NTU ENVIRONMENTAL LAB CHINLE

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CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023

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Early Work Package 1 1.20.23 Plans

Issued: 1.26.23

DRAWING INDEX

G001 TITLE SHEET & DRAWING INDEX

1
CIVIL
C100 GENERAL NOTES
C200 CIVIL SITE PLAN
C300 GRADING & DRAINAGE PLAN
C400 WATER & SEWER PLAN
C500 EROSION CONTROL PLAN
C600 CIVIL DETAILS
C601 CIVIL DETAILS
C610 WATER DETAILS
C611 WATER DETAILS
C620 SEWER DETAILS

STRUCTURAL S001 ABBREVIATIONS AND LEGEND S002 GENERAL STRUCTURAL NOTES S003 SPECIAL INSPECTION TABLES (2015 CONDENSED) S101 FOUNDATION PLAN S121 LOW ROOF AND HIGH ROOF FRAMING PLAN S301 WALL SECTIONS S302 WALL SECTIONS S501 FOUNDATION SECTIONS & DETAILS S511 FRAMING SECTIONS & DETAILS S601 SCHEDULES S701 TYPICAL CONCRETE DETAILS S702 TYPICAL CONCRETE DETAILS S711 TYPICAL STEEL CONNECTION DETAILS S731 TYPICAL COLD-FORMED DETAILS S741 TYPICAL MISC STEEL DETAILS

ARCHITECT

Revision Schedule

Date Description

PROJECT NUMBER DRAWN BY PROJ MGR Project # Author Designer

RVT FILE

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MATERIALS AND WORKMANSHIP

THE MATERIALS AND WORKMANSHIP SHALL BE DONE IN ACCORDANCE WITH THE FOLLOWING CURRENT SPECIFICATIONS, THESE PLANS AND GENERALLY ACCEPTED GOOD CONSTRUCTION PRACTICES. IN THE EVENT OF A CONFLICT BETWEEN THE SPECIFICATIONS AND THE PLANS, THE PLANS WILL TAKE PRECEDENCE. IN THE ABSENCE OF CONFLICT BETWEEN THE SPECIFICATIONS AND THE PLANS, THE PLANS SUPPLEMENT AND ADD TO THE SPECIFICATIONS. "LATEST EDITION" MEANS THE MOST RECENT SPECIFICATION OR STANDARD IN EFFECT AS OF THE DATE OF THE ENGINEER'S SEAL ON THESE PLANS.

PROJECT SPECIFICATIONS

WATER AND WASTEWATER FACILITIES, LATEST EDITION.

MARICOPA ASSOCIATION OF GOVERNMENTS, "UNIFORM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION", LATEST EDITION (MAG SPECIFICATIONS).

U.S. DEPARTMENT OF TRANSPORTATION, FEDERAL HIGHWAY ADMINISTRATION, "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS" (MUTCD).

INTERNATIONAL BUILDING CODE (IBC), LATEST EDITION (UNLESS SPECIFIED OTHERWISE BY THE OWNER). AMERICAN WATER WORKS ASSOCIATION (AWWA), "STANDARDS", SECTIONS C AND D, LATEST EDITION.

NAVAJO TRIBAL UTILITY AUTHORITY (NTUA), TECHNICAL SPECIFICATIONS FOR MATERIALS AND WORKMANSHIP FOR

IF TWO OR MORE GIVEN SPECIFICATIONS DIFFER IN CONTENT. THE MORE RESTRICTIVE OR STRINGENT STANDARD OR SPECIFICATION, IN THE OPINION OF THE ENGINEER, WILL GOVERN.

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN COPIES OF THE ABOVE STANDARDS, SPECIFICATIONS AND DETAILS, AS WELL AS ALL OTHER STANDARDS AND SPECIFICATIONS WHICH MAY BE NECESSARY TO COMPLETE AND ACCURATELY INTERPRET THESE PLANS. THIS REQUIREMENT EXTENDS TO ANY STANDARDS, DETAILS OR SPECIFICATIONS REFERENCED BY THE CONSTRUCTION DOCUMENTS AND NOT INCLUDED IN THE LIST ABOVE. THE CONTRACTOR AS STATED HEREIN SHALL MEAN THE GENERAL CONTRACTOR AND HIS ASSOCIATED SUBCONTRACTORS. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR THE COORDINATION AND PERFORMANCE OF THE WORK OF ALL OF HIS SUBCONTRACTORS AND SUPPLIERS.

SITE WORK INCLUDES BUT NOT LIMITED TO: SITE CLEARING, GRUBBING, DEMOLITION, DEBRIS REMOVAL FROM THE SITE, IMPORT AND/OR EXPORT OF SOILS AND OTHER MATERIALS TO AND FROM THE SITE, BORROW MATERIALS, TEMPORARY SOILS MATERIAL STOCKPILING, CUT AND FILL, SLOPES, SOIL AND BANK STABILIZATION AND PROTECTION, BERMING, RELOCATIONS, STRUCTURE EXCAVATIONS, TRENCHING, ALL BACKFILLING, SITE GRADING, PAVING, PIPING, UTILITY LINE AND STORM DRAINAGE CONSTRUCTION, SITE CONCRETE WORK, EROSION CONTROL, DUST CONTROL AND OTHER MISCELLANFOUS SITE WORK STRUCTURES AND ITEMS INDICATED ON THE PLANS AND THE CONTRACT DOCUMENTS. ALL GRUBBING AND WASTE MATERIAL SHALL BE REMOVED FROM THE SITE PER THE DIRECTION OF THE OWNER OR HIS REPRESENTATIVE. THE USE OF EXPLOSIVES WILL NOT BE PERMITTED.

THE ENGINEER WILL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OR FOR SAFETY PRECAUTIONS OR PROGRAMS UTILIZED IN CONNECTION WITH THE WORK, AND WILL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE ENGINEER IS NOT RESPONSIBLE FOR COORDINATING THE RELOCATIONS OF UTILITIES, POWER POLES,

SHOP DRAWINGS SHALL BE PROVIDED BY THE CONTRACTOR PER PROJECT SPECIFICATIONS.

THE ENGINEER MAY ORDER ANY OR ALL MATERIALS USED IN THE WORK TO BE TESTED ACCORDING TO AASHTO AND ASTM STANDARDS. THE CONTRACTOR SHALL, AT HIS EXPENSE, SUPPLY CERTIFICATES OR RESULTS OF TESTING. ALL WORK AND MATERIALS NOT CONFORMING TO SPECIFICATIONS OR PERFORMED WITHOUT THE CONSENT OF THE OWNER OR HIS AUTHORIZED REPRESENTATIVE WILL BE SUBJECT TO REJECTION BY THE OWNER AND/OR ENGINEER AND REPLACED AT THE CONTRACTOR'S EXPENSE.

THE CONTRACTOR SHALL GUARD AGAINST DAMAGE DURING CONSTRUCTION TO ADJACENT PROPERTIES, FENCES, WALLS AND UTILITY EQUIPMENT.

THE CONTRACTOR SHALL SUBMIT CAREFULLY DOCUMENTED AND CONSIDERED WRITTEN PROPOSALS FOR ADDITIONAL WORK OR ALTERNATE MATERIALS TO THE ENGINEER FOR APPROVAL. NO WORK ON ADDITIONAL WORK ITEMS SHALL BE PERFORMED UNTIL WRITTEN APPROVAL IS GRANTED BY THE OWNER. ANY WORK PERFORMED WITHOUT THE KNOWLEDGE AND APPROVAL OF THE OWNER OR HIS REPRESENTATIVE IS SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.

