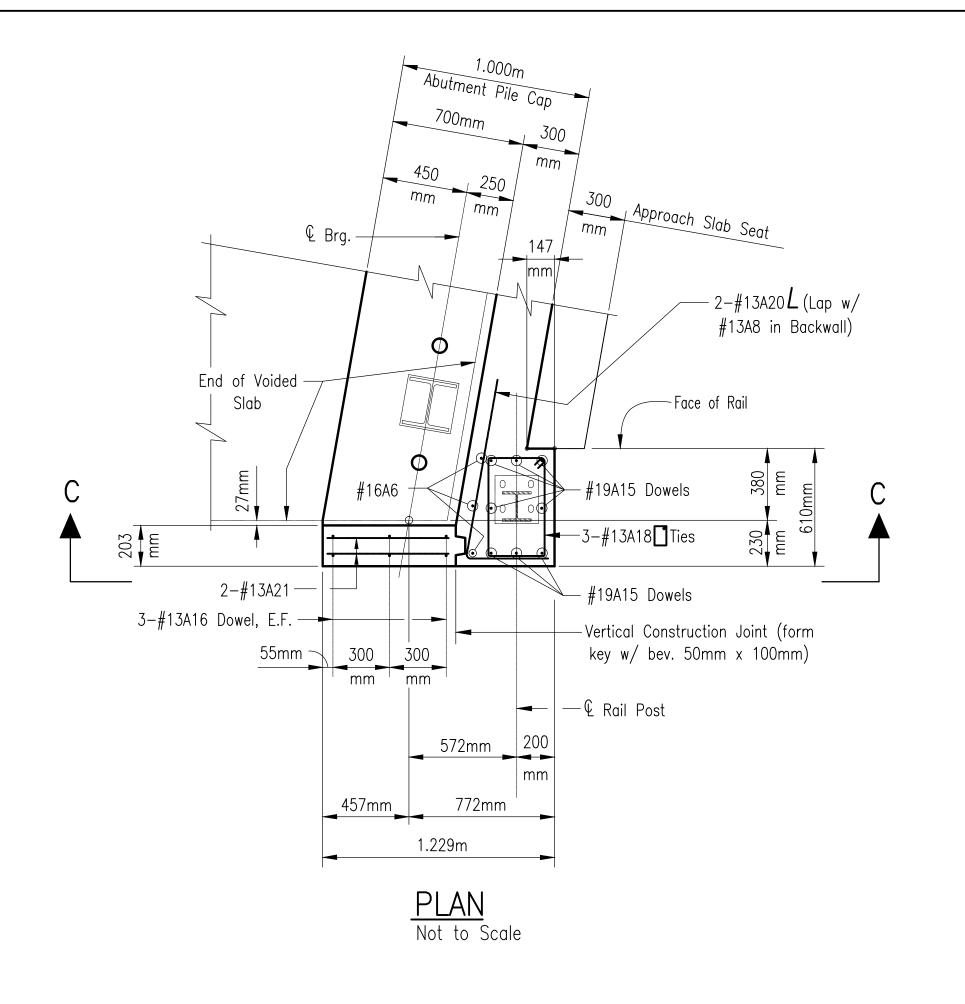
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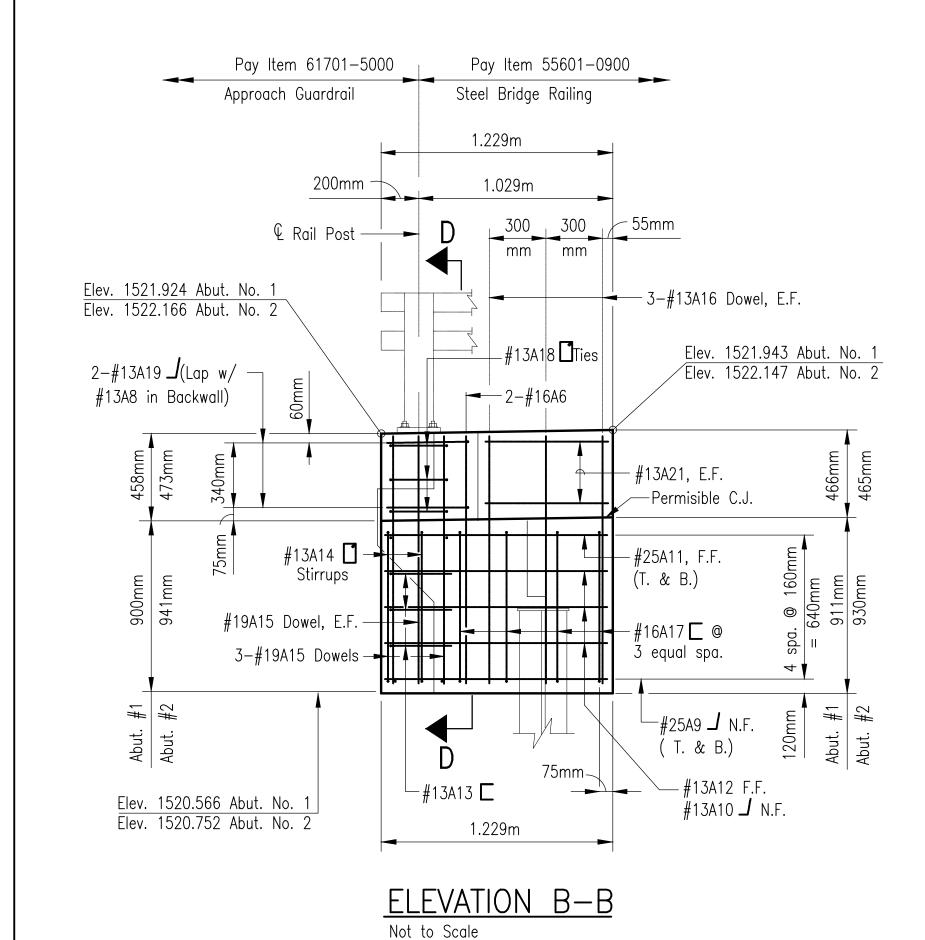
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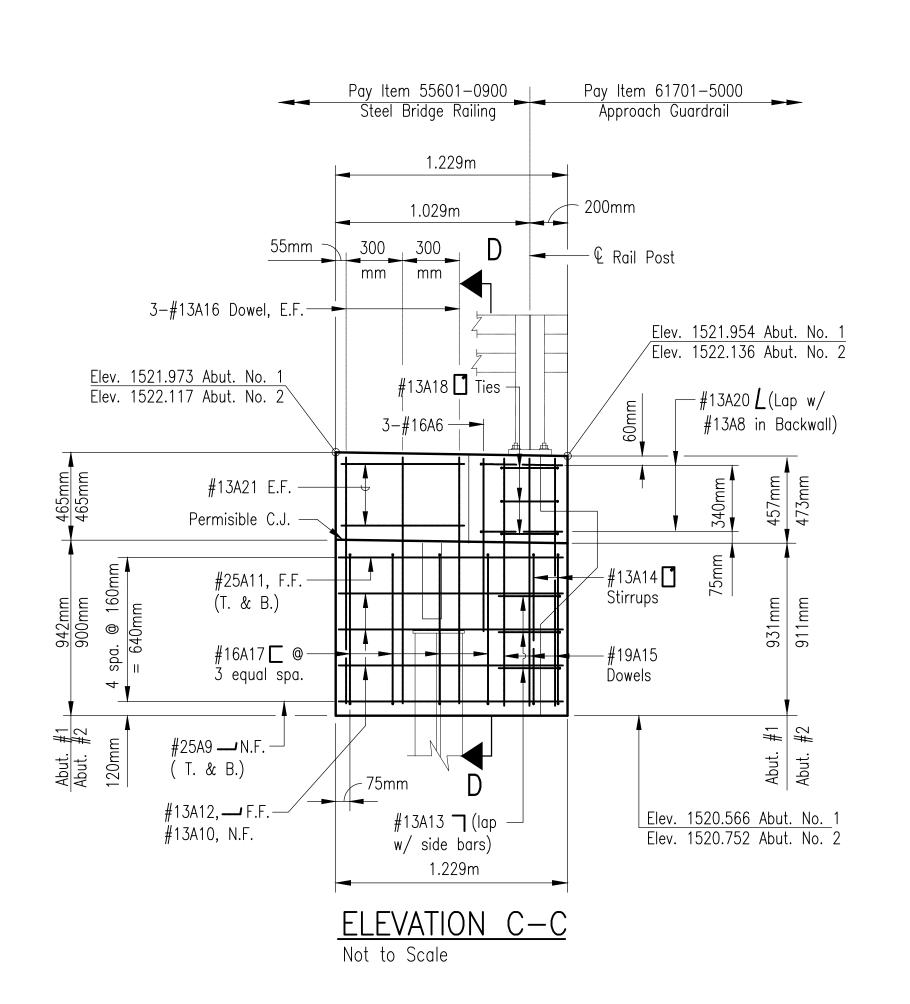


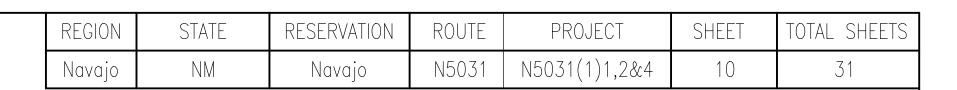


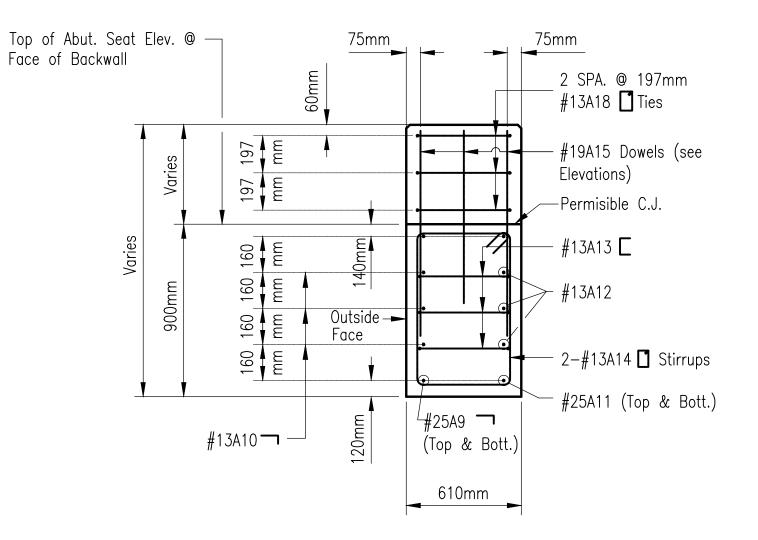






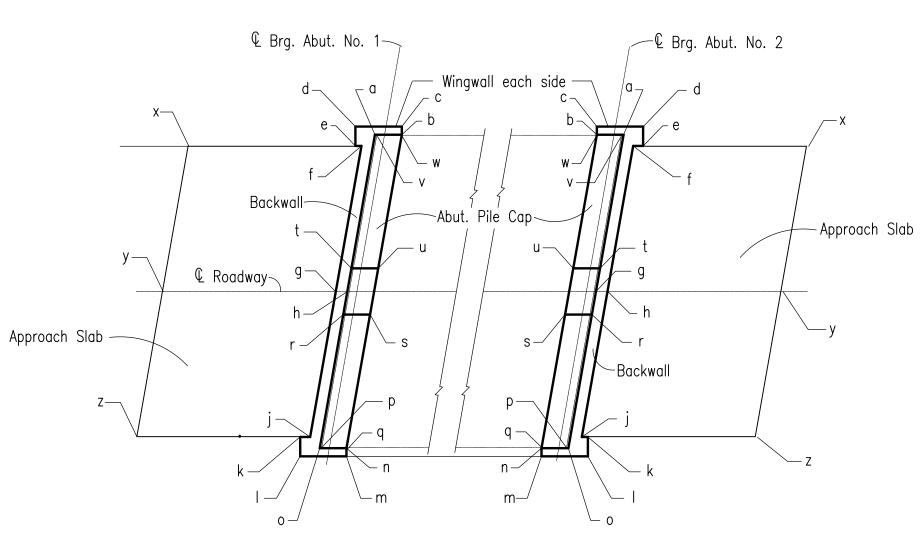






SECTION D-D

Not to Scale



SKETCH SHOWING TOP OF CONCRETE ELEVATION POINTS

Not to Scale

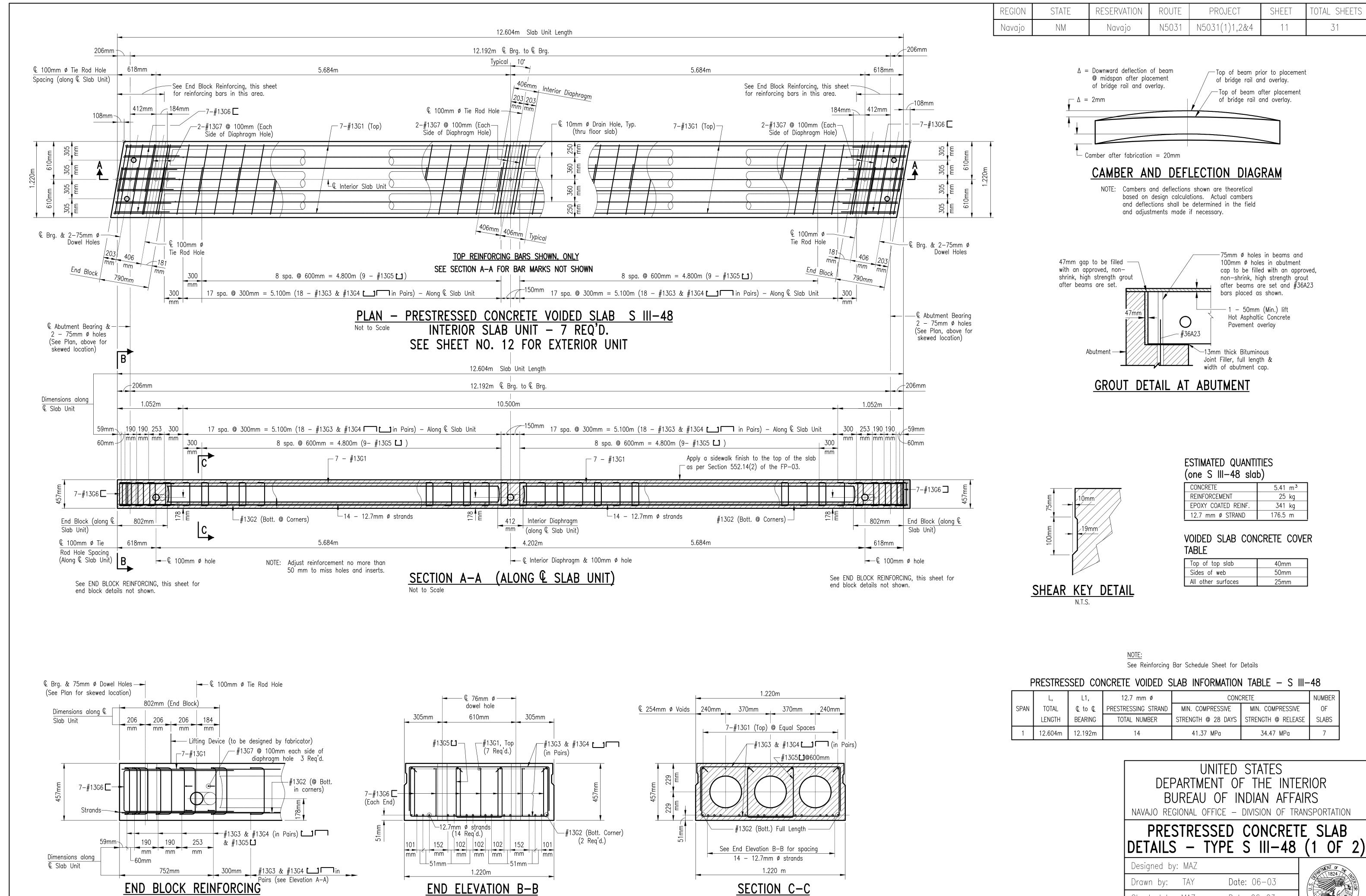
TOP OF CONCRETE ELEVATIONS						
LOCATION	POINT	ABUTMENT NO. 1	ABUTMENT NO. 2			
	a	1521.966	1522.163			
	b	1521.977	1522.151			
	С	1521.973	1522.147			
(WAI	d	1521.954	1522.166			
& BACKWALI	е	1521.966	1522.179			
ස ස	f	1521.968	1522.174			
	g	1522.057	1522.258			
WINGWALL	h	1522.062	1522.263			
NIN	j	1521.940	1522.146			
0F v	k	1521.936	1522.148			
	_	1521.924	1522.136			
T0P	m	1521.943	1522.117			
	n	1521.947	1522.121			
	0	1521.936	1522.132			
	р	1521.466	1521.663			
N.	q	1521.477	1521.652			
OF ABUTMENT PILE CAP	r	1521.578	1521.774			
ABU CA	S	1521.589	1521.763			
)F ,	t	1521.581	1521.778			
TOP (	u	1521.592	1521.767			
2	٧	1521.497	1521.693			
	W	1521.508	1521.682			
TOP OF	X	1521.893	1522.243			
APPROACH	у	1521.979	1522.332			
SLAB	Z	1521.858	1522.217			

UNITED STATES
DEPARTMENT OF THE INTERIOR
BUREAU OF INDIAN AFFAIRS
NAVAJO REGIONAL OFFICE – DIVISION OF TRANSPORTATION

# WINGWALL DETAILS AND TOP OF CONCRETE ELEVATIONS

Designed by:	MAZ	
Drawn by:	TAY	Date: 07-03
Checked by:	MAZ	Date: 07-03
File Name:	Wingdets.dwg	]