UNDERGROUND UTILITY LOCATIONS, AS SHOWN ON THESE PLANS, WERE DETERMINED FROM FIELD MEASUREMENTS, CONSTRUCTION PLANS, RECORD PLANS, OR UTILITY MAPS FURNISHED BY OTHERS. LOCATIONS OF UNDERGROUND LITHITIES ARE TO BE REGARDED AS APPROXIMATE ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ESTABLISH IN THE FIELD THE ACTUAL LOCATIONS OF ALL UNDERGROUND LINES WHICH MAY IN ANY WAY AFFECT THE WORK. COMPENSATION FOR UTILITY RELOCATIONS AND ADJUSTMENTS SHALL NOT INCLUDE ANY COSTS FOR REPAIR TO THE

UTILITY DAMAGED BY THE CONTRACTOR OR HIS SUBCONTRACTOR(S). THE CONTRACTOR IS NOT RELIEVED OF THE RESPONSIBILITY FOR DETERMINING THE LOCATION OF ALL UTILITIES AFFECTING THE WORK. THE CONTRACTOR SHALL NOT BE RELIEVED OF RESPONSIBILITY FOR MAKING COMPLETE AND ACCURATE ON—SITE DETERMINATIONS OF THE LOCATIONS OF ALL UTILITIES, STRUCTURES AND FIELD CONDITIONS, WHICH MAY AFFECT THE

PROGRESS OF THE WORK. WHERE PLANS CALL FOR CONNECTING NEW STRUCTURES TO EXISTING UNDERGROUND PIPES OR STRUCTURES, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE, AT THE TIME OF CONSTRUCTION, EXACT SIZES, TYPES AND LOCATIONS OF EXISTING UNDERGROUND IMPROVEMENTS AND TO FURNISH MATERIALS AS NEEDED TO MAKE THE

ALL IMPROVEMENTS SHALL BE CONSTRUCTED BY CONTRACTORS LICENSED BY THE ARIZONA STATE REGISTRAR OF CONTRACTORS, WITH A CLASS OF LICENSE(S) FOR THE SPECIFIC WORK BEING PERFORMED (UNLESS OTHERWISE SPECIFIED BY THE OWNER).

THE CONTRACTOR IS RESPONSIBLE FOR ALL CONSTRUCTION METHODS, SEQUENCING, AND SAFETY DURING

THE CONTRACTOR IS REQUIRED TO COMPLY WITH ALL TRIBAL, STATE AND FEDERAL LAWS AND REGULATIONS APPLICABLE TO THE CONSTRUCTION OF THIS PROJECT.

NO EXISTING SURVEY MONUMENTS SHALL BE REMOVED OR DISTURBED BY THE CONTRACTOR. THE CONTRACTOR SHALL BE FINANCIALLY RESPONSIBLE FOR COSTS TO REESTABLISH MONUMENTS OR CONTROLS REMOVED WITHOUT PRIOR NOTICE AND APPROVAL.

CONTRACTOR SHALL OBTAIN ALL PERMITS AT HIS OWN EXPENSE FROM LOCAL COUNTY, STATE, FEDERAL AND TRIBAL AGENCIES AND AIR POLLUTION CONTROL AUTHORITIES PRIOR TO BEGINNING CONSTRUCTION. REQUIRED PERMITS SHALL BE SECURED BY THE CONTRACTOR FROM THE APPROPRIATE AGENCIES. FEDERAL PERMITS,

PAVING AND CONCRETE WORK

ALL PAVING IS TO BE IN ACCORDANCE WITH SPECIFICATIONS SECTIONS AND ANY OTHER SECTION OR OTHER SPECIFICATION REFERENCED THEREIN OR REFERENCED ON PLANS. SUBGRADE PREPARATION - SPECIFICATION AND GEOTECHNICAL REPORT UNTREATED AGGREGATE BASE (ABC) - SPECIFICATION SECTION 31 2000 AND GEOTECHNICAL REPORT CONCRETE CURB & GUTTERS, SIDEWALKS SIDEWALK RAMPS, DRIVEWAYS - SPECIFICATION SECTION 32 1313 ASPHALT PAVEMENT - SPECIFICATION SECTION 32 1216

GRADING AND EARTHWORK

ALL GRADING AND EARTHWORK SHALL BE PER GEOTECHNICAL ENGINEERING REPORT, PROJECT SPECIFICATIONS OR ANY OTHER SPECIFICATION REFERENCED THEREIN OR REFERENCED ON PLANS.

- SECTION 31 1000 EARTH MOVING - SECTION 31 2000 - SECTION 31 3700 - SECTION 32 1216 CONCRETE PAVING - SECTION 32 1313 CONCRETE PAVING JOINT SEALANTS - SECTION 32 1373 PARKING SAFETY CURB SECTION 32 1713 - SECTION 33 0000 STORM UTILITY DRAINAGE PIPING - SECTION 33 4100

CONTRACTOR SHALL VERIFY FROM SOILS REPORT RECOMMENDATIONS THE DEGREE OF DIFFICULTY REQUIRED FOR TRENCHING AND EXCAVATION WORK BASED ON DEPTH AND TYPES OF MATERIALS TO BE ENCOUNTERED. VATER SOURCE: THE CONTRACTOR SHALL MAKE THE NECESSARY ARRANGEMENTS FOR OBTAINING ALL WATER REQUIRED FOR SOIL COMPACTION, DRINKING PURPOSES AND DUST CONTROL WITH NAVAJO NATION WATER CODE

TOPSOIL THAT WILL BE AFFECTED BY ROUGH GRADING OR EXCAVATION SHALL BE STOCKPILED ON THE SITE SEPARATELY AND SHALL NOT BE USED FOR FILL, BUT SHALL BE CONSERVED AND USED FOR FINE AND FINISH

THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM ALL WALLS AND FOUNDATIONS. ALL STORM DRAINS, DRAIN LINES, OVERFLOWS, OUTLETS, AND/OR OTHER DRAINAGE TYPE OUTLETS WHICH CONDUCT MOISTURE NEAR THE STRUCTURES SHALL BE POSITIVELY DRAINED AWAY FROM THE STRUCTURE. NO WATER SHALL BE PERMITTED TO POND NEAR STRUCTURES OR FOUNDATIONS. ALL DRAINAGE SHALL BE CHANNELIZED AND SHALL BE DIRECTED AWAY FROM THE BUILDING STRUCTURE AT A MINIMUM 5% FOR 10FT.

SUBGRADE PREPARATION: IF THE NATURAL SUBGRADE IS LESS THAN THE REQUIRED DENSITY, IT SHALL BE SCARIFIED AND COMPACTED TO A MINIMUM DEPTH AS NOTED IN THE PROJECT SOILS REPORT IMMEDIATELY PRIOR TO PLACING SUBSEQUENT FILL MATERIAL THEREON. THE CONTRACTOR IS RESPONSIBLE FOR MAINTENANCE AND REPAIR OF DAMAGE TO PREPARED SUBGRADE CAUSED BY CONTRACTORS OPERATIONS OR PUBLIC TRAFFIC. NO MATERIALS SHALL BE PLACED UPON THE PREPARED SUBGRADE UNTIL IT MEETS THE SPECIFIED REQUIREMENTS. SUBGRADE COMPACTION INCLUDES SUBGRADE UNDER DRIVEWAY, CURB. SIDEWALKS, SHOULDERS AND FILL SLOPES. SUBGRADE TOLERANCES SHALL BE AS SPECIFIED IN PROJECT SPECIFICATIONS AND THE PROJECT SOILS REPORT. OPEN LANDSCAPED AREAS SHALL BE GRADED TO +/- 0.20 FEET.