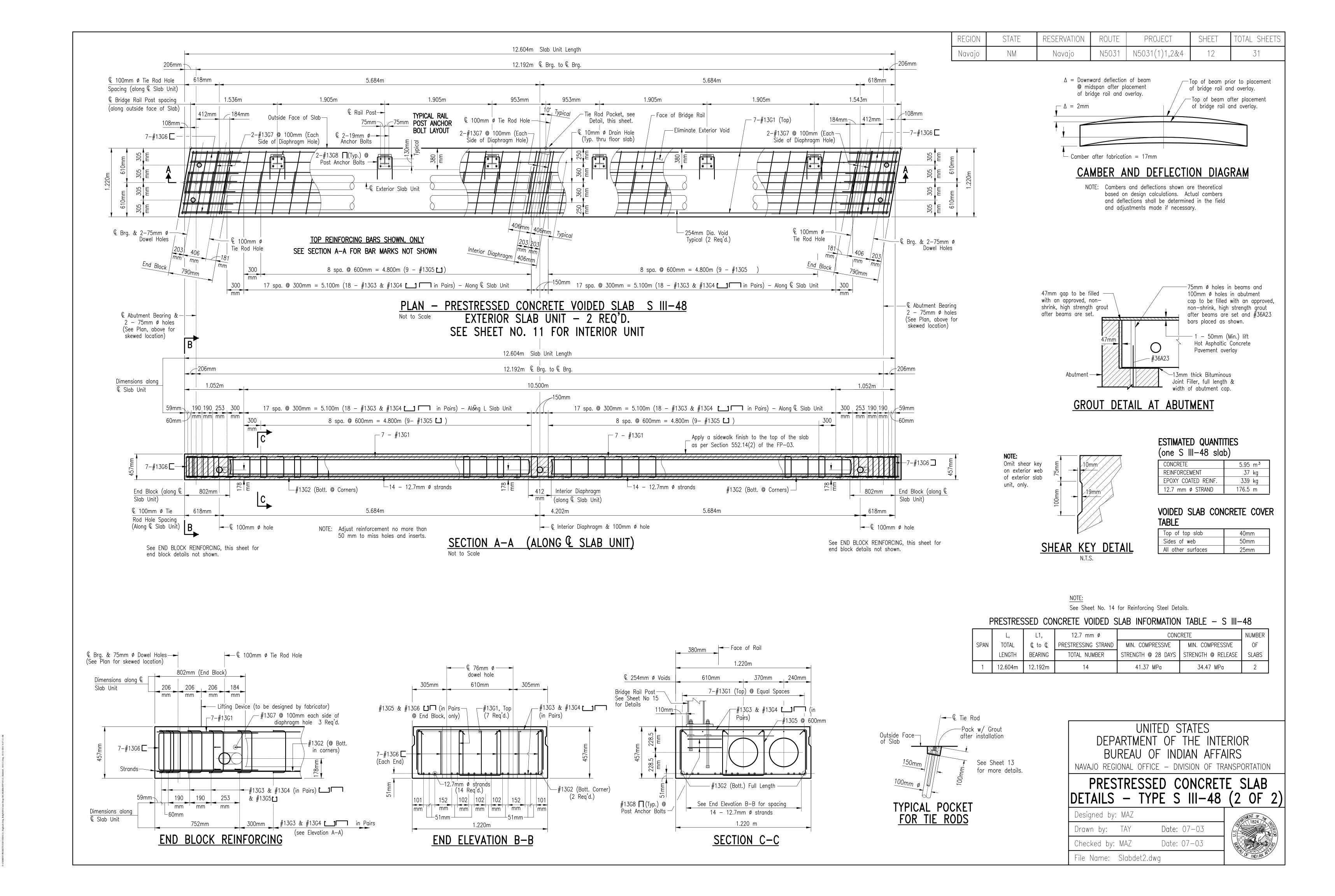




SECTION C-C

END ELEVATION B-B

Drawn by: TAY Date: 06-03 Date: 06-03 Checked by: MAZ File Name: Slabdet1.dwg



# DECK PLAN - PRESTRESSED VOIDED SLABS Not to Scale

Navajo NM Navajo N5031 N5031(1)1,2&4 13 31

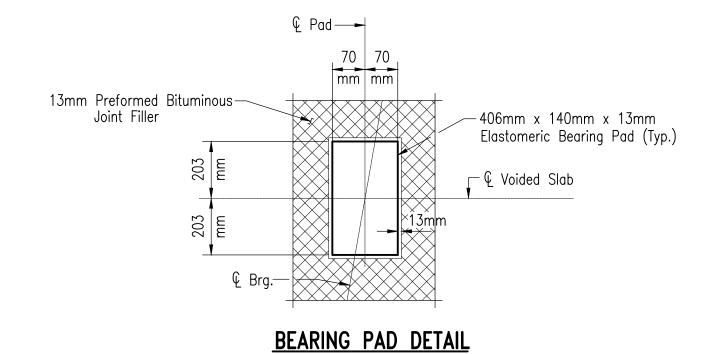
### HACP OVERLAY NOTES

- 1. A Water Proofing Membrane and Hot Asphaltic Concrete Pavement overlay shall be placed over the slabs and approach slabs as follows:
- a. Lightly sand blast top surfaces of slabs and approach slabs to clean any loose or foreign material.
- b. Place primer or adhesive as recommended by the membrane manufacturer over clean surfaces of box beams and approach
- slabs within limits of overlay shown in on this sheet.

  c. Place water proofing membrane per manufacturer's recommendations.
- All joints shall be overlapped a minimum of 150 mm.
- d. Place 50 mm of Hot Asphaltic Concrete Pavement over water proofing membrane within limits shown.
- 2. Do not place Hot Asphaltic Concrete Pavement until the non—shrink grout in shear keys has cured for 21 days.
- 3. See note about HACP in Bridge General Notes for additional details.
- 4. The water proofing membrane, adhesive and sand blasting are incidental Item 40201—0500.

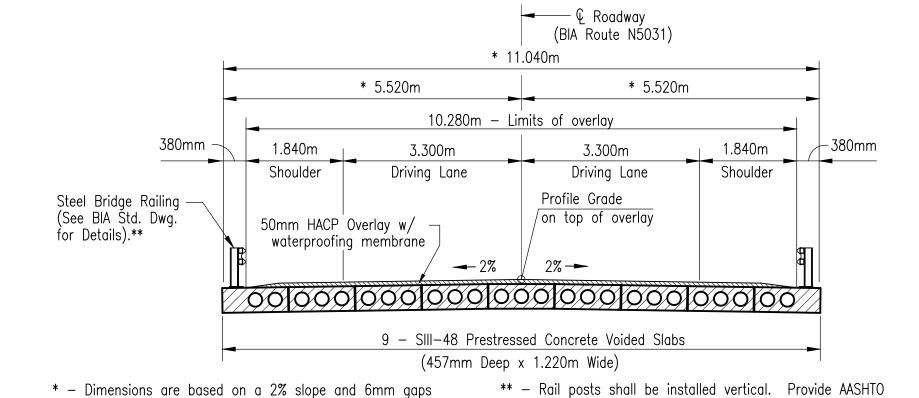
### TIE ROD NOTES

- 1. Tie Rods shall be either 32mm Ø AASHTO M270M, Grade 250, steel rods or AASHTO M 203M, Grade 1860, prestressing strands tensioned to a load of 135 kN. Tie rod threads shall be cut to the coarse thread Series 2A. At the Contractor's option, equivalent rods with rolled threads may be substituted. Tie Rods shall be galvanized after fabrication.
- 2. After tie rods have been placed and tensioned, longitudinal shear keys shall be filled with an approved non—shrink, high strength grout.
- 3. If prestressing strands are used, the Contractor shall submit a detail showing method of anchorage for approval prior to use.
- 4. Tie Rods and related hardware shall be incidental to Item 55301-0500, Precast Prestressed Concrete Structural Members, Voided Slab (S III-48), Class P.



# ITEM 40101-0500 - BRIDGE OVERLAY HACP QUANTITIES

WIDTH	THICKNESS	VOLUME				
10.280m	50mm	11.58m³				
TC	TAL:	11.58m³				
UNIT WEIGHT = $2324 \text{ kg/m}^3 = 2.324 \text{ t/m}^3$						
TC	TAL:	26.9 t				
	10.280m TO WEIGHT = 2	10.280m 50mm  TOTAL:				



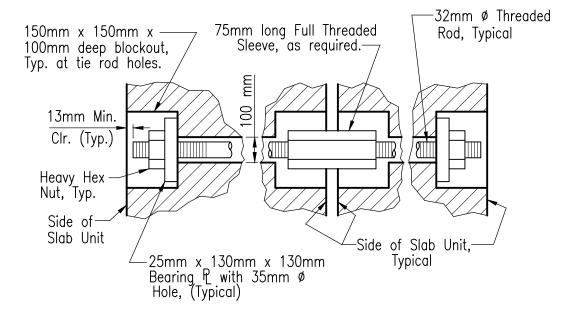
TYPICAL BRIDGE DECK SECTION

N.T.S.

between box beams due to fabrication deviations.

M270M steel shims for proper alignment as necessary.

Shims shall be considered incidental to Item 55601—0900.

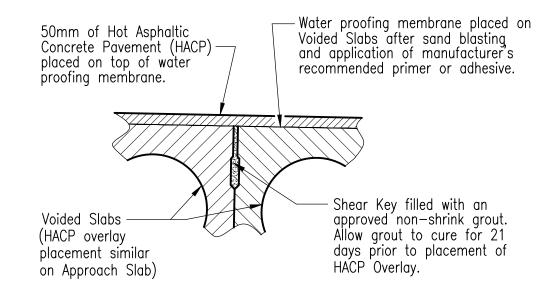


# TIE ROD AND TIE ROD HOLE DETAIL

N.T.S.

NOTES: Fill blockouts at tie rod holes with an approved, non—shrink, high strength grout after tensioning tie rods to the specified load.

See Detail on Sheet No. 12 for Typical Tie Rod Pocket @ skewed end.



HOT ASPHALTIC CONCRETE
PAVEMENT PLACEMENT DETAILS

N.T.S.

DECK PLAN AND SECTION

UNITED STATES

DEPARTMENT OF THE INTERIOR

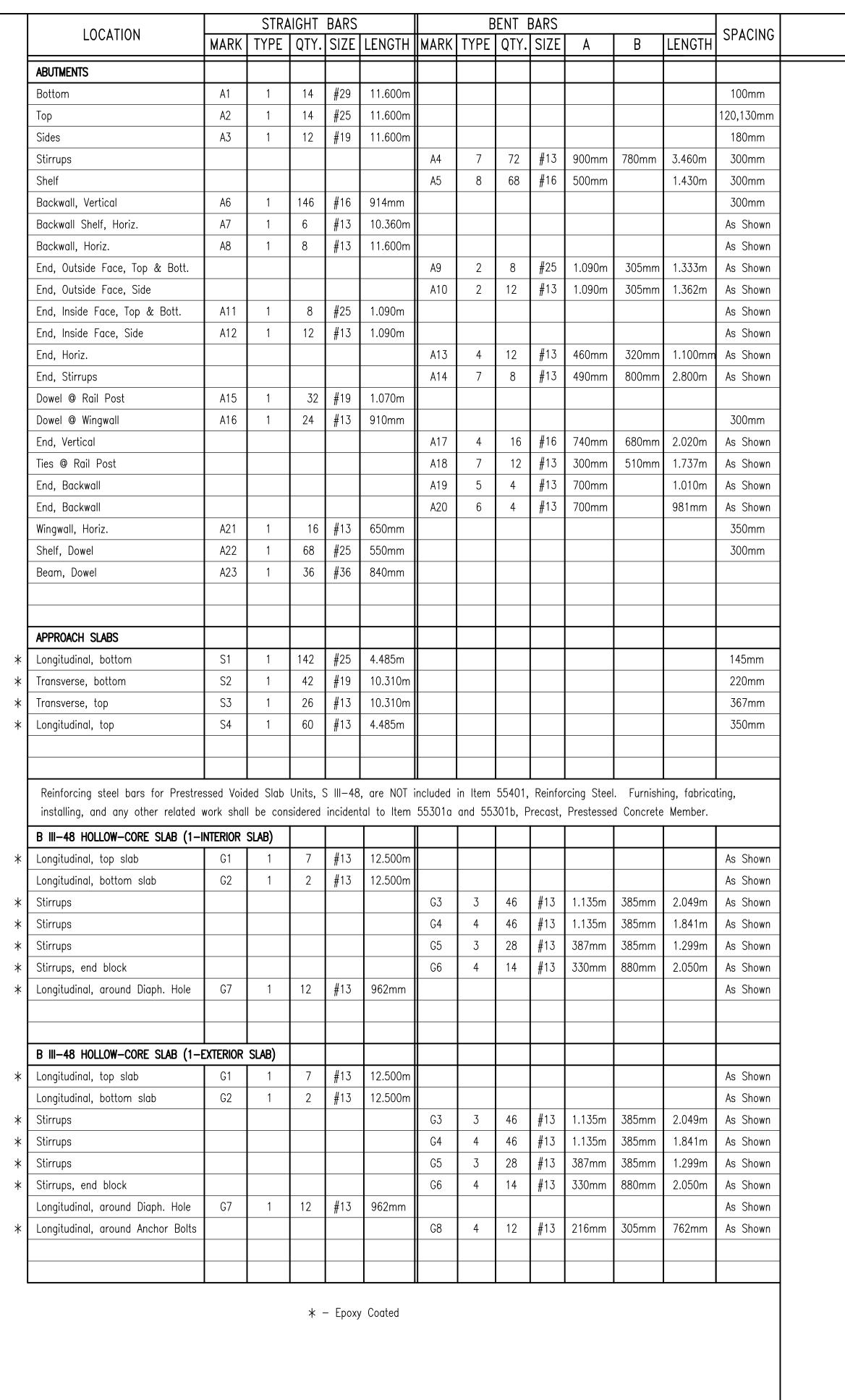
BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

Designed by: MAZ	
Drawn by: TAY	Date: 07-03
Checked by: MAZ	Date: 07-03

File Name: Deckplan.dwg



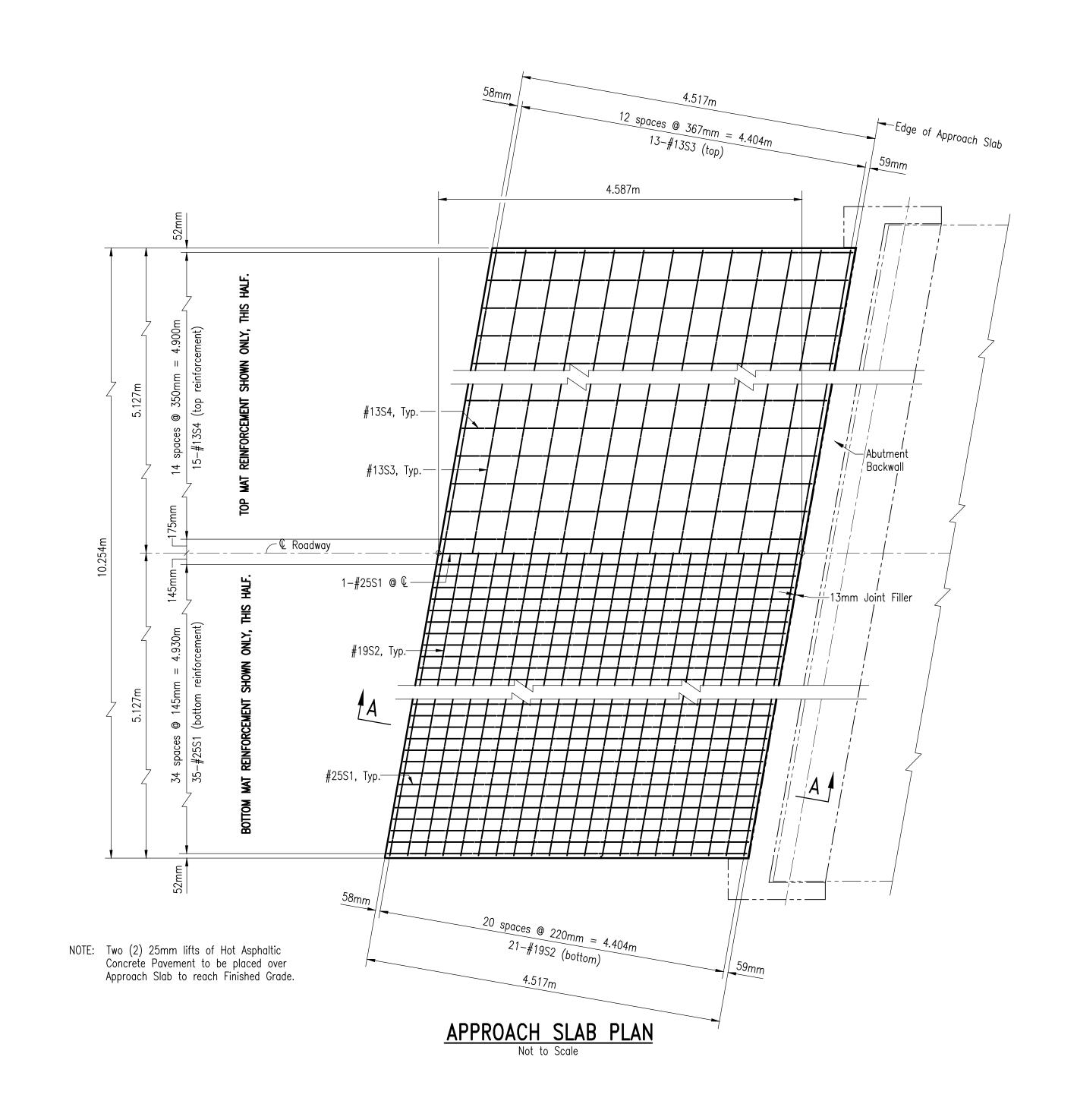


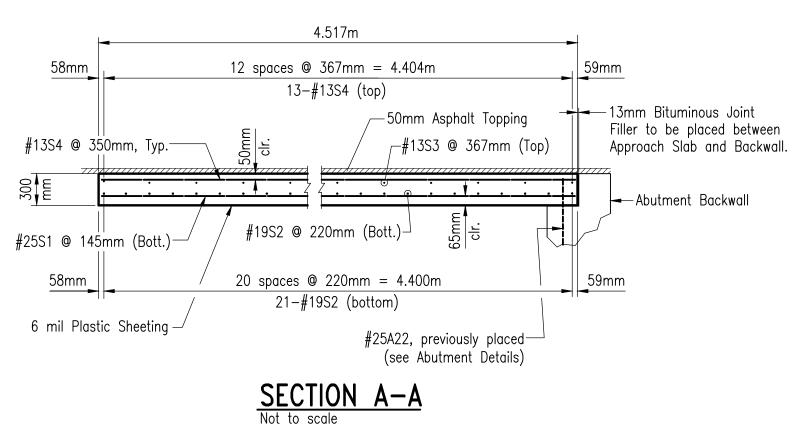
Length TYPE 1 −R=38mm-(Typical) TYPE 3 TYPE 2 TYPE 4 TYPE 5 TYPE 6 TYPE 7 TYPE 8