EARTH FILL: AREAS TO BE FILLED SHALL BE LEVELED TO PROVIDE A LEVEL BASE TO SUPPORT FILL MATERIALS. SUBGRADE AND SUBBASE REQUIRING FILL MATERIAL SHALL BE SCARIFIED MOISTENED AND COMPACTED PRIOR TO PLACING FILL. ALL FILL, SUBGRADE AND SUBBASE MATERIALS SHALL BE COMPACTED TO SPECIFIED DENSITIES AT OR NEAR OPTIMUM MOISTURE CONTENTS AS VERIFIED AND RECOMMENDED BY THE SOILS ENGINEER. PLACE FILL IN HORIZONTAL LIFTS NOT EXCEEDING EIGHT INCHES IN LOOSE THICKNESS BEFORE UNLESS OTHERWISE DIRECTED IN GEOTECHNICAL REPORT COMPACTION. SLOPED SURFACES SHALL BE PLOWED, STEPPED, AND BENCHED SO THAT THE FILL MATERIAL WILL BOND WITH THE EXISTING MATERIAL. SEE PROJECT SPECIFICATIONS AND THE PROJECT SOILS

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT THE SOILS ENGINEER FOR SCHEDULING OF COMPACTION TESTING. THE FREQUENCY OF DENSITY AND MOISTURE TESTS REQUIRED FOR ADEQUATE CONTROL SHALL BE THE RESPONSIBILITY OF THE SOILS ENGINEER WHO SHALL CERTIFY TO THE OWNER, ENGINEER AND CONTRACTOR AT PROJECT END THAT THE FILL IS COMPACTED PER THESE PLANS AND SPECIFICATIONS. COMPACTION SHALL BE

ACHIEVED BY MECHANICAL MEANS. IN NO CASE SHALL STRUCTURE BACKFILLING BE FLOOD WATER SETTLED.

ALL SLOPE CONSTRUCTION AND ROADWAY EXCAVATION SHALL CONFORM TO THE REQUIREMENTS OF PROJECT SPECIFICATIONS AND IBC. CUT AND FILL SLOPES SHALL BE AS INDICATED ON THE PLANS, SHALL BE PROVIDED WITH THE APPROPRIATE BENCHES AS SPECIFIED IN THE REFERENCE DOCUMENTS AND INDICATED ON THE PLANS. ALL FILL SLOPES SHALL BE COMPACTED AS EACH LIFT OF FILL MATERIAL IS PLACED. ALL CUT AND FILL SLOPES SHALL BE UNIFORMLY GRADED TO LINES AND GRADES INDICATED. TOPS OF ALL CUT SLOPES SHALL BE ROUNDED AND ALL UNSTABLE AND LOOSE MATERIAL AT TOP OF SLOPE SHALL BE REMOVED.

UNLESS OTHERWISE SPECIFIED, COMPACT TO THE FOLLOWING SPECIFIED PERCENT OF MAXIMUM DENSITY AS DETERMINED IN ACCORDANCE WITH ASTM D698, PROJECT SPECIFICATIONS, AND PROJECT SOILS REPORT. MINIMUM PERCENT COMPACTION SUBGRADE SOIL OR AS DIRECTED IN THE PROJECT SOILS REPORT:

• PAVED AREAS (PAVEMENTS, SIDEWALKS, & PADS) — 95% • AGGREGATE BASE COURSE - 100%

DIRECTED OTHERWISE BY ENGINEER.

 BACKFILL AROUND STRUCTURES — 90% • BACKFILL FOR UTILITY TRENCHES (PER NTUA TECHNICAL SPECIFICATIONS). • BACKFILL FOR TRENCHES WITHIN 10' OF STRUCTURES AND WALLS SHALL BE COMPACTED TO 95% UNLESS

WATERING: CAREFULLY WATER EARTH FILL DURING PLACING BY MEANS OF A FINE SPRAY OR OTHER APPROVED METHOD, SO THAT EACH LAYER IS THOROUGHLY AND UNIFORMLY WETTED. MOISTURE CONTENT OF THE MATERIAL SHALL BE CAREFULLY CONTROLLED AT ALL TIMES AND CHECKED AT PROPER INTERVALS TO INSURE CORRECT MOISTURE FOR COMPACTION SEE THE PROJECT SPECIFICATIONS AND THE PROJECT SOILS REPORT.

TRENCHES: BACKFILLING TRENCHES SHALL PROGRESS AS RAPIDLY AS THE CONSTRUCTION AND TESTING OF THE WORK PERMIT. TRENCH BACKFILL FOR WATER, SEWER, AND UTILITY LINES SHALL CONFORM TO NTUA TECHNICAL

MAINTENANCE AND PROTECTION: THE CONTRACTOR IS RESPONSIBLE FOR MAINTENANCE, CLEANING AND PROTECTION OF ALL CONSTRUCTION WORK UNTIL FINAL COMPLETION, APPROVAL OF PROJECT AND ACCEPTANCE BY OWNER, ENGINEER AND GOVERNING AUTHORITY. SETTLEMENT OF WASHING THAT OCCURS IN GRADED, TOPSIDE, OR BACKFILLED AREAS PRIOR TO ACCEPTANCE OF THE WORK SHALL BE REPAIRED AND GRADING RE-ESTABLISHED TO THE REQUIRED ELEVATIONS AND SLOPES.

THE SITE OPERATOR (CONTRACTOR), SHALL MAINTAIN AND MANAGE THE STORMWATER POLLUTION PREVENTION PLAN (SWPPP), FILE THE NOTICE OF INTENT, IMPLEMENT THE REQUIREMENTS OF SWPPP, AND FILE THE NOTICE OF TERMINATION. THE CONTRACTOR SHALL MINIMIZE POLLUTANTS IN STORMWATER DISCHARGES USING BEST MANAGEMENT PRACTICES (BMP'S) FOR EROSION CONTROL.

MATERIAL TESTING STANDARDS

ALL TESTING SHALL BE PER SPECIFICATIONS AND NTUA TECHNICAL SPECIFICATIONS.

WATER AND SEWER NOTES

CONTRACTOR SHALL NOTIFY LOCAL DISTRICT OFFICE PRIOR TO EXCAVATING WATER AND/OR WASTEWATER UTILITIES. CONTRACTOR SHALL OBTAIN APPROVED PERMISSION TO TAP FOR PRIOR TO TAPPING EXISTING WATER MAIN AND

ALL WATER AND WASTEWATER MAIN LINES SHALL BE PRESSURED TESTED PER N.T.U.A. TECHNICAL SPECIFICATIONS. ALL WATER AND WASTEWATER MAINS AND SERVICE LINES SHALL MEET ALL HORIZONTAL AND SEPARATION REQUIREMENTS PER N.T.U.A. TECHNICAL SPECIFICATIONS.

WASTEWATER SEWER SERVICE LINES SHALL HAVE ONE—WAY CLEANOUTS AT EVERY BEND.

ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH CURRENT NTUA TECHNICAL SPECIFICATIONS AND DETAILS.

ADDITIONS, PER AWWA STANDARDS AND SPECIFICATIONS. WATER SERVICE LINES TRENCHING EXCAVATION, BACKFILLING AND COMPACTION - SECTION TP 1.0 WATERLINE TESTING - SECTION TP 3.09 WATER DISINFECTION - SECTION TP 3.10 WATER THRUSTBLOCKING - SECTION TP 3.05 FIRE HYDRANT ASSEMBLY - SECTION TP 3.03 SEWER PIPE INSTALLATION - SECTION TP 4.04 - SECTION TP 4.05 MANHOLE INSTALLATION SEWER SERVICE INSTALLATIONS - SECTION TP 4.07 SEWER LINE TESTING - SECTION TP 4.08

NTUA WATER AND WASTEWATER GENERAL NOTES:

ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE N.T.U.A. TECHNICAL SPECIFICATIONS FOR MATERIALS AND WORKMANSHIP FOR WATER AND WASTEWATER FACILITIES, SEPTEMBER 2008. 2. CONTRACTOR SHALL OBTAIN APPROVED PERMISSION TO TAP (PTT) FORM PRIOR TO TAPPING EXISTING WATER AND

SEWER MAIN. 3. CONTRACTOR SHALL COORDINATE WITH N.T.U.A. PRESENT AT THE SITE TO VERIFY LOCATION, DEPTH, SIZE AND TYPE OF UNDERGROUND UTILITIES. 4. CONTRACTOR TO COORDINATE WITH N.T.U.A. REGARDING WATER SHUT OFF WITH AT LEAST 3 DAYS ADVANCE

NOTICE TO ISOLATE LINE(S), TO NOTIFY AFFECTED CUSTOMERS AND TO MINIMIZE OUTAGE TIME PRIOR TO CONNECTION OF NEW WATER SERVICE. UNLESS OTHERWISE DIRECTED, ONLY AUTHORIZED N.T.U.A. STAFF WILL BE ALLOWED TO CLOSE/OPEN WATER

VALVES FOR ANY CONNECTIONS TO EXISTING LINES AND FOR THE USAGE OF WATER. 6. ROUGH GRADING SHALL BE COMPLETED WITHIN 1/10 OF A FOOT OF PLAN GRADE PRIOR TO INSTALLING WATER AND WASTEWATER UTILITIES. COORDINATE WITH LOCAL UTILITY PROVIDERS FOR ANY REMOVAL OF EXISTING FACILITIES PRIOR TO CONSTRUCTION.

CONTRACTOR SHALL PROTECT EXISTING UTILITIES IN PLACE. CONTRACTOR SHALL RESTORE AT OWN EXPENSE ANY DAMAGE TO EXISTING UTILITIES. 9. ALL WATER MAINS SHALL BE POLYVINYL CHLORIDE (PVC) PRESSURE PIPE WITH A MINIMUM OF 200 PSI PRESSURE CLASS UNLESS SPECIFIED OTHERWISE. 10. ALL FITTINGS AND VALVES 4" OR GREATER IN SIZE SHALL BE MADE FROM DUCTILE IRON FURNISHED WITH

MECHANICAL JOINT ENDS AND SHALL HAVE A PRESSURE RATING OF 350 PSI. ALL MJ ENDS SHALL HAVE "MEGALUG" MECHANICAL RESTRAINT WITH CONCRETE THRUST BLOCK, STD DTL WS-19. POLYETHYLENE WRAPPING (8 MILS MINIMUM THICKNESS IN ACCORDANCE WITH AWWA STANDARD C-105) SHALL BE INSTALLED AROUND DUCTILE IRON PIPES, FITTINGS, AND VALVES, FIRE HYDRANT BARRELS AND RODS AND CLAMPS. 11. A WARNING/DETECTABLE MARKING TAPE SHALL BE INSTALLED AT 12" MINIMUM AND 18" MAXIMUM ABOVE THE WATER AND SEWER PIPE. MARKING TAPE SHALL CONSIST OF ONE LAYER OF ALUMINUM FOIL LAMINATED BETWEEN TWO COLORED LAYERS OF INERT PLASTIC FILM. THE LAMINATION BOND SHOULD BE STRONG ENOUGH THAT THE LAYERS CANNOT BE SEPARATED BY HAND. TAPE SHALL BE A MINIMUM OF 5 MILS THICK AND 6 INCHES WIDE. APE SHALL BEAR A CONTINUOUS, PRINTED MESSAGE EVERY 16 TO 36 INCHES WARNING OF THE INSTALLATION BURIED BELOW. TAPE SHALL BE TERRA TAPE, LINETEC, OR AS APPROVED BY N.T.U.A. 12. DEFLECTION (VERTICAL OR HORIZONTAL) OF PIPES IS PERMITTED AND SHALL CONFORM TO AMERICAN WATER WORKS ASSOCIATION (AWWA) JOINT DEFLECTION FOR AWWA PRESSURE PIPE AND AT 80% OF MANUFACTURER'S RECOMMENDED MAXIMUM DEFLECTION, WHICHEVER IS MORE STRINGENT. A COPY OF THE MANUFACTURER'S

RECOMMENDATION SHALL BE SUBMITTED TO N.T.U.A. 13. PIPES SHALL NOT BE BACKFILLED (INCLUDING BEDDING MATERIAL ABOVE THE SPRING LINE OF THE PIPE) UNTIL THE CONSTRUCTION HAS BEEN INSPECTED AND APPROVED FOR BACKFILLING BY A N.T.U.A REPRESENTATIVE. 14. CONTRACTOR SHALL PROVIDE AS-BUILT DRAWINGS OF ALL UTILITIES TO OWNER AND N.T.U.A. IN HARD COPY AND

15. HYDROSTATIC TESTING, FLUSHING AND CHLORINATION (SHALL BE CONDUCTED IN ACCORDANCE WITH N.T.U.A. TECHNICAL SPECIFICATIONS FOR MATERIALS AND WORKMANSHIP FOR WATER AND WASTEWATER FACILITIES, SEPTEMBER 2008) SHALL BE COORDINATED WITH N.T.U.A. AT LEAST 3 DAYS IN ADVANCE. N.T.U.A. REPRESENTATIVE SHALL BE PRESENT TO RECORD THE INFORMATION AND CERTIFY THE TESTING. 16. UTILITY CONSTRUCTION ACCEPTANCE AND UTILITY TRANSFER: CONTRACTOR SHALL SCHEDULE A FINAL OR PRE-FINAL INSPECTION WITH N.T.U.A. HQ ENGINEERING AND LOCAL DISTRICT OFFICE, OWNER AND GENERAL CONTRACTOR AT THE END OF CONSTRUCTION. DOCUMENTS SHALL BE PROVIDED BY THE CONTRACTOR AS FOLLOWS: AS-BUILT DRAWING, APPROVED PTT, HYDROSTATIC TESTING RESULTS, BACTERIOLOGICAL TESTING RESULTS, MATERIALS SUBMITTALS AND COST OF PLANT. PLEASE FOLLOW THE N.T.U.A. TECHNICAL SPECIFICATIONS FOR MATERIALS AND WORKMANSHIP FOR WATER AND WASTEWATER FACILITIES, SEPTEMBER 2008, SECTION TP 5.0. 17. THE CONTRACTOR/ENGINEER SHALL PROVIDE WARRANTY ON ALL NEW WATER AND WASTEWATER FACILITIES AGAINST DEFECTS IN MATERIALS, WORKMANSHIP AND FOR ANY DESIGN DEFICIENCIES, ERRORS AND OMISSIONS FOR

THE PERIOD OF ONE YEAR WHEN THE FACILITIES WERE INSPECTED, ACCEPTED AND APPROVED. 18. PROVIDE 10 FT. MINIMUM HORIZONTAL SEPARATION IN SEPARATE TRENCHES BETWEEN THE WATER AND SEWER SERVICES. PROVIDE 5 FT. MIN. HORIZONTAL SEPARATION BETWEEN THE SEWER SERVICES AND OTHER UTILITIES. IF SEWER SERVICES CROSSES OTHER SERVICES. SEE N.T.U.A. CROSSING POLICY, OR CONTACT N.T.U.A. 19. ALL SEWER PIPES SHALL BE MADE OF MATERIAL CONFORMING TO REQUIREMENTS OF ASTM D1784, TYPE I, GRADE

CONNECTIONS TO NEW SEWER MAINS SHALL BE WYE FITTING 20. SEWER CLEANOUTS ARE REQUIRED ON ALL BENDS IN EXCESS OF 45 DEGREES AS PER PLUMBING CODE ADOPTED BY THE NAVAJO NATION. MODIFY MATERIAL LIST ACCORDINGLY AFTER CONSULTING WITH N.T.U.A. HEADQUARTERS

I FOR RIGID POLYVINYL CHLORIDE COMPOUNDS. ALL SEWER PIPES AND FITTINGS SHALL BE AT LEAST SDR-35,

TYPE PSM WITH ELECTROMETRIC GASKET JOINTS MEETING THE REQUIREMENTS OF ASTM D 3034. SERVICE

21. ADDITIONAL SEWER CLEANOUTS ARE REQUIRED ON SEWER SERVICES LONGER THAN 50 FT. AS PER UNIFORM PLUMBING CODE ADOPTED BY THE NAVAJO NATION. MODIFY MATERIAL LIST ACCORDINGLY AFTER CONSULTING WITH N.T.U.A. HEADQUARTERS ENGINEERING. EACH ADDITIONAL CLEANOUT IS AT THE CUSTOMER'S EXPENSE. INSTALL AT LEAST ONE CLEANOUT AS REQUIRED. IF CUSTOMER REQUESTS FEWER AND REALIZED THAT IT VIOLATES NAVAJO TRIBAL CODE. THEN INSTALL PER THE CUSTOMER'S REQUEST AND NOTE ON THE INDIVIDUAL AS-BUILT, N.T.U.A. RECOMMENDS THAT CLEANOUTS BE SPACED NO MORE THAN 100'.