BENDING DIAGRAMS

ALL DIMENSIONS ARE OUT TO OUT

REGION STATE RESERVATION ROUTE PROJECT SHEET TOTAL SHEETS
Navajo NM Navajo N5031 N5031(1)1,2&4 14 31





UNITED STATES

DEPARTMENT OF THE INTERIOR

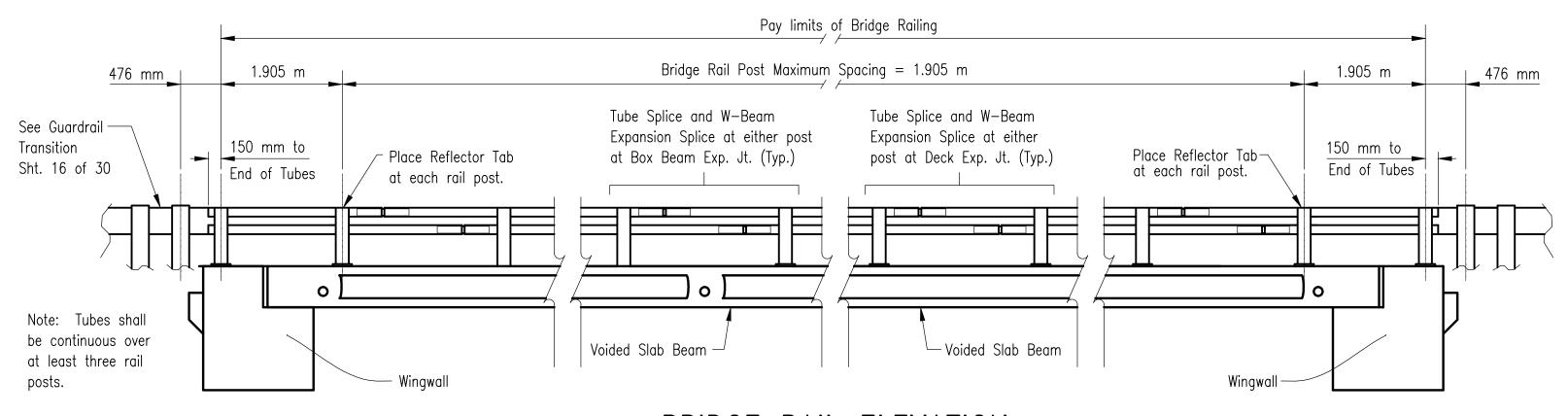
BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE — DIVISION OF TRANSPORTATION

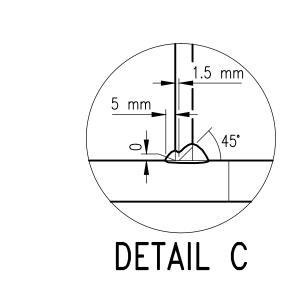
# REINFORCING BAR SCHEDULE & APPROACH SLAB DETAILS

Designed by: MAZ	
Drawn by: TAY	Date: 07-03
Checked by: MAZ	Date: 07-03
File Name: Rnfschd.dwg	





# BRIDGE RAIL ELEVATION



102 mm x 76 mm Tube — © 16 mm Ø Slotted Truss Head or Hex Head Bolts with Hex Nuts and Washers. as shown. ¢ 16 mm ø B. H. Oval Shoulder or Truss Head 32 mm Ø Dbl. Extra-Strong Pipe Sleeve x 54 mm long with Washers. 140 mm 13 mm -W150 x 30 Post 23 mm See Detail C Back Flange Bridge Deck and Web 8 mm + 1.5 mm Front Flange

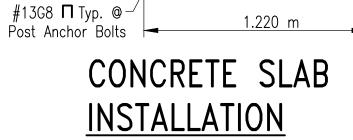
Note: In lieu of Front Flange Weld shown, a 9.5 mm fillet weld all around including edges of flange may be used.

Note: Details shown are similar for Box Beam installation except for items pertaining to 25 mm Base Plate.

# ELEVATION OF RAIL POST

# GENERAL NOTES

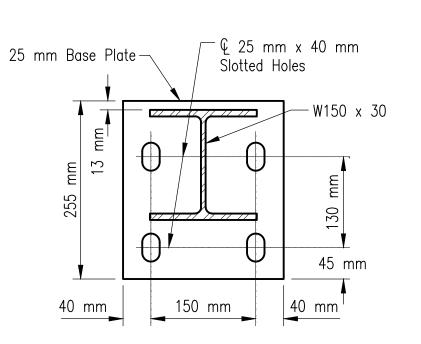
- 1. Panel lengths of tube members shall be attached continuously to a minimum of three posts (except at abutments with expansion joints). Tube splice assemblies shall be provided at a 15 m maximum spacing throughout the bridge railing.
- 2. Dimensions and specified hardware is given in SI (metric) units. For hardware specified in SI (metric) units, English unit hardware may be substituted provided that they are equal or greater in strength.
- 3. Rail posts shall be placed as shown and perpendicular to the adjacent roadway grade. They may be vertical to normal roadway cross slopes, and shall be perpendicular to super elevated roadway cross slopes. The face of railing shall have a smooth transition between normal and super elevated positions. Metal shims shall be used where necessary.
- 4. All nuts, bolts, washers, anchorage plates, bottom plates and necessary hardware are considered as parts of the rail for payment.
- 5. All steel components except reinforcing steel and bolt anchorage plates shall be galvanized unless otherwise shown in plans.
- 6. Anchor bolts shall be M20 x 2.5 AASHTO M164, Type 1, galvanized bolts. Each bolt shall have a hardened steel washer and a 50 mm plain steel washer. Nuts shall conform to AASHTO M291, Class 10S, Class 2A or 2B fit tolerances and be heavy hex, galvanized nuts.
- 7. All tubes and tube sleeve dimensions shall be as shown in the Tube Sleeve Table. All posts, plates and shims shall conform to AASHTO M270M.
- 8. All W—Beam and W—Beam connection hardware shall conform to the requirements of AASHTO M 180.
- 9. Shop drawings shall be submitted and approved at least 14 days prior to the fabrication of the bridge rail components.
- 10. Dimensions noted by asterisk (\*) shall be increased by 50 mm when a 50 mm HACP overlay is specified for the bridge deck.



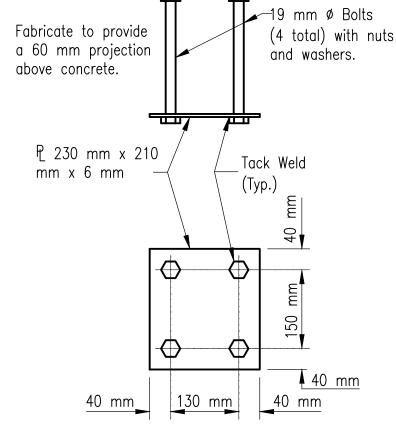
Bridge Rail Post See Sheet No 15—

110 mm-

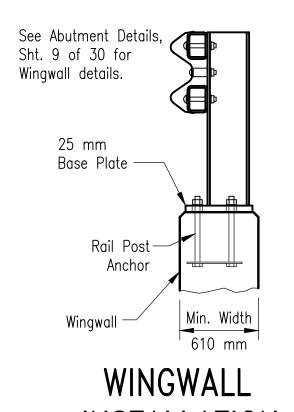
for Details



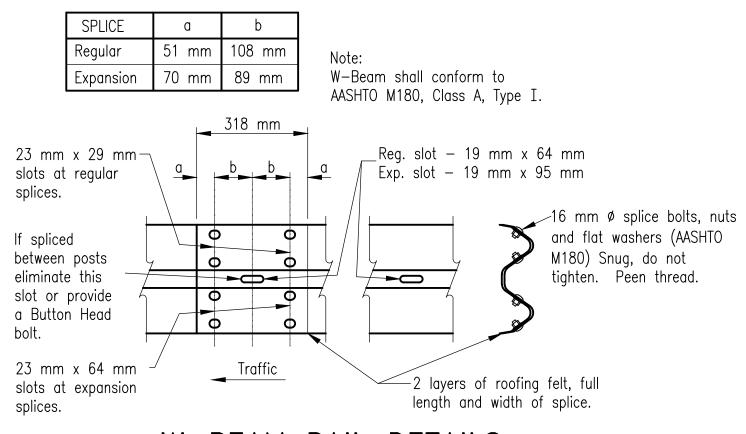
BASEPLATE DETAIL



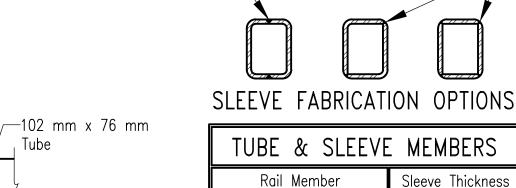
RAIL POST ANCHOR



INSTALLATION

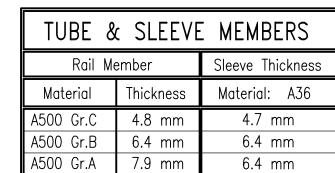


W-BEAM RAIL DETAILS





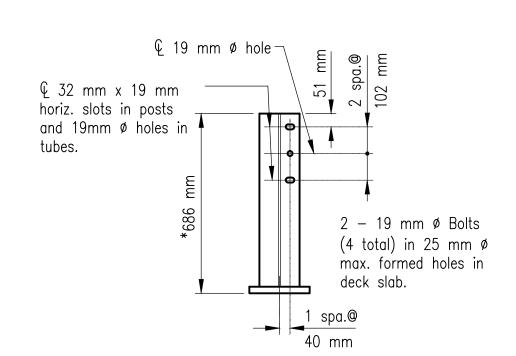
5 mm



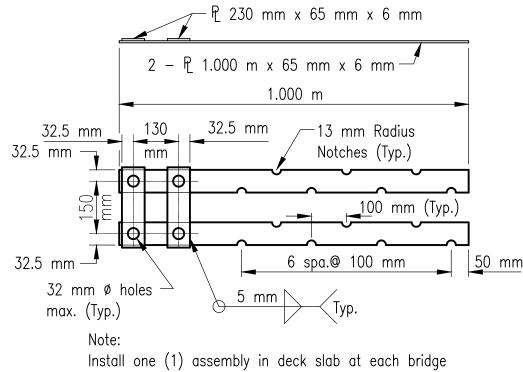
Weld and grind

-smooth as required—

Note: Other sections of equal or greater strength are acceptable for sleeves.

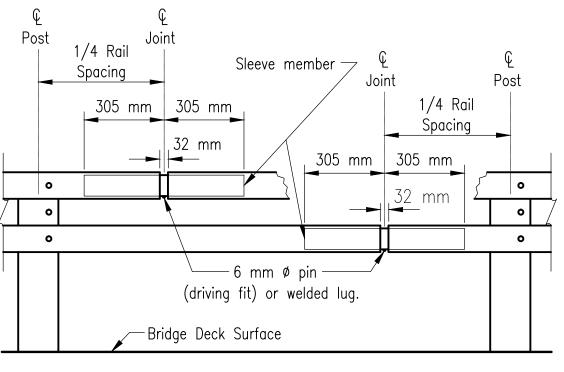


BRIDGE RAIL POST FOR CONCRETE DECK AND WINGWALL



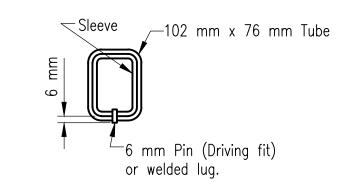
Note:
Install one (1) assembly in deck slab at each bridge rail post. Field cut or bend as required to fit special conditions. Do <u>not</u> galvanize or oil.

BOLT ANCHORAGE PLATES



Note: W—Beam Rail not shown.

TUBE SPLICE DETAILS



The difference between the outside dimensions of the sleeve and the inside dimensions of the rail shall not exceed 3 mm along either axis.

TUBE SPLICE SECTION

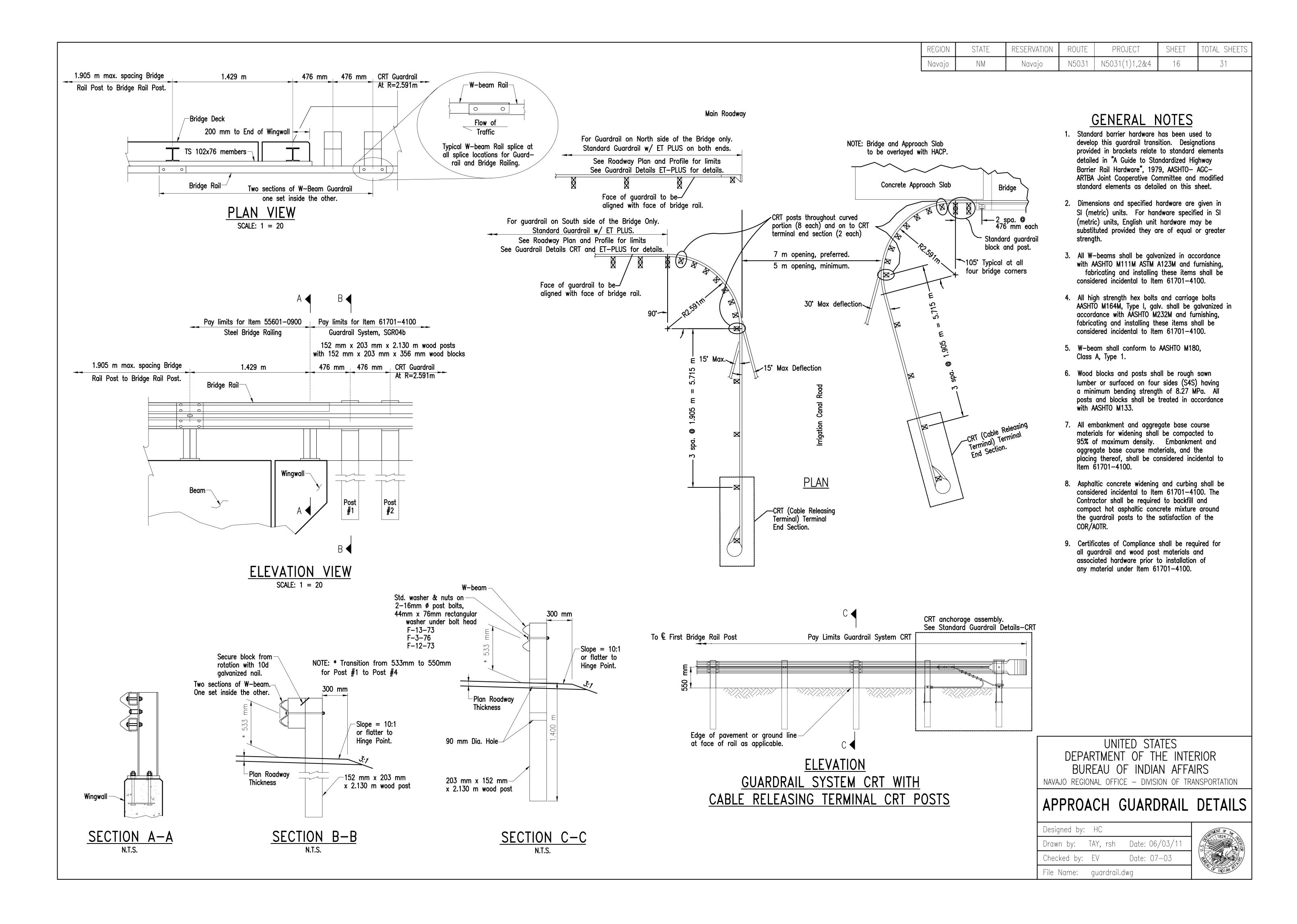
# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

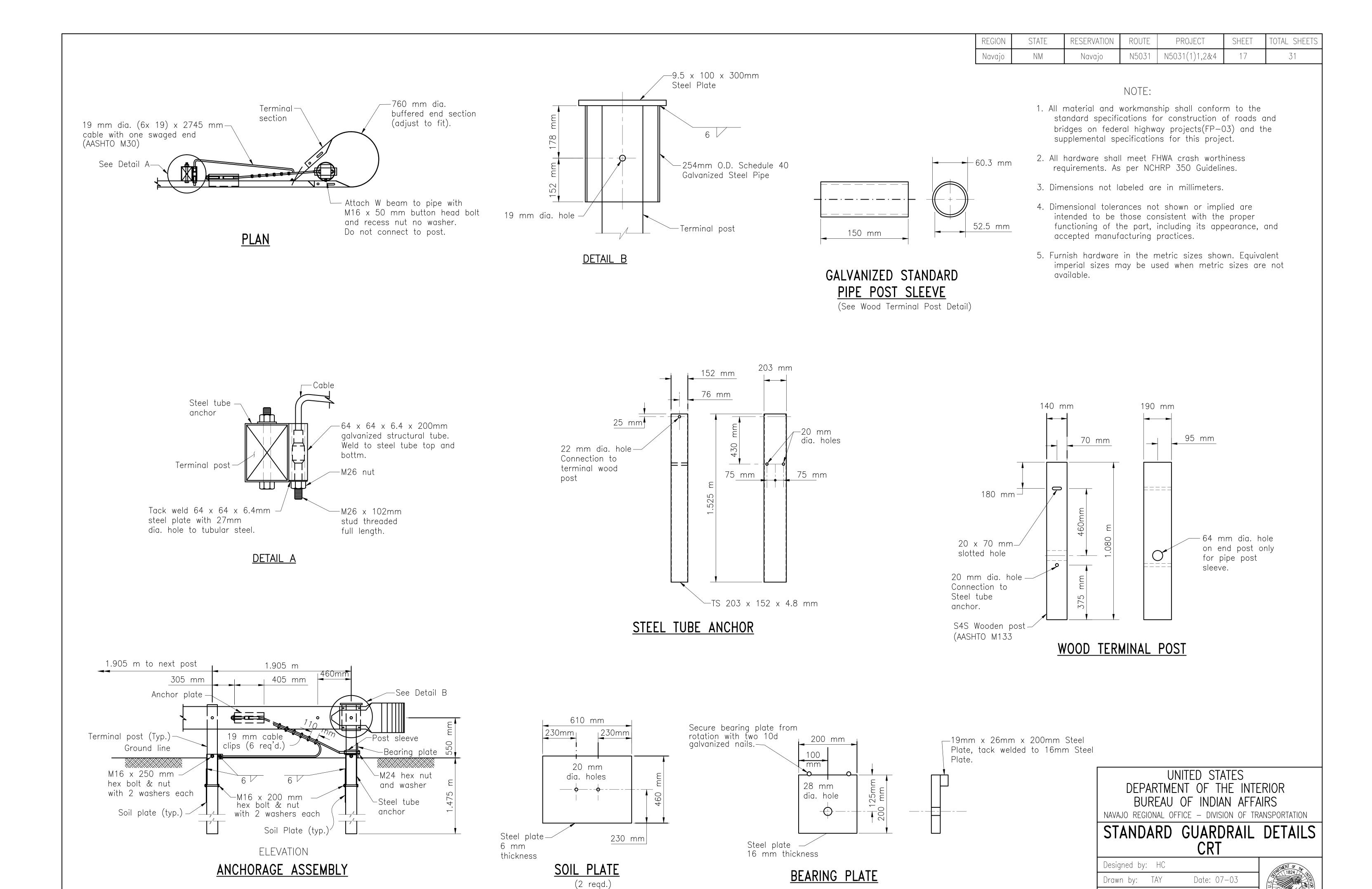
NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

# BRIDGE RAIL DETAILS

Designed by:	BOR –	Structural Unit	
Drawn by:	TAY	Date: 07-03	
Checked by:	HRC	Date: 07-03	
File Name:	Brdgrail.	dwg	





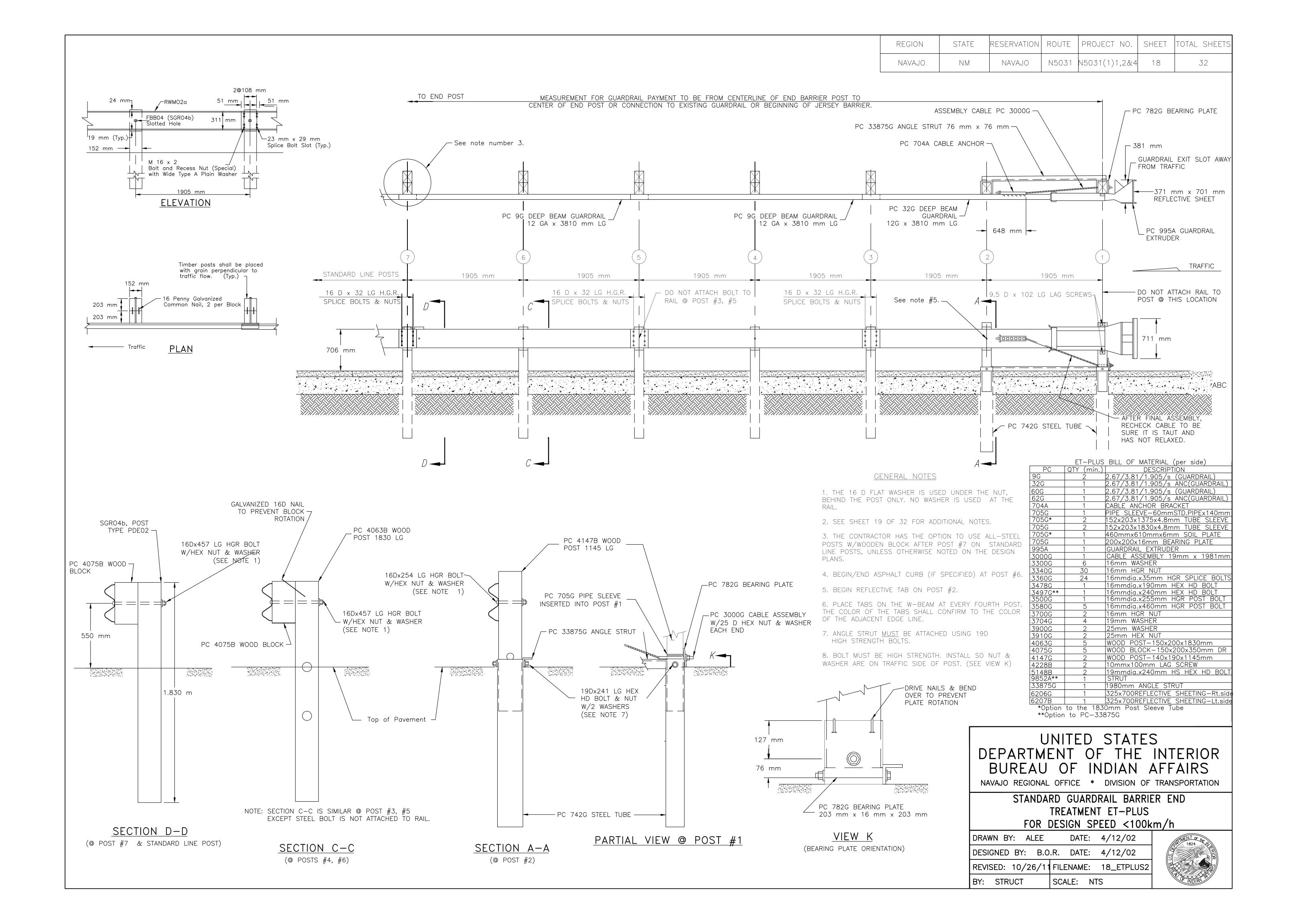


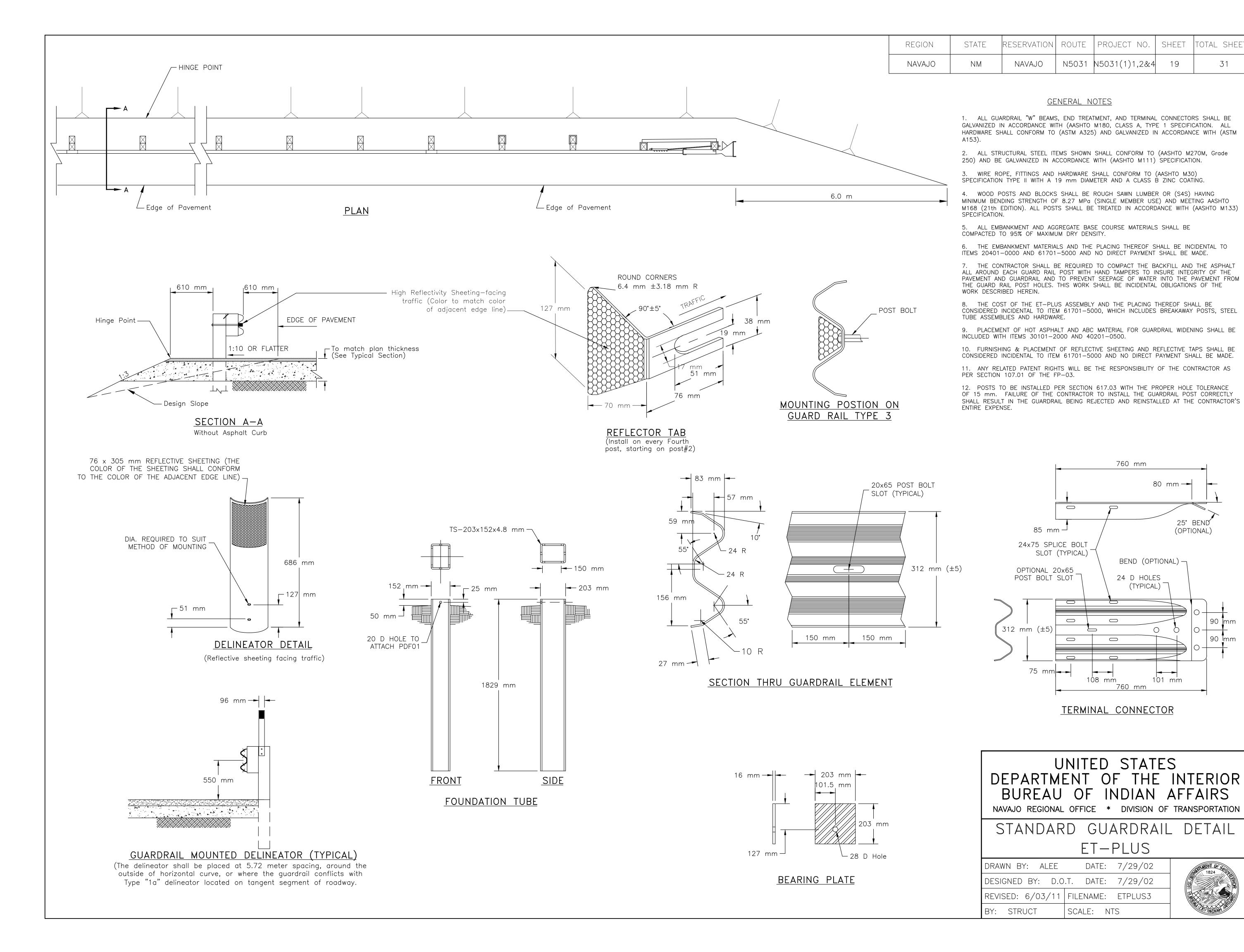
Checked by: EV

File Name: crt.dwg

Date: 07-03

F.C.(RRENT PROJECTS/M22/M50317). Hawback Irrie Brien/2266 Caris Dave BIA Modifie





SHEET TOTAL SHEETS

80 mm -

25° BEND

(OPTIONAL)

31

REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	NEW MEXICO	NAVAJO	N5031	N5031(1)2&4	20	31

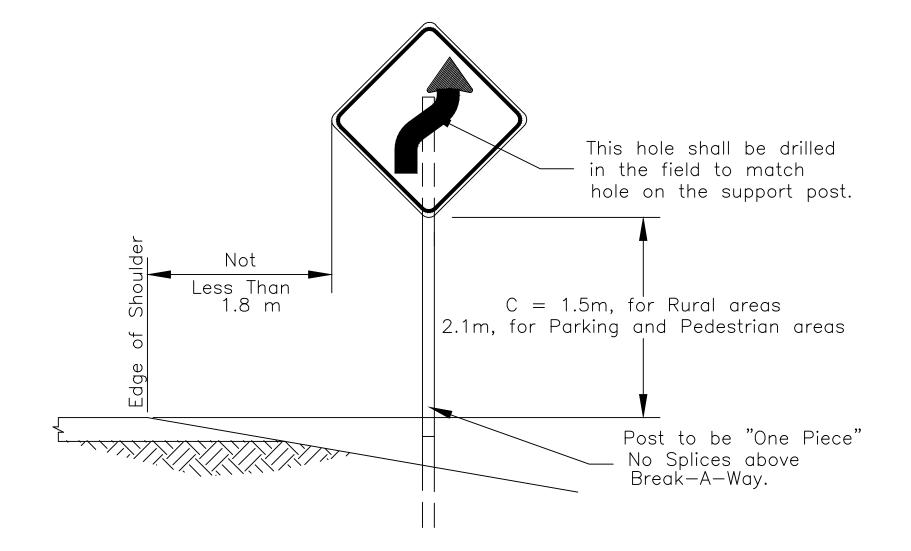
			С	HART	TO D	DETER	MINE	SING	LE PO	ST S	SIZE			_
POST	K FACTOR				B D	IMENSION	N (Meter	)						
WEIGHT	(B x A)	1.52	1.83	2.13	2.44	2.74	3.05	3.35	3.66	3.96	4.27	4.57	4.87	
2.976 kg/m	<b>⊢</b>	0.36	0.36	0.36	0.36	0.36	0.36	0.36	0.34	0.32	0.30	0.27	0.26	. 2.
3.348 kg/m	O Z >-	0.47	0.47	0.47	0.47	0.47	0.47	0.43	0.39	0.36	0.33	0.32	0.30	A(m <sup>2</sup> )
4.092 kg/m	S. J.	0.62	0.62	0.62	0.62	0.62	0.62	0.56	0.51	0.47	0.44	0.41	0.38	SIGN AREA
4.464 kg/m	OES APPI	0.68	0.68	0.68	0.68	0.68	0.68	0.68	0.62	0.58	0.52	0.47	0.46	
5.952 kg/m		0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.80	0.74	0.70	0.65	

CHART	TO	DETERMINE	DOUBLE	POST	SIZE
-------	----	-----------	--------	------	------

POST	K FACTOR				В	DIMENS	ION (Me	ter)						
WEIGHT	(B x A)	1.52	1.83	2.13	2.44	2.74	3.05	3.35	3.66	3.96	4.27	4.57	4.87	
2.976 kg/m	2.74	1.80	1.50	1.28	1.12	1.00	0.90	0.82	0.75	0.70	0.64	0.60	0.57	
3.348 kg/m	3.08	2.03	1.69	1.45	1.27	1.13	1.01	0.92	0.85	0.78	0.72	0.68	0.63	$A(m^2)$
4.092 kg/m	4.03	2.64	2.20	1.89	1.65	1.47	1.32	1.20	1.10	1.01	0.94	0.88	0.83	SIGN AREA
4.464 kg/m	4.91	3.23	2.69	2.31	2.03	1.79	1.62	1.47	1.35	1.24	1.15	1.08	1.01	]
5.952 kg/m	6.83	4.48	3.73	3.20	2.80	2.49	2.24	2.03	1.87	1.72	1.60	1.50	1.40	

# CHART TO DETERMINE THREE POST SIZE

POST	K FACTOR		B DIMENSION (Meter)											
WEIGHT	(B x A)	1.52	1.83	2.13	2.44	2.74	3.05	3.35	3.66	3.96	4.27	4.57	4.87	
2.976 kg/m	4.12	2.69	2.25	1.92	1.68	1.50	1.35	1.23	1.12	1.04	0.97	0.90	0.85	0
3.348 kg/m	4.65	3.05	2.54	2.17	1.90	1.69	1.52	1.38	1.27	1.17	1.09	1.01	0.96	$A(m^2)$
4.092 kg/m	6.02	3.96	3.30	2.82	2.47	2.19	1.98	1.79	1.64	1.52	1.41	1.32	1.24	SIGN AREA
4.464 kg/m	7.40	4.85	4.04	3.47	3.03	2.69	2.42	2.20	2.02	1.86	1.73	1.62	1.51	
5.952 kg/m	10.20	6.71	5.58	4.78	4.19	3.73	3.35	3.05	2.79	2.57	2.40	2.23	2.09	



TYPICAL ROADSIDE SIGN LOCATION

### GENERAL NOTES

- 1. The contractor shall be required to adjust the length of sign support posts. This work shall be included in the unit price for the appropriate bid items shown in the bid schedule.
- 2. Sign dimension equal to or in excess of 762mm x 762mm size shall be install with a minimum of two (2) steel posts.

# illustration of posts/weight determination:

REQUIRED: Determine Post Requirement
For 1.52 m wide x 1.22 m high Sign.
Located On A Rural Highway.

W = 1.52 m

 $D=1.22 \, \text{m}$ C = 1.5 m for Rural areas

SOLUTION: 1. D/2 = 0.61m; K factor = 3.90 2. B=C+D/2=1.5 + 0.61; B=2.11 m

3. A=WxD=1.52 x 0.61 = 1.85 sq. m.
4. Begin with single post chart for column of B=2.13 m, and continue down until area of sign equal or exceed 1.85 sq. m. The area exceeds the single post chart, so go to the double post table.

Select two (2) Posts of 4.09 kg/m Yields a factor of 4.03 Which Is Optimum.

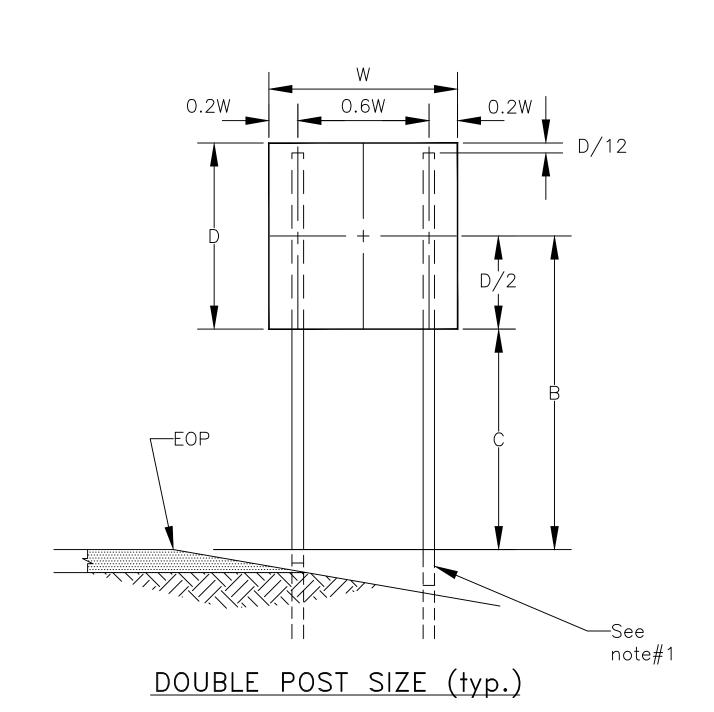
EXAMPLE:	POST	B=2.13
	4.09 kg/m	1.89
	4.46 kg/m	2.31
	5.95 kg/m	3.20

# PERMANENT ROADSIDE SIGNS

			ILIMANLINI	ROADSIDE SIGI				
STATION	LOC.	SIZE DETAIL NO.	DESCRIPTION	SIGN PANEL SIZE (mm)	AREA OF SIGN (Sq. m.)	NO. Of POSTS	kg/m	TOTAL SIGN PANELS
10+250.000	Rt.	W5-1	ROAD NARROWS	762 x 762	0.58	2	2.98	1
10+329.969	Lt.	R1-1	STOP	762 x 762	0.58	2	2.98	1
10+030.000 10+300.000	Rt. Lt.	R2-1 R2-1	SPEED LIMIT 25	610 x 762 610 x 762	0.46 0.46	1 1	4.09 4.09	1 1
10+020.000	Rt.	M-1	<b>503</b> 1	460 x 610	0.28	1	4.09	1

# 

SINGLE POST SIZE (typ.)