22. PROVIDE PROPOSED ELEVATION AT WALL. PROVIDE 6 IN. DIAMETER SLEEVE IF PIPING PENETRATES WALL OR 4 INCH DEPTH OF SAND BETWEEN FOOTING AND TOP OF PIPING IS BELOW THE FOOTING. ORDER ASTM D-1785 SCH. 40 PIPE WITH LENGTH AS NEEDED FOR THE SLEEVE. CONTACT N.T.U.A. HEADQUARTERS ENGINEERING ON PIPING SMALLER THAN 2 INCH IN SIZE.

23. MINIMUM SLOPE OF 1/4 INCH PER FOOT (2%) OR AS APPROVED BY N.T.U.A. HEADQUARTERS ENGINEERING. 24. BACKFILL IS TO BE HAND TAMPED (NO MECHANICAL) AND COMPACTED IN 6 INCH LAYERS FOR AT LEAST 12 IN. ABOVE PVC PIPE. INSTALL PER ASTM D-2321 AND UNIFORM PLUMBING CODE ADOPTED BY NAVAJO NATION.

25. NEW SEWER MAIN SHALL BE INSTALLED WITH 42" OF MINIMUM COVER PER STANDARD DETAIL WWS-10, UNLESS OTHERWISE INDICATED ON THESE PLANS. 26. ALL NEWLY CONSTRUCTED LINES ARE REQUIRED TO BE CLAMPED AND TESTED FOR EXFILTRATION. TESTING IS TO F CONDUCTED WITH NAVAJO AREA STANDARDS AND CONSTRUCTION REQUIREMENTS TP 4.08. THE ENGINEER REQUIRED TO COMPLETE THE CERTIFICATION FORMS AS APPROPRIATE. THE COMPLETED FORMS ARE TO BE FORWARDED BY THE ENGINEER TO THE N.T.U.A. HEADQUARTER. SPECIAL PROJECT DEPARTMENT, FOR REVIEW AND

APPROVAL. A LETTER OF ACCEPTANCE OF THE TEST RESULTS WILL BE FORWARDED FROM THE N.T.U.A.

PUBLIC UTILITIES NAVAJO TRIBAL UTILITY AUTHORITY NAVAJO TRIBAL UTILITY AUTHORITY TELEPHONE 1-800-528-5011

NAVAJO NATION SANITATION

TOPOGRAPHIC SURVEYOR

TOPOGRAPHIC SURVEY PREPARED BY SOUDER, MILLER & ASSOCIATES, INC. CONTACT: (928) 428-9141. CONTACT SURVEYOR FOR HORIZONTAL CONTROL INFORMATION.

NTU ENVIRONMENTAL LAB CHINLE

CHINLE, APACHE COUNTY, AZ EARLY DESIGN PACKAGE # 1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023



2733 E. Lakin Dr.

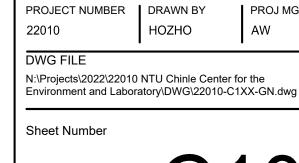
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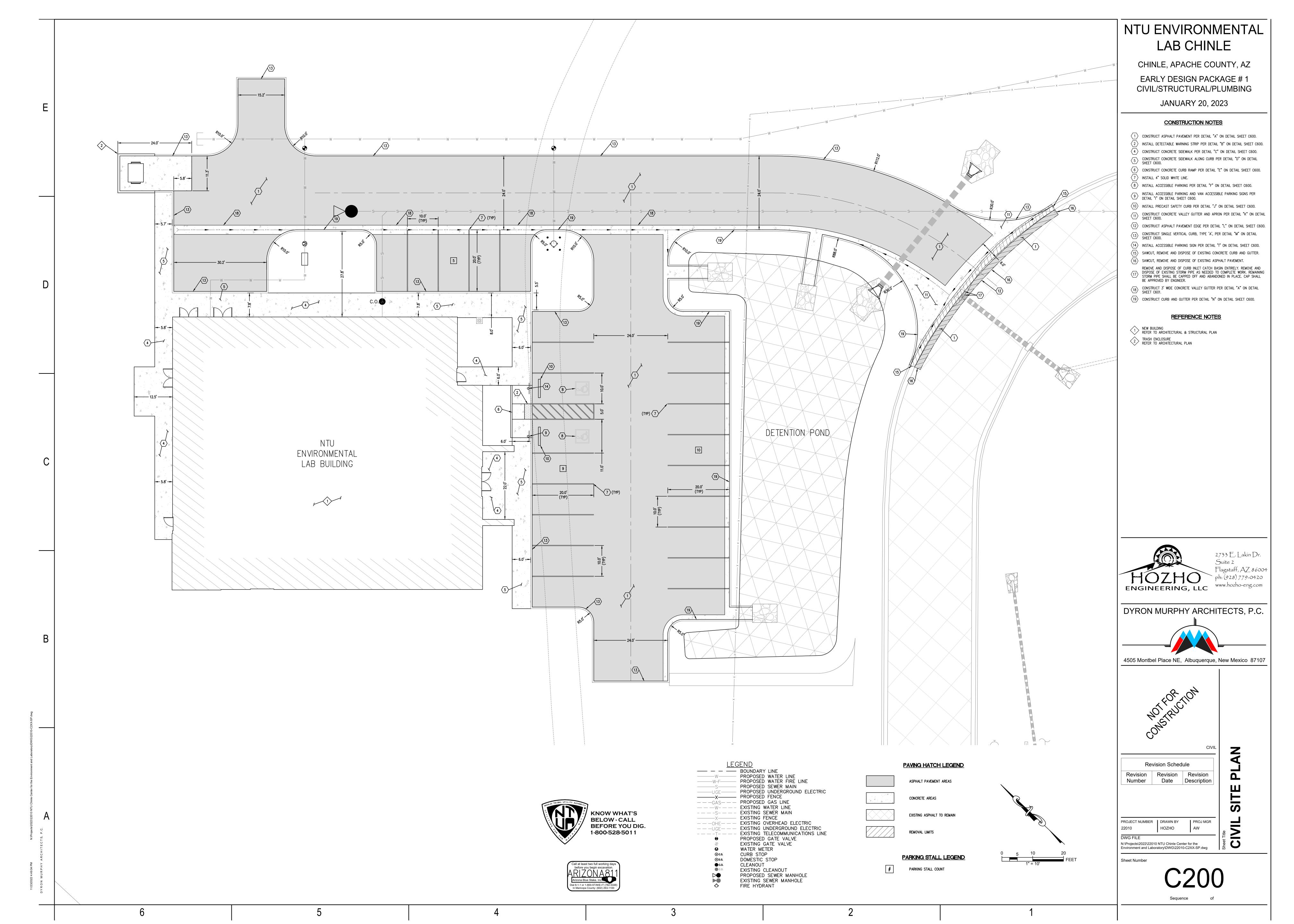


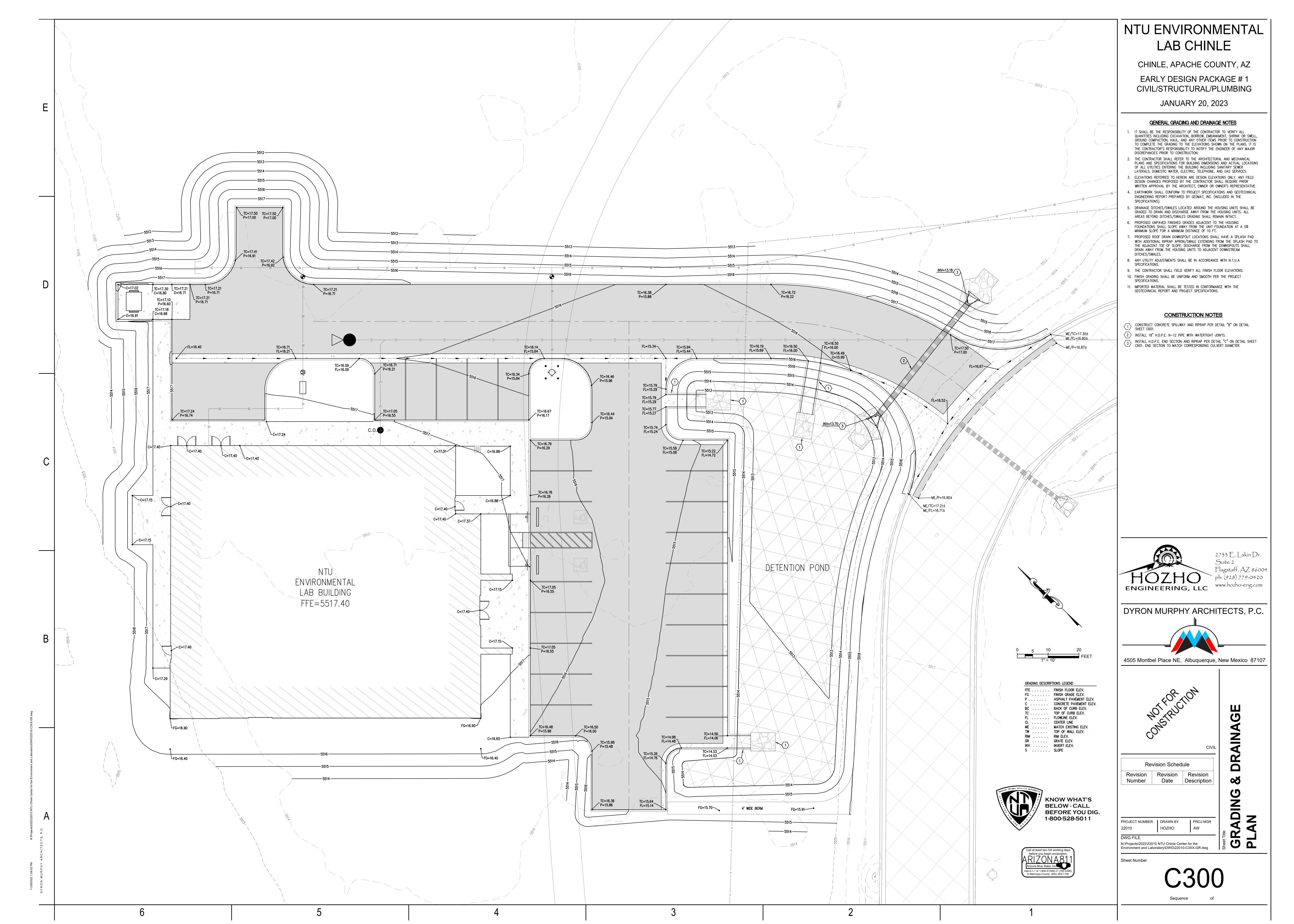


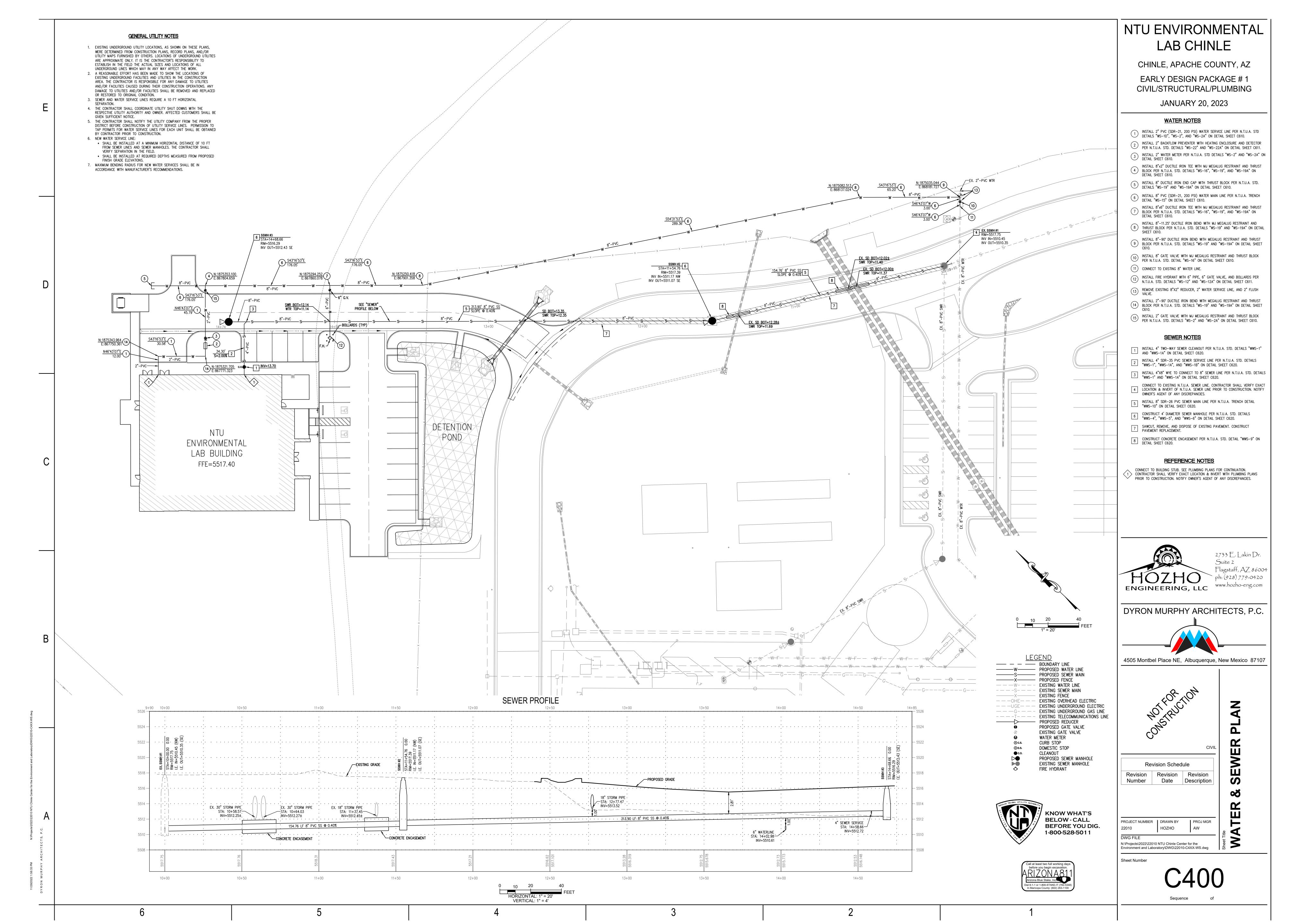
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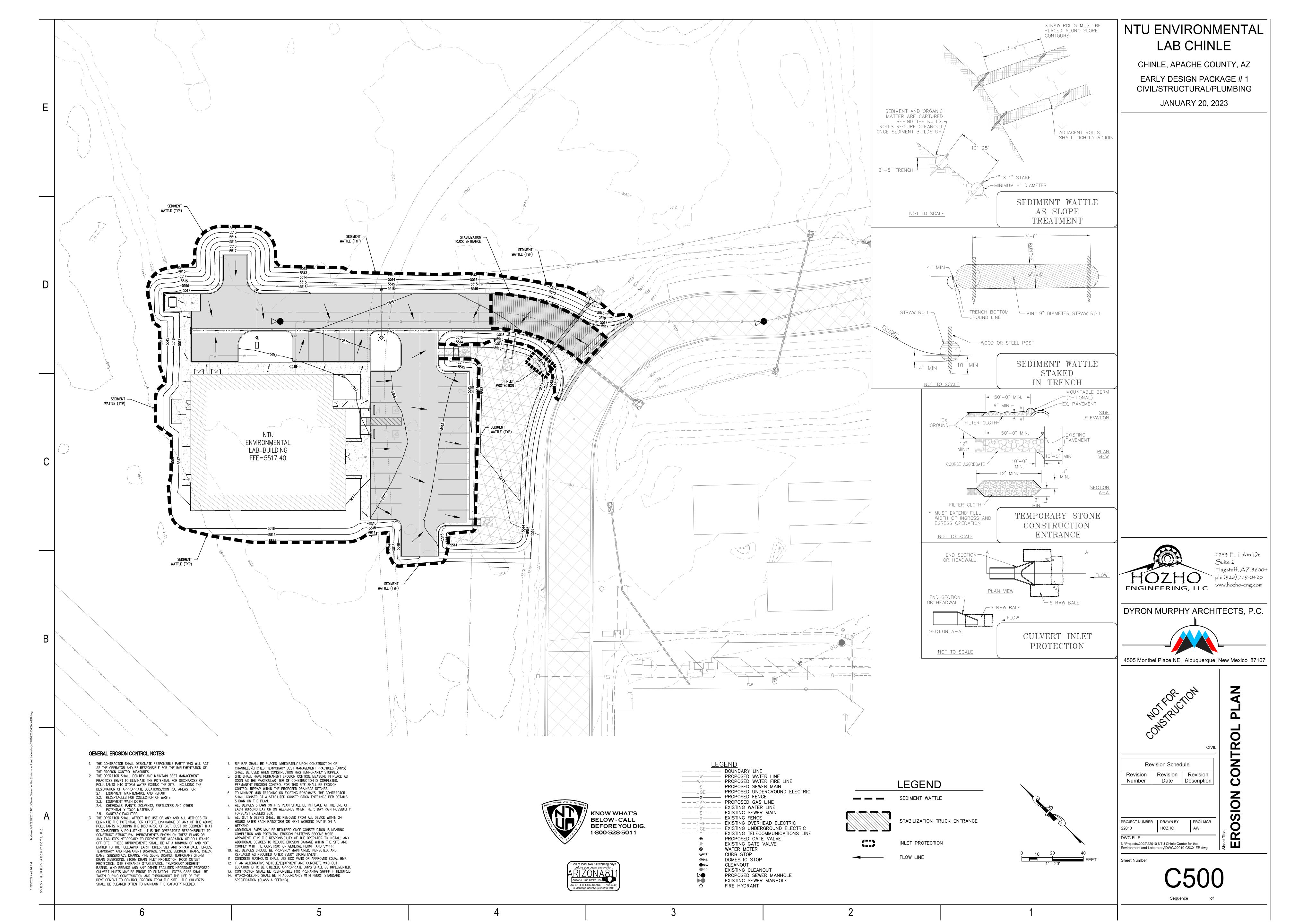