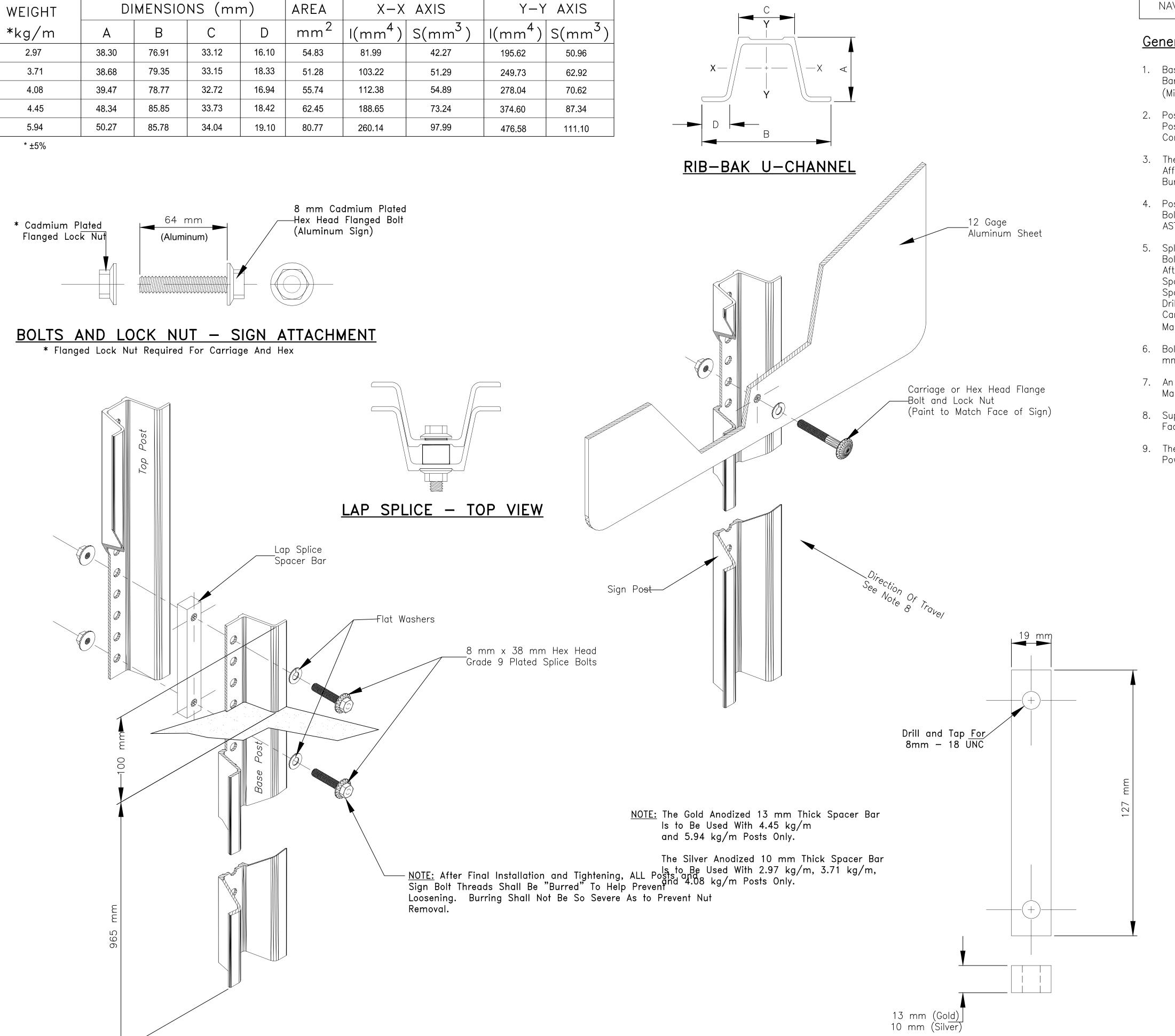
0.15W -See Note#1 THREE POST SIZE (typ.)

UNITED STATES DEPARTMENT OF THE INTERIOR NAVAJO REGIONAL OFFICE \* DIVISION OF TRANSPORTATION

# PERMANENT SIGNING DETAIL

DRAWN BY: B.O.R.	DATE: 11/97
DESIGNED BY: B.O	.R. DATE: 11/97
REVISED: 6/11	FILENAME: Permsign
BY: STRUCT	SCALE: NTS





RIB-BAK U-CHANNEL SIGN SUPPORTS

LAP SPLICE CONNECTION DETAIL

REGION	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	NEW MEXICO	NAVAJO	N5031	N5031(1)1,2&4	21	31

### <u>General Notes</u>

LAP SPLICE SPACER BAR

- 1. Base Post and Sign Post Shall Be Rib—Bak U—Channel Fabricated From Hot Rolled Carbon Steel Bars Conforming to the Requirements of ASTM A499. Yield Point of the Steel Shall Be 550 MPa (Minimum). Tensile Shall Be 689.47 MPa (Minimumum).
- 2. Posts Shall Be a Uniform, Modified, Flanged Channel Section of the Rib-Bak Design. Weight of the Posts Shall Be as Specified By the User,  $\pm 5\%$  Before Punching. The Posts Shall Be Punched With Continuous 9 mm Holes on 25 mm Centers For the Entire Length of the Post.
- 3. The Posts Shall Be Machine Straightened to Have a Smooth, Uniform Finish Free From Defects Affecting Their Strength, Durability, or Appearance. All Holes and Rough Edges Shall Be Free From Burrs. The Permissible Tolerance for Straightness Shall Be Within 6.35 mm in 1.52 Meter.
- 4. Posts Shall Be Galvanized After Fabrication in Accordance With the Requirements of ASTM A123. Bolts, Nuts, Washers and Spacer Shall Be Cadmium Plated in Accordance With the Requirements of ASTM A165 or Zinc Plated in Accordance With the Requirements of ASTM B633.
- 5. Splice Hardware Shall Consist of Two Fully Threaded, 8 mm X 38 mm Grade 9 Plated, Hex Head Bolts, With Flat Washers, and Self Locking Hex Nuts Per Post. In Addition, One Shall Be Galvanized After Fabrication in Accordance With the Requirements of ASTM A123. Bolts, Nuts, Washers and Spacers Shall Be Cadmium Plated in Accordance With ASTM B766. One 19 mm X 127 mm Plated Spacer Bar Shall Be Used, Per Post, To Stiffen the Splice Connection. Each Spacer Bar Shall Be Drilled and Tapped With 8 mm—18 UNC Threads. The Spacer Shall Be Fabricated From Hot Rolled Carbon Steel Bars Conforming to ASTM A36 or M1020. Bolts Shall Be Red in Color With the Head Marking "M180".
- 6. Bolts and Lock Nut Hardware For Sign Attachment Shall Be Hex Head Flange Type. Size Shall Be 8 mm—18 UNC. Bolts and Nuts Shall Be Cadmium Plated to ASTM B766 Specification.
- 7. An Alternate Breakaway and Support Post Assembly May Be Submitted to the NRDOT Division Manager for Review and Approval.
- 8. Supplemental Signs on the Opposite Side of Road Shall Have the Post Reversed So That Rib—Bak Is Facing Away From the Opposing Traffic.
- 9. The Post Shall Be Coated With a Baked on Green Alkyd Resin, Paint, Painted With a Polyester Powder or Galvanized Per Note 4 Above.

## INSTALLATION PROCEDURE

TEP 1:
Drive Base Post to Within Approximately 300 mm Above
Ground Level. Place One Bolt and Cut Washer in Fifth Hole
From the Top and Securely Tighten Threaded Spacer Onto
Bolt.

STEP 2:
Drive Base Post to 100 mm Above Ground Level. Place
Remaining Bolt and Cut Washer in First Hole From the End
and Securely Tighten Threaded Spacer Onto Bolt.

STEP 3:
Dig Out Approximately 50 mm From Around Back of Ground
Post to Allow Room For Top Post to Be Attached.

STEP 4:

Nest Top Post Onto Protruding Base Post Bolts, Through the First and Fifth Holes of the Top Post.

STEP 5:
Place a Self—Locking Flange Nut on Each Bolt. Tighten
Nuts and Tamp Earth Around Post Firmly.

# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGION OFFICE \* DIVISION OF TRANSPORTATION

# SIGN POST AND HARDWARE DETAILS

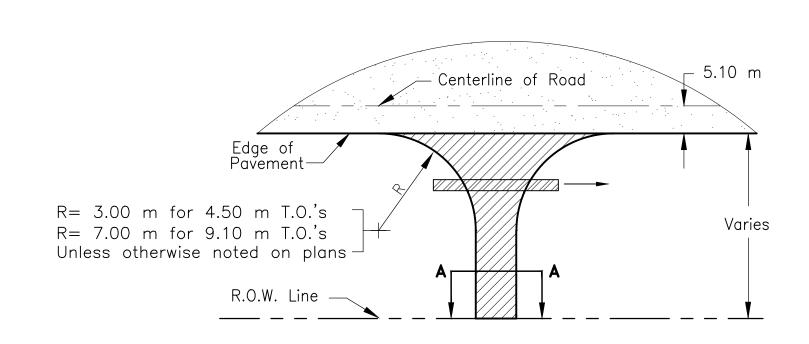
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Drawn by: NRO-DOT Date: 06/03/11

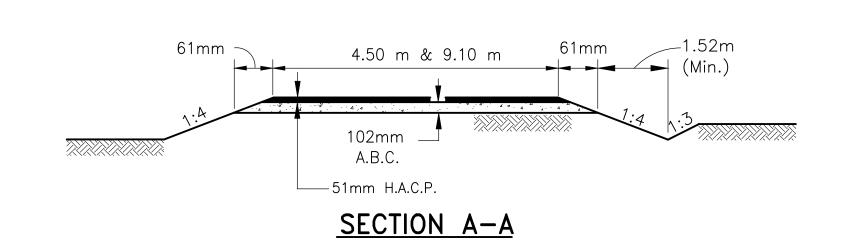
Revised by: Date:

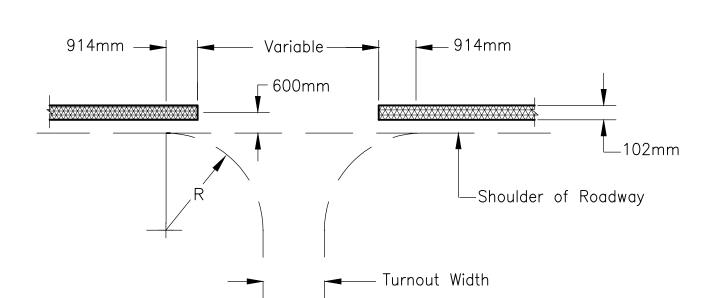
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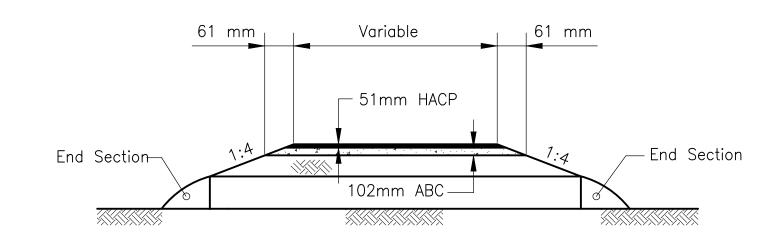


TYPICAL TURNOUT - TYPE "A"

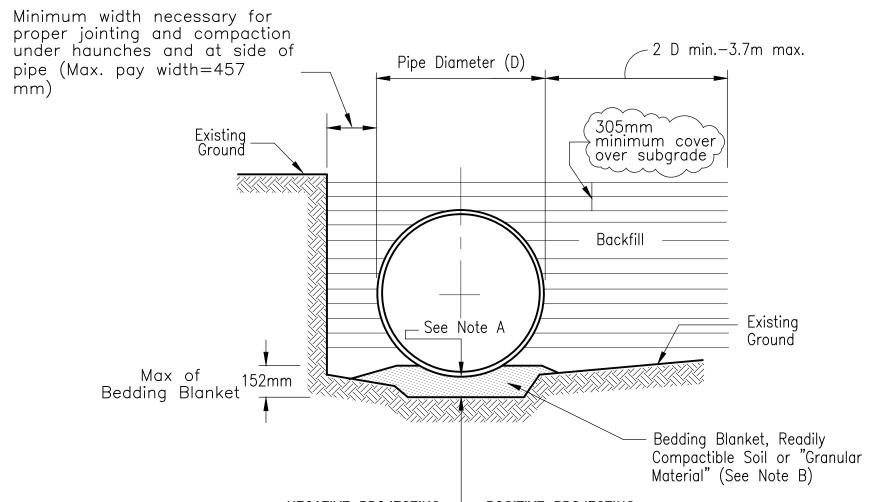




TYPICAL TURNOUT TRAFFIC MARKING

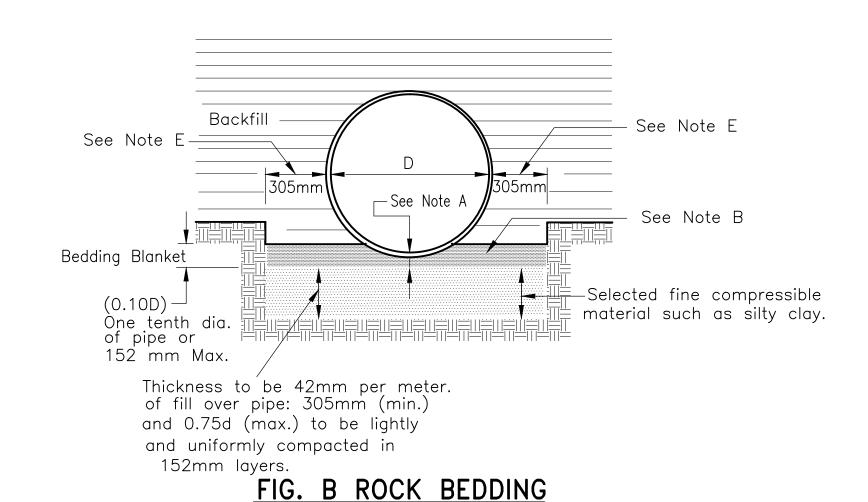


TYPICAL TURNOUTS WITH CSPC INSTALLATION



NEGATIVE PROJECTING POSITIVE PROJECTING

FIG. A. CLASS C BEDDING



Backfill

See Note A Loose "Granular Material"

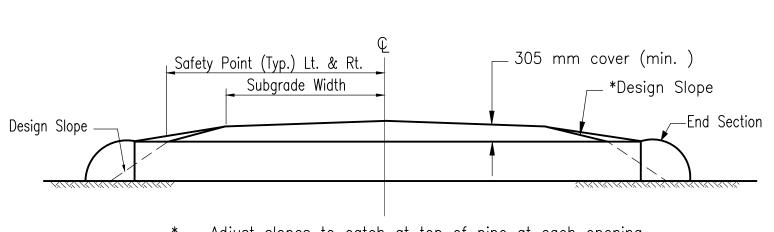
(See Note B)

Compacted "Granular material"

Specified under "Backfil".

3D

FIG. C. FOUNDATION STABILIZATION BEDDING



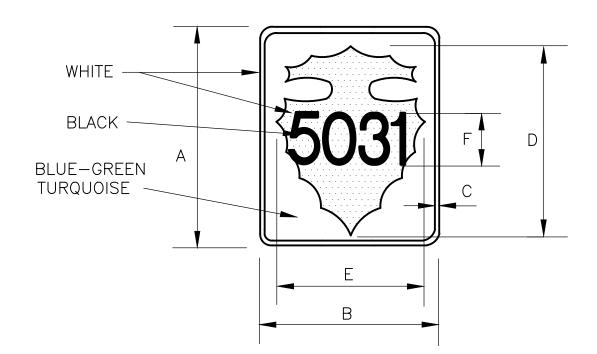
\* — Adjust slopes to catch at top of pipe at each opening

TYPICAL PIPE INSTALLATION

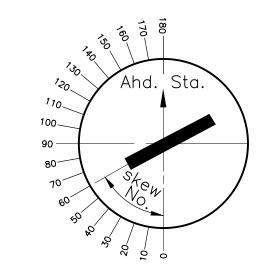
REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	NM	Navajo	N5031	N5031(1)1,2&4	22	31

# **GENERAL NOTES:**

- A. Place loose bedding roughly shaped to fit bottom of pipe, then compact under haunches after pipe placement.
- B. See sections 204, 209, 602, and 704 of FP-03, including the supplemental specification for additional notes
- C. All drainage structure material shall be unloaded and handled with reasonable care. No structure shall be dragged or allowed to strike any hard surface during placement. Any damaged structure shall be repaired or replaced, by Contractor, at no additional cost to the government.
- D. All structural plate pipes, arches, and super span structure shall be assembled and installed in accordance with the fabricators recommendation.
- E. Backfill material shall be placed minimum 305 mm up to (D/2) pipe diameter width on the sides. Backfill material beyond the limits shall be regular earthwork embankment material. The backfill material shall be approved by the COR/AOTR prior to it's use and shall be placed in accordance with the plans and specifications.
- F. Pounding or jetting pipe backfill shall not be permitted.
- G. All pipe excavation, backfill, de—watering, pumping or cofferdams required to properly install the drainage pipe shall be considered incidental to completion of the project and additional payment shall be made.
- H. Multiple pipe installations shall be placed 457 mm between end sections unless otherwise directed by the COR/AOTR or as shown on the plans.
- I. All pipes shall be protected by a cover of not less than 914 mm of embankment above pipe before any heavy equipment is allowed to pass over the structure(s) during construction.
- J. All culverts shall be installed at the original ground line and slope to assure positive drainage up to the R.O.W. limits. In no case shall the pipe be placed below the original ground elevations, unless as directed by the COR/AOTR. This work shall be considered incidental to completion of project and no additional payment shall be made.
- K. All culverts under turnouts and driveways shall be placed at the proposed ditch flowline and contractor shall adjust turnout profile grades to provide for 305 mm cover (To Subgrade) over pipe. The Contractor shall be required to make field adjustments in grades as directed by the COR/AOTR.



	SIGN	DIMENSION (mm)					F NUMERALS						
		Α	В	С	D	E	DIGITS IN ROUTE	1	2	3	4		
	MIN.	610	457	13	495	343	SIZE & SERIES (mm)	E 254	D 254	C 208	B 152		



# STRUCTURE SKEW DIAGRAM

UNITED STATES
DEPARTMENT OF THE INTERIOR
BUREAU OF INDIAN AFFAIRS
NAVAJO REGIONAL OFFICE – DIVISION OF TRANSPORTATION

# STANDARD PIPE INSTALLATION, TURNOUT & ROUTE SIGN DETAILS

Designed by:	B.O.R.	
Drawn by:	TAY	Date: 07-03
Checked by:	HRC	Date: 07-03
File Name:	Stdpipe.dw	g



REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	NM	Navajo	N5031	N5031(1)1,2&4	23	31

1. All 3 piece bodies to have 2.77 mm thick sides and 3.51 mm thick center panels. Multiple panel bodies shall have lap seams tightly joined together by

3. Type No. 3 connections is allowed and available for all CSPC's and CSPA's (except for the pipes with 76 mm by 25 mm corrugations). The stub

section shall have the same gage thickness as the end section, and shall

4. Type No. 5 connections are allowed and available for all CSPC's and CSPA's

5. All helical pipe joints shall be rerolled to allow for watertight band coupler.

**GENERAL NOTES** 

2. Types No. 1 & No. 2 for pipes with annular ends only.

be included in the unit bid price for the end section.