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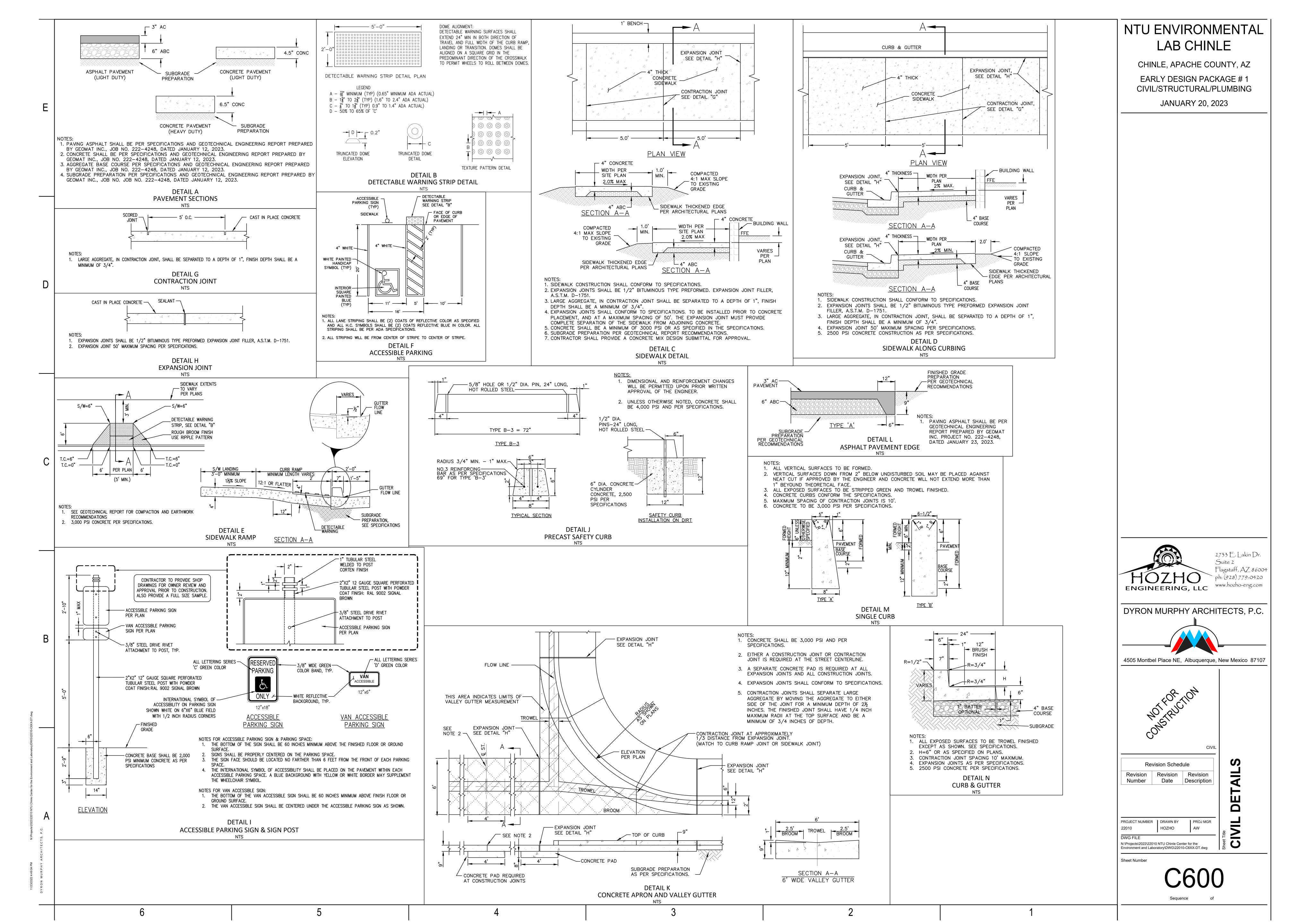
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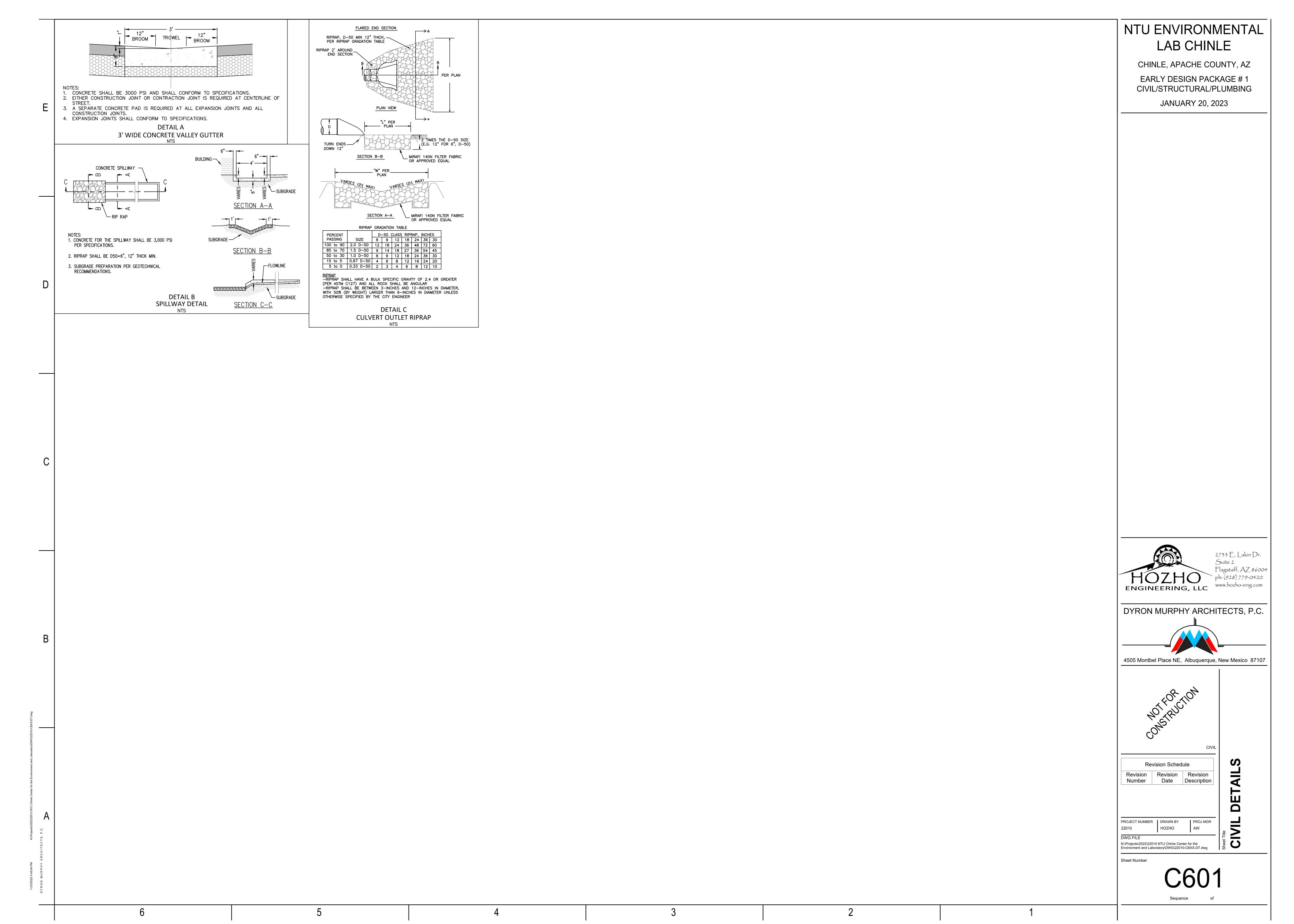