(except those pipes having 76 mm by 25 mm corrugations).

galvanized rivets or bolts.

# ESTIMATED STRUCTURE QUANTITIES

STATION	На	STRUCTURE DESCRIPTION	SKEW NO.	L.m	Ea.
10+210.000, CL	NA	1-711 mm x 508 mm x 22.82 m CSPA	150°	22.82	2
10+219.328, Rt.	NA	1-711 mm x 508 mm x 11.73 m CSPA	N/A	11.73	2
10+271.718, Rt.	NA	1-711 mm x 508 mm x 16.12 m CSPA	N/A	16.12	2
10+336.969, Lt.	NA	1-711 mm x 508 mm x 13.55 m CSPA	N/A	13.55	2
		SUBTOTAL	•	64.22	8

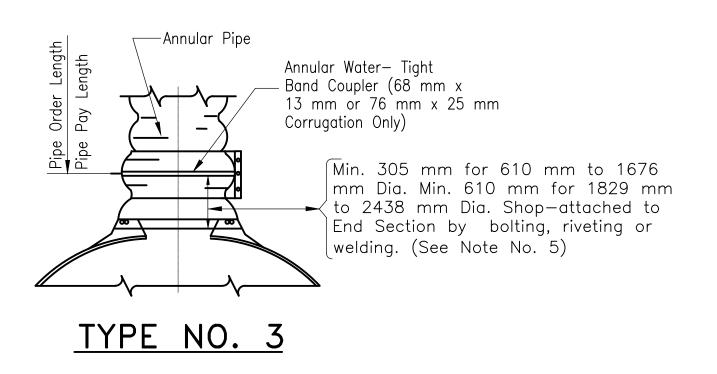
# 

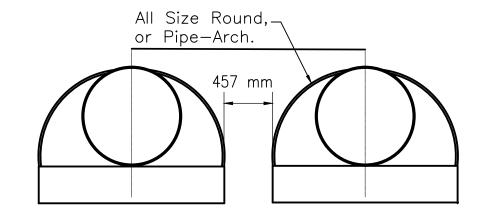
# TYPE NO. 1 For 305 mm thru 610 mm C.S.P. & 711 mm x 508 mm C.S.P.A. (See Note No. 4)

# TYPE NO. 2

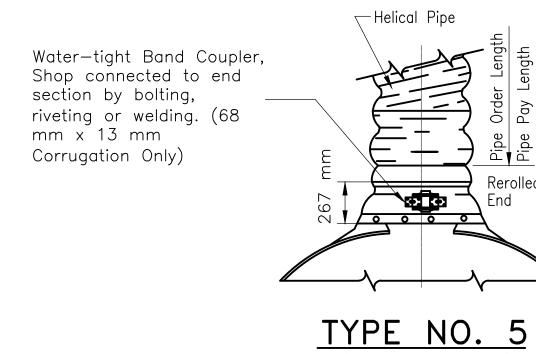
For 762 mm & 914 mm, and for 178 mm x 330 mm thru 1448 mm x 965 mm only (See Note No. 4)

ITEM NO.





MULTIPLE INSTALLATION SPACING



(SEE NOTE No. 6)

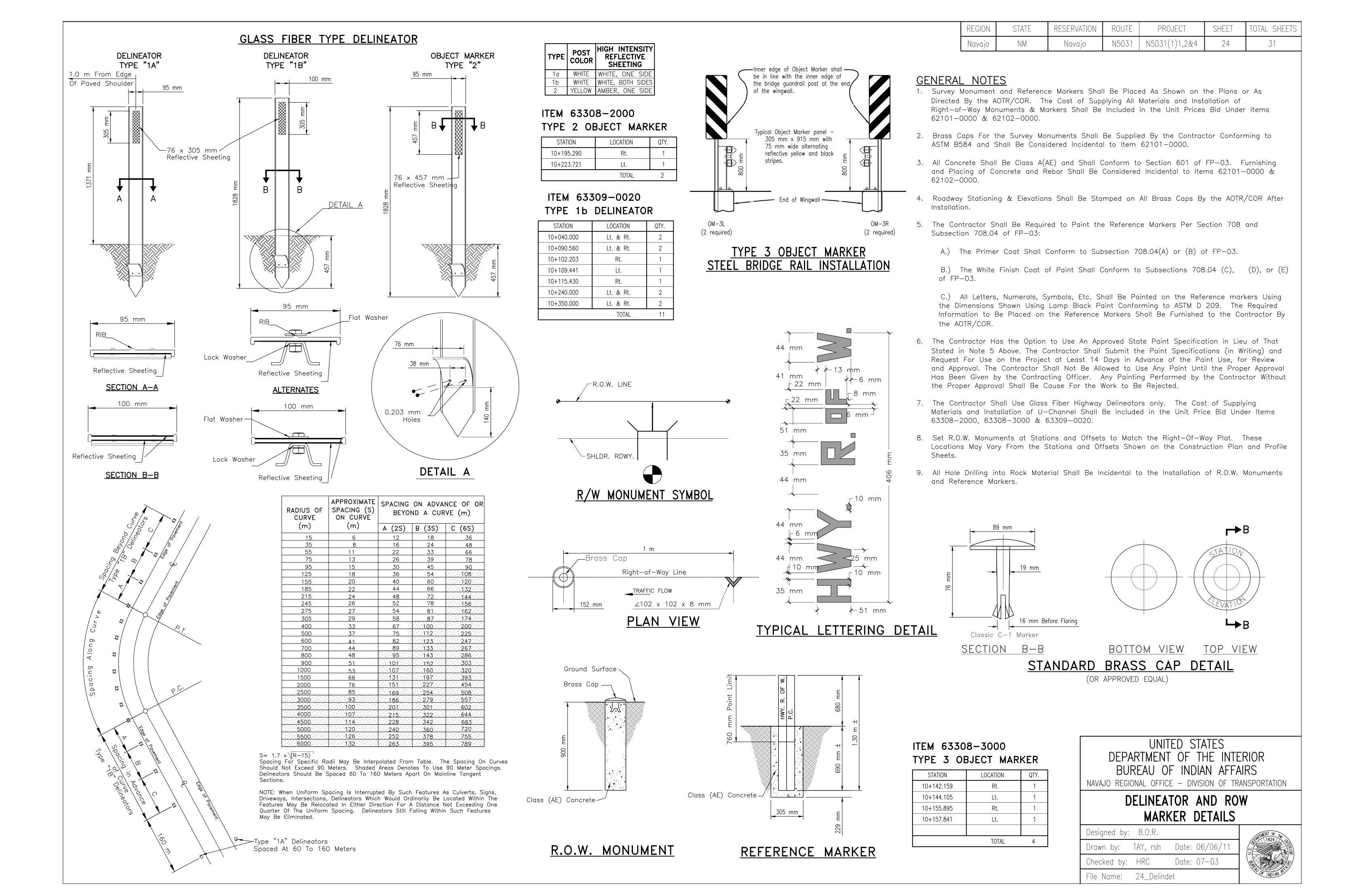
# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

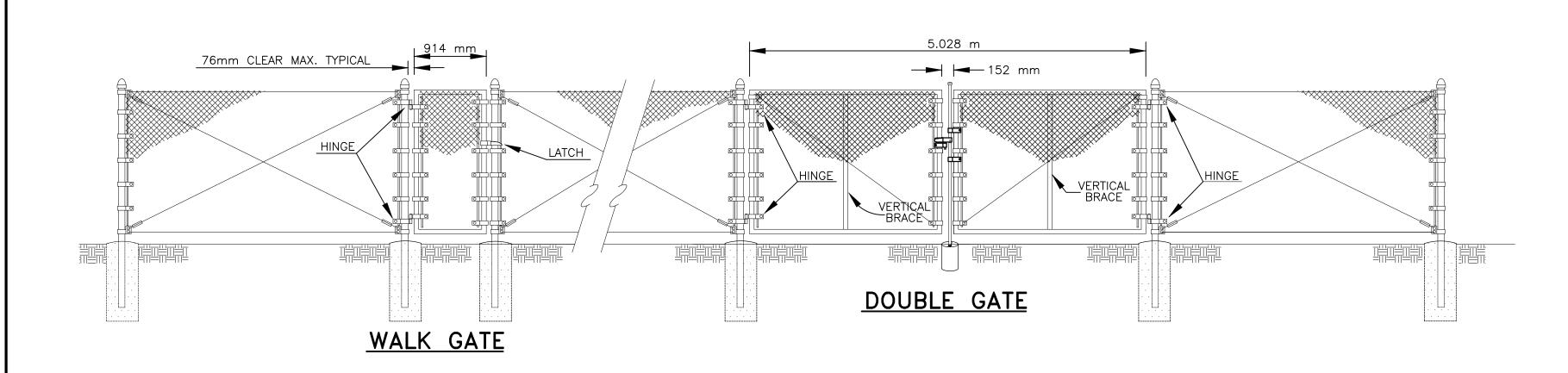
# PIPE CULVERT DETAILS

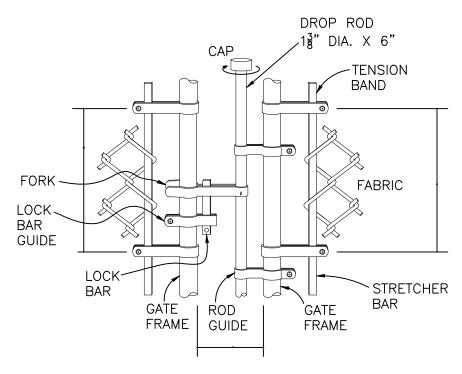
Designed by:	BOR – Str	uctural Unit
Drawn by:	TAY	Date: 07-03
Checked by:	HRC	Date: 07-03
File Name:	Culvpipe.dwg	]



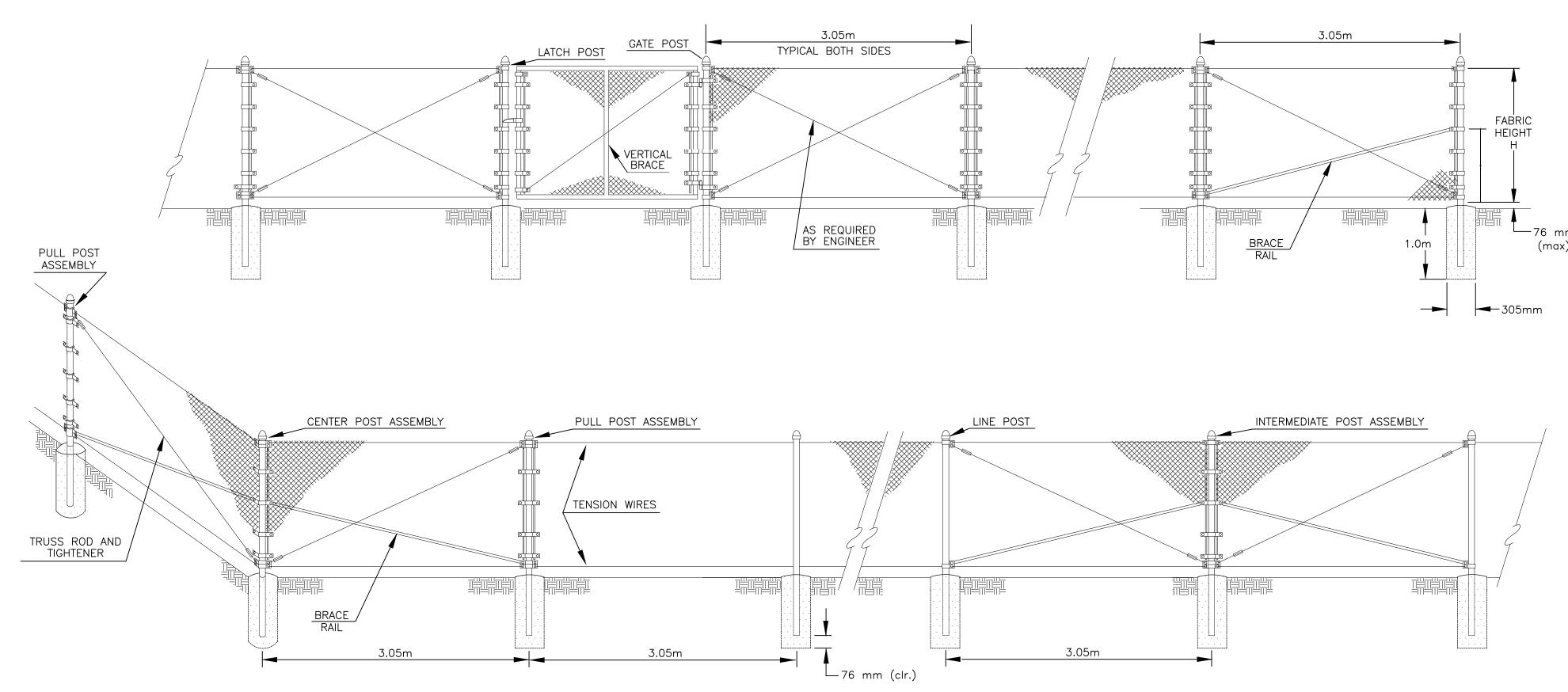


AREA	STATE	RESERVATION	ROUTE	PROJECT NO.	SHEET	TOTAL SHEETS
NAVAJO	NM	NAVAJO	N5031	N5031(1)1,2&4	25	31

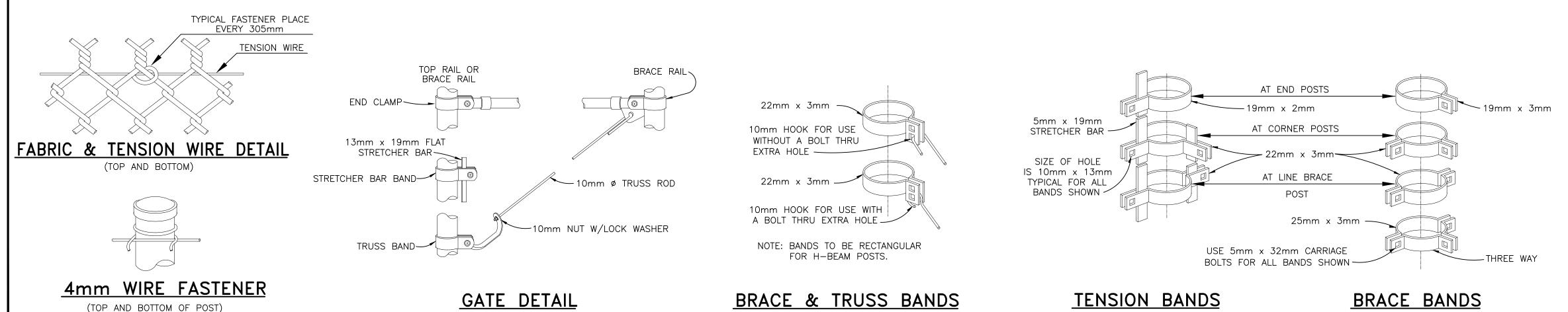




TYPICAL DROP ROD ASSEMBLY



TYPICAL CHAIN LINK FENCE INSTALLATION-TYPE 1 SHOWN



### **GENERAL NOTES**

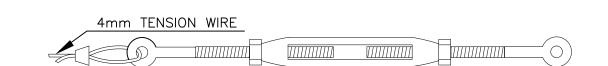
- 1. POSTS SHALL BE ROUND PIPE, H-SECTION OR ROLL-FORMED AND SHALL CONFORM TO THE NOMINAL DIMENSIONAL REQUIREMENTS SHOWN ON THE PLANS. IN ADDITION, THE MATERIAL OF WHICH POSTS ARE FABRICATED SHALL HAVE A NOMINAL THICKNESS, BEFORE GALVANIZING, OF NOT LESS THAN 3mm.
- 2. CHAIN LINK FABRIC SHALL BE EITHER ZINC-COATED OR ALUMINUM-COATED STEEL WIRE FENCE FABRIC. ZINC-COATED STEEL FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A392, CLASS 1 COATING. ALUMINUM-COATED STEEL FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A491, WITH A MINIMUM WEIGHT OF COATING OF 122 GRAMS PER SQUARE METER OF WIRE SURFACE AREA. THE FABRIC SHALL BE 3.05mm (Dia.) FOR ALL FENCE FABRIC 1829mm OR LESS IN HEIGHT AND SHALL BE 3.76mm (Dia.) FABRICS GREATER THAN 1829mm IN HEIGHT.
- 3. TENSION WIRES SHALL BE 4.50mm (Dia.) COIL SPRING STEEL WIRE WITH A MINIMUM TENSILE STRENGTH OF 517.12 MPa, AND SHALL BE ZINC—COATED OF ALUMINUM—COATED. TENSION AND BRACE BANDS TO BE INSTALLED ON MAXIMUM OF 40mm CENTERS.
- 4. TRUSS RODS SHALL BE 10mm DIAMETER ADJUSTABLE RODS. TRUSS TIGHTENERS SHALL HAVE A STRAP THICKNESS OF NOT LESS THAN 1.3mm.
- 5. STRETCHER BARS SHALL BE 5mm BY 19mm STEEL FLAT BARS.
  STRETCHER BAR BANDS SHALL BE 3mm BY 25mm PREFORMED STEEL
  BANDS
- 6. BOTTOM TENSION WIRE SHALL BE 127mm FROM TOP OF CROWN ON CONCRETE FOOTINGS.
- 7. INTERMEDIATE POST ASSEMBLIES SHALL BE SPACED AT 154m INTERVALS OR MIDWAY BETWEEN PULL POSTS WHEN THE DISTANCE BETWEEN SUCH POSTS IS LESS THAN 308m AND MORE THAN 154m.
- 8. CHAIN LINK FENCE POST DIAMETERS SHALL BE AS FOLLOWS:

  LINK POST (O.D.) = 60mm

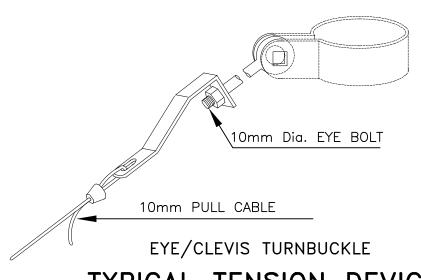
  CORNER, END, GATE, INTERMEDIATE = 73mm

LATCH, PULL POST

9. NEW DOUBLE GATE ASSEMBLIES SHALL INCLUDE ALL MATERIALS AND LABOR BETWEEN AND INCLUDING THE OUTER GATE POSTS. WHEN NEW GATE ASSEMBLIES ARE A PART OF EXISTING FENCE RELOCATION, THE RESET (EXISTING) MATERIAL SHALL STOP AT AND CONNECT TO THE OUTER GATE POST. IF THE ADJOINING FENCE IS EQUIPPED WITH A BARBED WIRE TOP, INSTALL A MATCHING BARBED WIRE TOP ON THE GATE ASSEMBLY FROM OUTER GATE POST TO OUTER GATE POST INCLUDING THE GATE LEAFS. ON THE GATE LEAFS, INSTALL BARBED WIRE SUPPORTS AT BOTH GATE ENDS AND AT EACH VERTICAL BRACE. FENCING BETWEEN GATE POSTS SHALL INCLUDE TOP AND/OR BOTTOM TENSION WIRES OR TOP RAIL AS NEED TO MATCH ADJOINING FENCING.



# 10mm EYE & EYE/TURNBUCKLE



TYPICAL TENSION DEVICES

UNITED STATES
DEPARTMENT OF THE INTERIOR
BUREAU OF INDIAN AFFAIRS
NAVAJO REGIONAL OFFICE \* DIVISION OF TRANSPORTATION

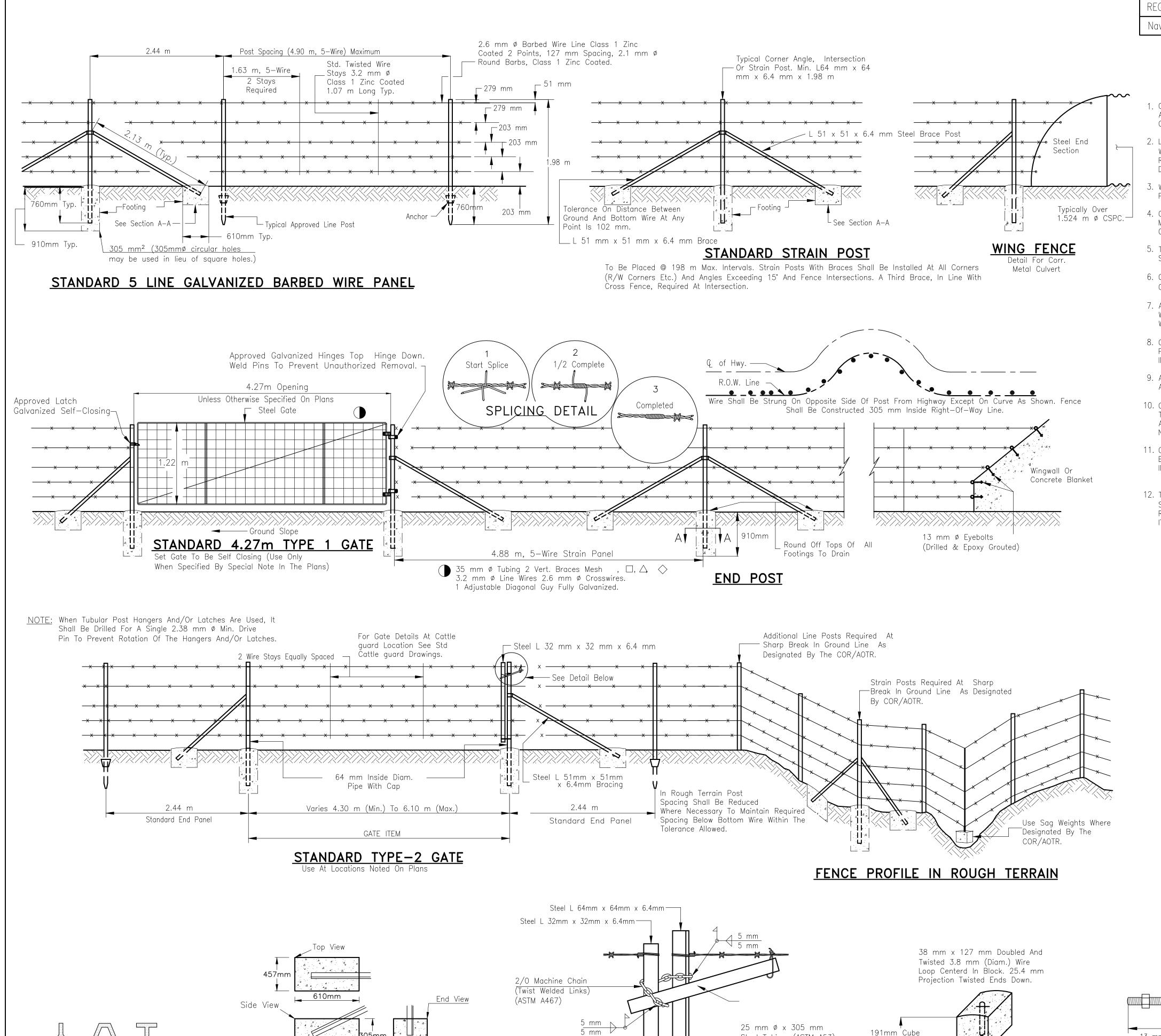
# CHAIN LINK FENCING DETAILS

DRAWN BY: BOR DATE: 1991

DESIGNED BY: BOR DATE: 1991

REVISED: 6/11 FILENAME: 25\_chain\_link\_fence
BY: STRUCT





TYPICAL STEEL POST SECTION

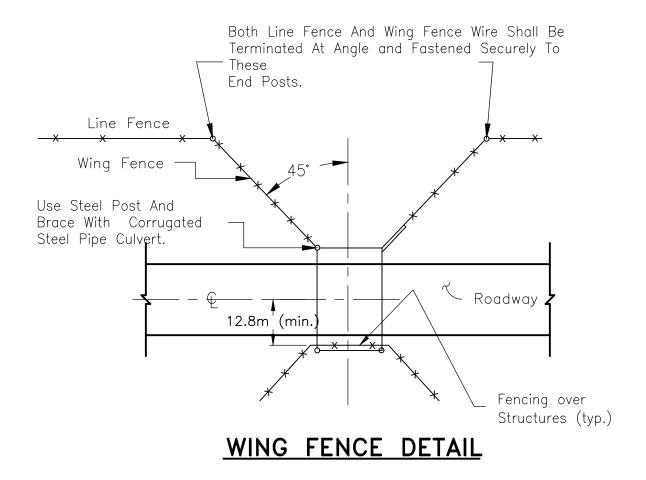
SECTION A-A

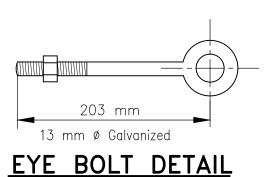
Concrete Footing of Corner Metal Posts with Bracing

RESERVATION PROJECT SHEET OTAL SHEETS REGION ROUTE NM N5031 N5031(1)1,2&4 26 31 Navajo Navajo

### **GENERAL NOTES**

- . CORNER, GATE, INTERMEDIATE BRACE POSTS AND LINE POSTS SHALL BE EITHER GALVANIZED OR PAINTED IN ACCORDANCE WITH AASHTO M281. METAL POST AND BRACES SHALL BE FABRICATED FROM RAIL, BILLET, OR COMMERCIAL GRADE STEEL CONFORMING WITH THE REQUIREMENT OF ASTM A702.
- 2. LINE POSTS SHALL BE FABRICATED IN ACCORDANCE WITH AASHTO M281, AND SHALL HAVE A NOMINAL WEIGHT OF 1.98 kg/m EXCLUSIVE OF ANCHOR PLATES. ANCHOR PLATES SHALL BE CLAMPED, WELDED OR RIVETED TO THE SECTION IN SUCH A MANNER AS TO PREVENT DISPLACEMENT WHEN THE POSTS ARE
- WHEN LINE POST ANCHORS ARE OMITTED, DUE TO CHANGE IN SOIL CONDITIONS SUCH AS ROCK, THEN THE POSTS SHALL BE SET IN CONCRETE. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO ITEM 61921-1000.
- 4. CONCRETE FOR ANCHORS, POST HOLES, ETC. SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 20.7 MPa IN 28 DAYS AND SHALL CONFORM TO SECTION 601 OF THE FP-03. FURNISHING AND PLACEMENT OF CONCRETE SHALL BE INCLUDED WITH ITEM 61921-1000.
- 5. TWO SPLICES ON THE SAME LINE BETWEEN THE STRAIN POST ASSEMBLIES SHALL NOT BE PERMITTED. NO SPLICES SHALL BE PLACED CLOSER THAN 30 METER OF ANY POST ASSEMBLIES.
- 6. CONNECT ALL R.O.W. FENCING TO CATTLE GUARDS, CULVERTS (GREATER THAN 1.524m DIA.), AND CONCRETE STRUCTURES AS SHOWN ON THESE PLANS, AND/OR AS DIRECTED BY THE COR/AOTR.
- 7. ANY CONFLICT IN PLACEMENT OF THE R/W FENCING AT DRAINAGE PIPE LOCATION, DUE TO NARROW R/W WIDTH OR OTHER CONSTRICTIONS, THE FENCE MAY BE PLACED OVER THE DRAINAGE STRUCTURE. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO ITEM 61921-1000.
- 8. CLEARING AND GRUBBING SHALL INCLUDE SHAPING AND/OR REMOVAL OF SMALL MOUNDS NECESSARY TO PRESENT A SMOOTH UNIFORM APPEARANCE OF BOTH GROUND AND FENCING LINE. THIS WORK SHALL BE INCIDENTAL TO THE INSTALLATION OF FENCING AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- 9. ALL DRILLING INTO ROCK MATERIAL, ETC. SHALL BE INCIDENTAL TO THE INSTALLATION OF FENCING AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- 10. GATE CLOSURE DEVICE SHALL BE STEEL PIPE, NPS 3/4 (26.7 mm Ø ) SCHEDULE 40, CONFORMING TO THE REQUIREMENT OF ASTM A53. THE GATE CLOSURE STEEL CHAIN SHALL BE WELDED TO THE STEEL PIPE AND ANGLE IRON FENCE POST. THIS WORK SHALL BE INCIDENTAL TO THE INSTALLATION OF FENCING AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- 11. CONTRACTOR SHALL BE REQUIRED TO INSTALL SAG WEIGHTS WHERE VERTICAL CLEARANCE BETWEEN THE BOTTOM WIRE AND NATURAL GROUND IS 610mm OR GREATER. THIS WORK SHALL BE INCIDENTAL TO THE INSTALLATION OF FENCING.
- 12. TIE WIRE, WIRE FASTENERS OR WIRE CLIPS FOR FASTENING BARBED AND WOVEN FABRIC FENCING TO THE STEEL POSTS SHALL BE 3.0mm DIA. STEEL WIRE, CLASS 1 (ZINC COATED), SOFT TEMPER AND MEET THE REQUIREMENTS OF ASTM A641. FURNISHING AND PLACEMENT OF FASTENERS SHALL BE INCLUDED WITH ITEM 61921-1000.





191mm Cube

CONCRETE SAG WEIGHT DETAIL

Min. Weight Of Concrete Sag Shall Be 16 kg.

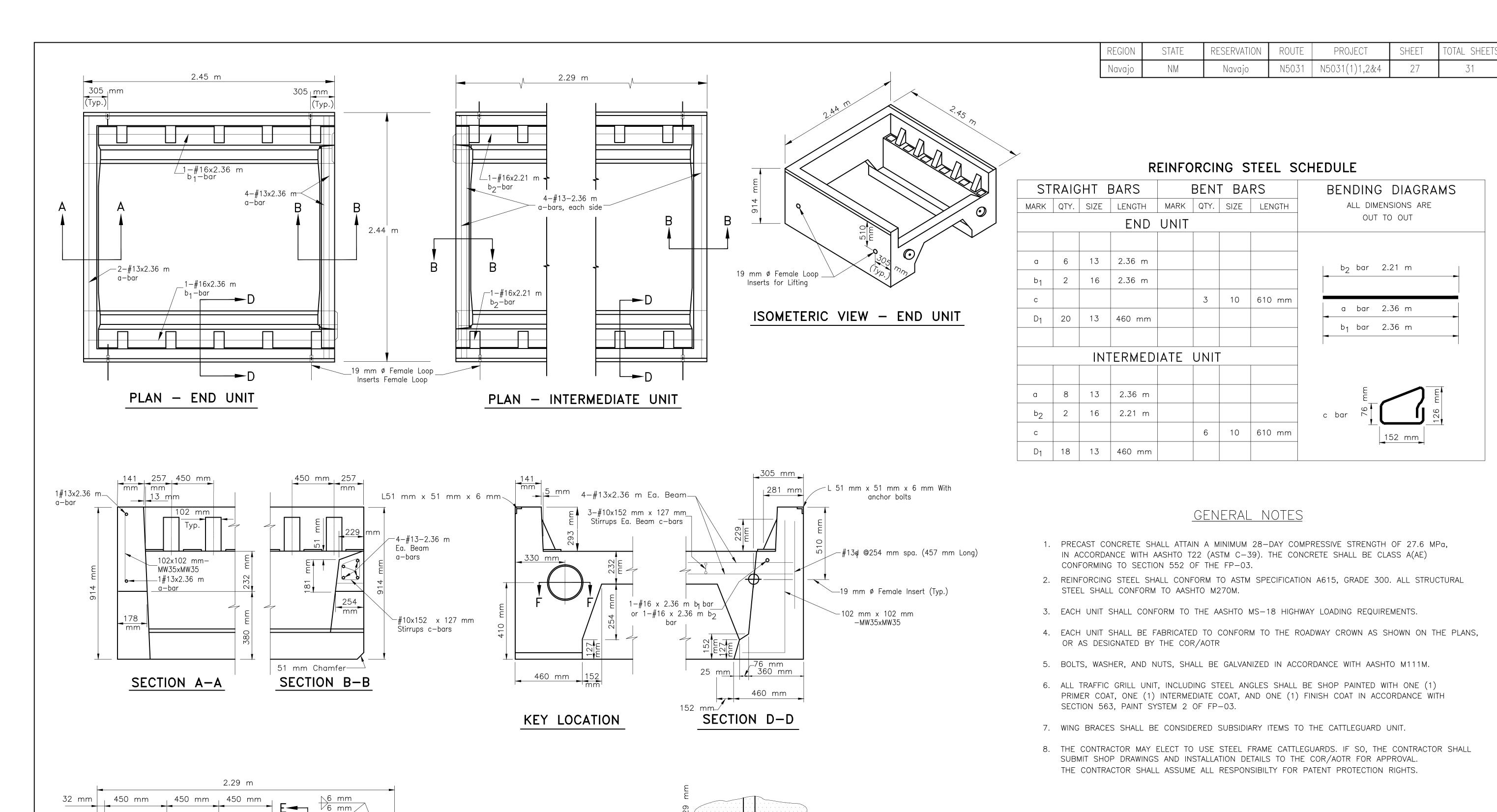
Steel Tubing. (ASTM A53)

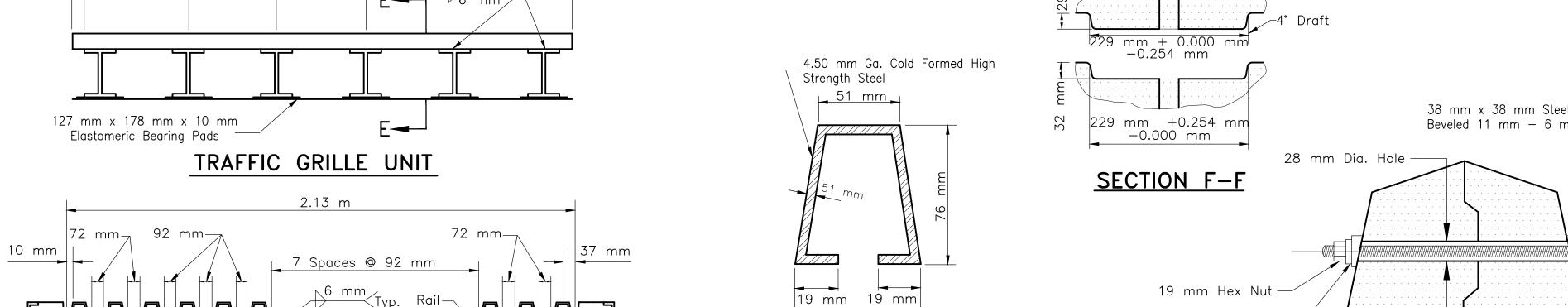
GATE SECURING DETAIL

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

# FENCING DETAILS

Designed by:	NRDOT			
Drawn by:	DESIGN2	Date:	01/01	
Checked by:	HRC	Date:	07-03	
File Name:	26_BARBW	/IREfence		



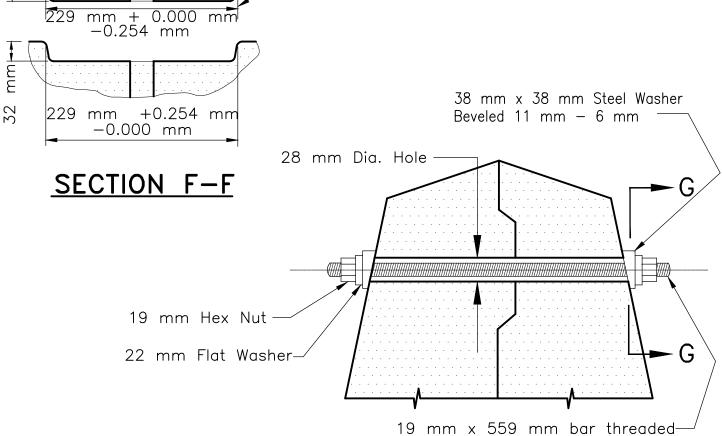


W 200x27

SECTION E-E



89 mm



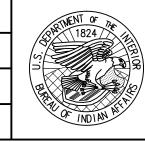
TIE BOLT DETAIL

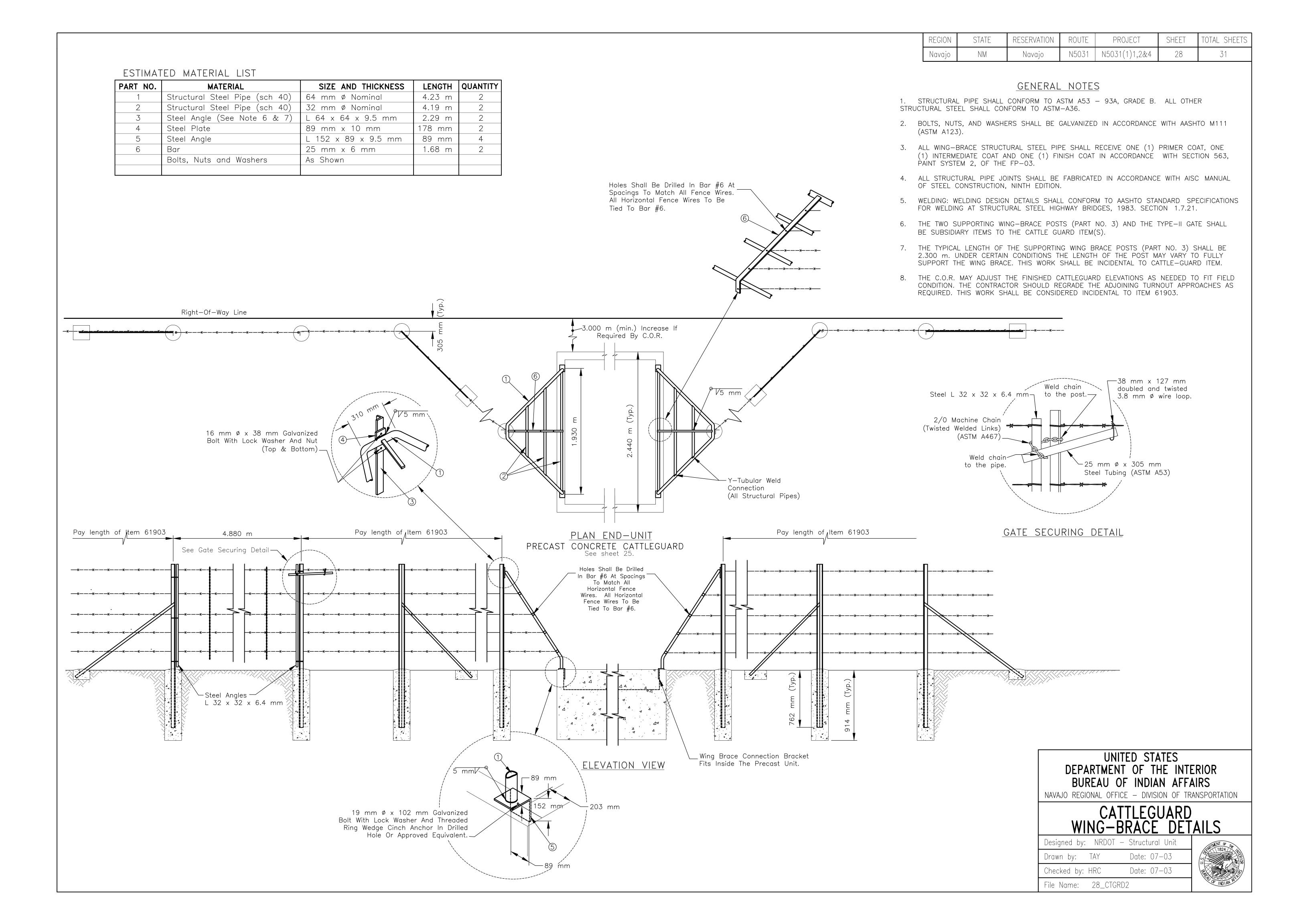
SECTION G-G

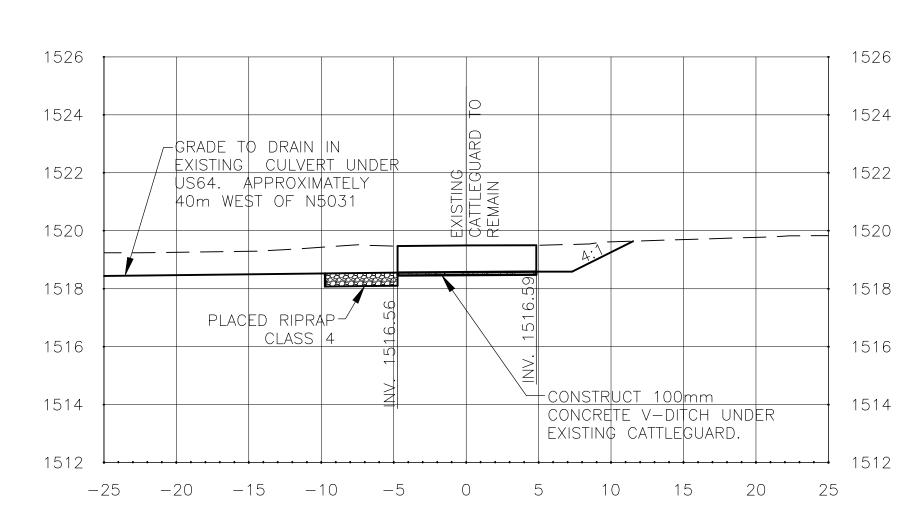
# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

# PRECAST CONCRETE CATTLEGUARD DETAILS

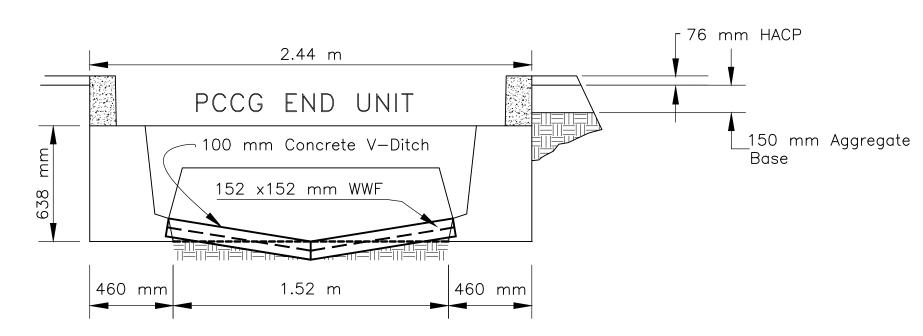
Designed by: BOR - Structural Unit Drawn by: TAY Date: 07-03 Checked by: HRC Date: 07-03 File Name: 27\_CTGRD1



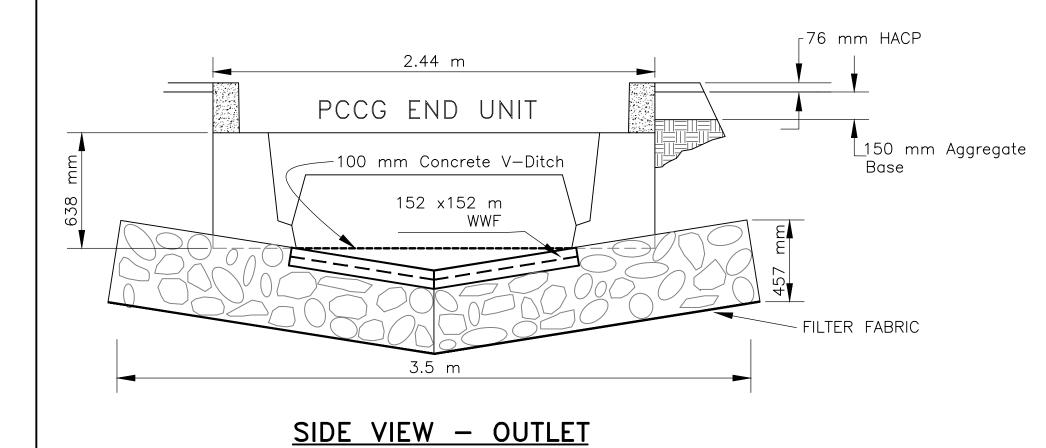




# CATTLEGUARD CROSS SECTION DETAILS 10+006.969



SIDE VIEW - INLET

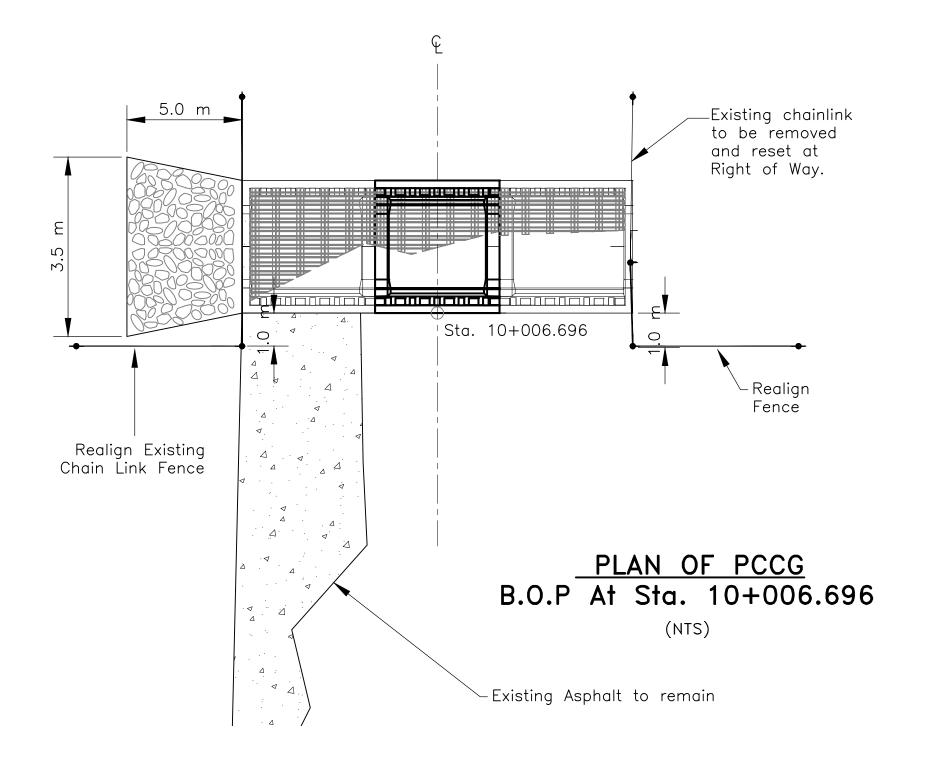


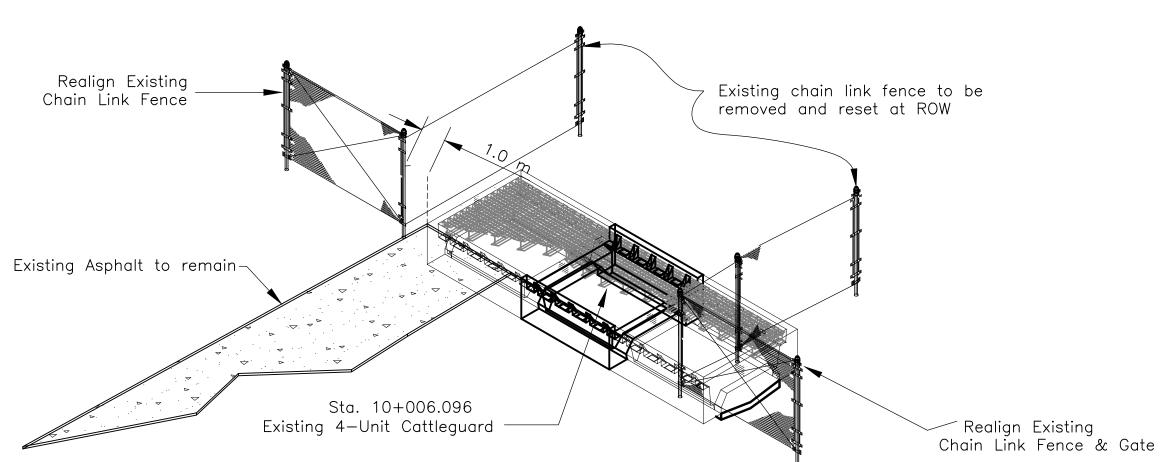
ITEM 60101-0000: CONCRETE

LOCATION	No. OF PCCG UNITS	LENGTH OF PAD (m)		TOTAL Wt. Of WWF @ 2 kg/m² (kg)
0+839.50 — Mainline	4	9.573	1.45	29.10
		TOTAL	1.45	29.10

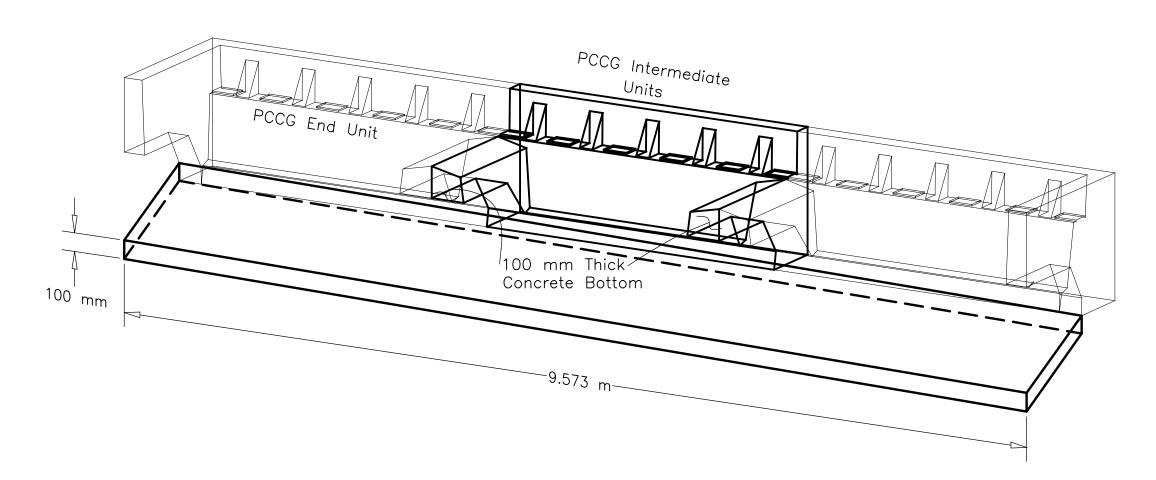
# ITEM 25101-4000: PLACED RIPRAP, CLASS 4

LOCATION	LENGTH (m)	WIDTH (m)	THICKNESS (m)	TOTAL VOLUME (m³)	
0+839.50 - Rt.	5.00	3.5	0.457	7.99	
<b>TOTAL</b> 7.99					





TYPICAL ISOMETRIC EXPLODED VIEW OF PCCG
At Sta. 10+006.969



TYPICAL ISOMETRIC SECTION VIEW PCCG & CONCRETE BOTTOM

(NTS)

REGION	STATE	RESERVATION	ROUTE	PROJECT	SHEET	TOTAL SHEETS
Navajo	NM	Navajo	N5031	N5031(1)1,2&4	29	31

### GENERAL NOTES

- 1. WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR CONSTRUCTION OF ROADS AND BRIDGES ON FEDERAL HIGHWAY PROJECTS (FP-03), ALONG WITH THE SUPPLEMENTAL SPECIFICATIONS FOR THIS PROJECT.
- 2. ALL CONCRETE SHALL BE CLASS A(AE) WITH CLASS 1 FINISH. CHAMFER ALL EXPOSED EDGES 19 mm. ALL CONCRETE SHALL CONFORM TO SECTION 552 & 601 OF THE FP-03. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF F'C = 20.7 MPa IN 28 DAYS.
- 3. REINFORCING STEEL SHALL CONFORM TO AASHTO SPECIFICATION M31M ASTM A 615M, GRADE 280, AND SECTION 554 OF FP-03. ALL BEND DIMENSIONS FOR REINFORCING STEEL SHALL BE OUT TO OUT OF BARS. ALL REINFORCING STEEL SHALL HAVE 51 mm CLEARANCE COVER UNLESS OTHERWISE NOTED. ALL REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO COMPLETION OF STRUCTURE.
- 4. IN NO CASE SHALL ANY BACKFILL BE PLACED UNTIL THE CONCRETE HAS ATTAINED A COMPRESSIVE STRENGTH OF 17.2 MPa.
- 5. ALL STRUCTURE EXCAVATIONS SHALL BE DONE TO NEAT LINES AND WILL BE CONSIDERED INCIDENTAL TO COMPLETION OF THE STRUCTURE.
- 6. FOOTINGS SHALL BE PLACED TO THE ELEVATIONS SHOWN. IF UNSUITABLE MATERIAL IS FOUND AT THE FOOTING LOCATION AND ELEVATIONS, THE MATERIAL SHALL BE REMOVED AND REPLACED WITH APPROVED STRUCTURAL BACKFILL AS DETERMINED BY THE COR/AOTR ALL STRUCTURAL BACKFILL SHALL BE COMPACTED TO 95% OF MAXIMUM DENSITY AS DETERMINED BY AASHTO T99 METHOD C, BEFORE AND AFTER FOOTINGS ARE PLACED. THE STRUCTURAL BACKFILL MATERIAL SHALL CONFORM TO SECTION 208 AND 209 OF THE FP-03.
- 7. THE CONTRACTOR WILL BE REQUIRED TO MAKE FIELD ADJUSTMENTS AS DIRECTED BY THE COR/AOTR TO MATCH FIELD CONDITIONS. THESE ADJUSTMENTS SHALL BE CONSIDERED INCIDENTAL TO COMPLETION OF PROJECT AND NO ADDITIONAL PAYMENT SHALL BE MADE.
- 8. SLOPES RESHAPING, CLEANING, AND EXCAVATION SHALL BE DONE IN ACCORDANCE WITH THE PLANS AND AS DETERMINED BY THE COR/AOTR ANY WASTE MATERIAL SHALL BE USED AS BORROW WHERE NEEDED IN OTHER PROJECT LOCATION AS DESIGNATED AND APPROVAL BY THE COR/AOTR ALL EXCAVATION, CLEANING, AND RESHAPING SHALL BE CONSIDERED INCIDENTAL TO COMPLETION OF THE STRUCTURE.

# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF INDIAN AFFAIRS

NAVAJO REGIONAL OFFICE - DIVISION OF TRANSPORTATION

# CONCRETE DRAINAGE PAD DETAILS FOR PCCG

Designed by:	NRDOT	
Drawn by:	HRC	Date: 08-03
Checked by:	HRC	Date: 08-03
File Name:	29_ST-(	CATTLEDRAIN