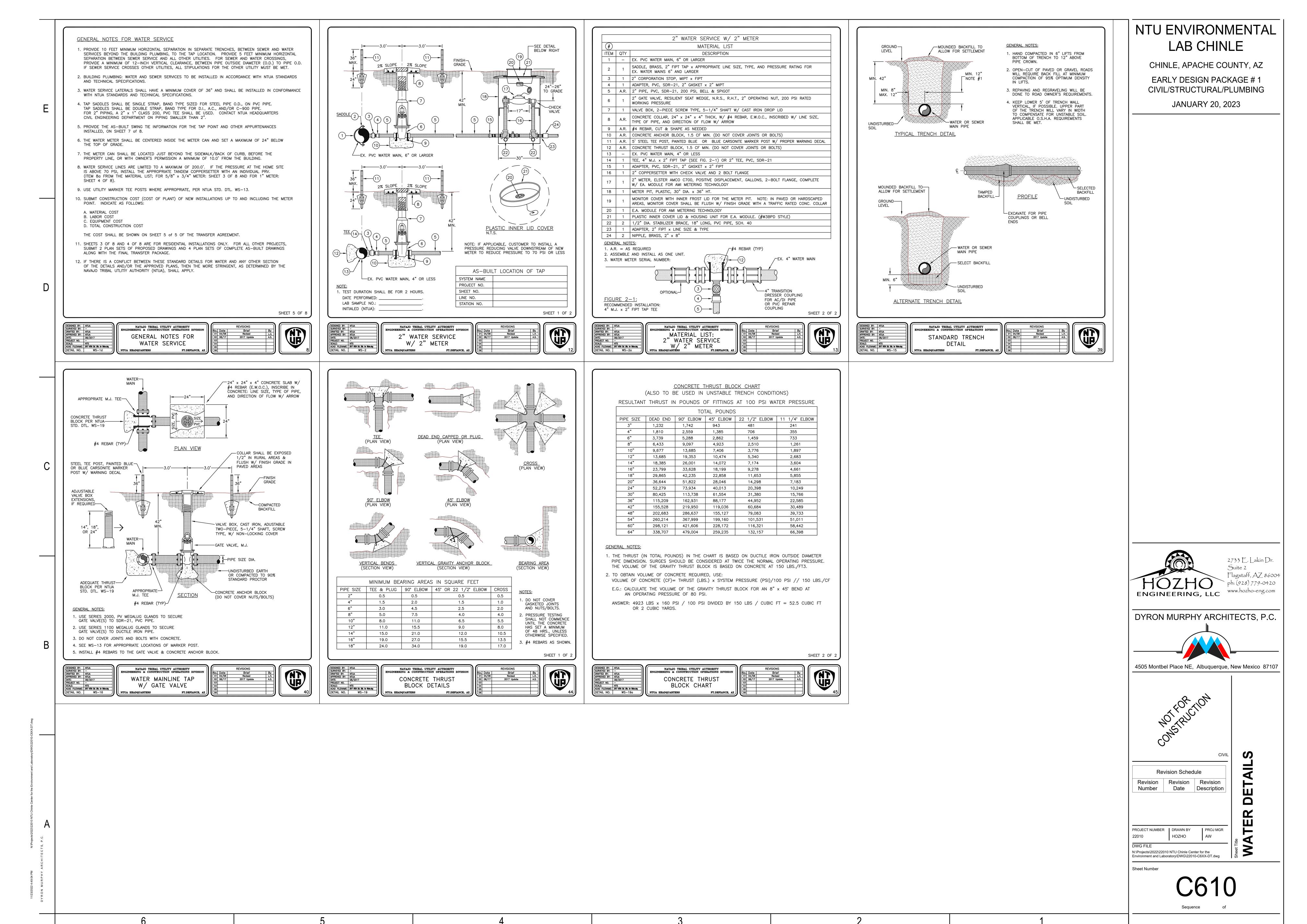


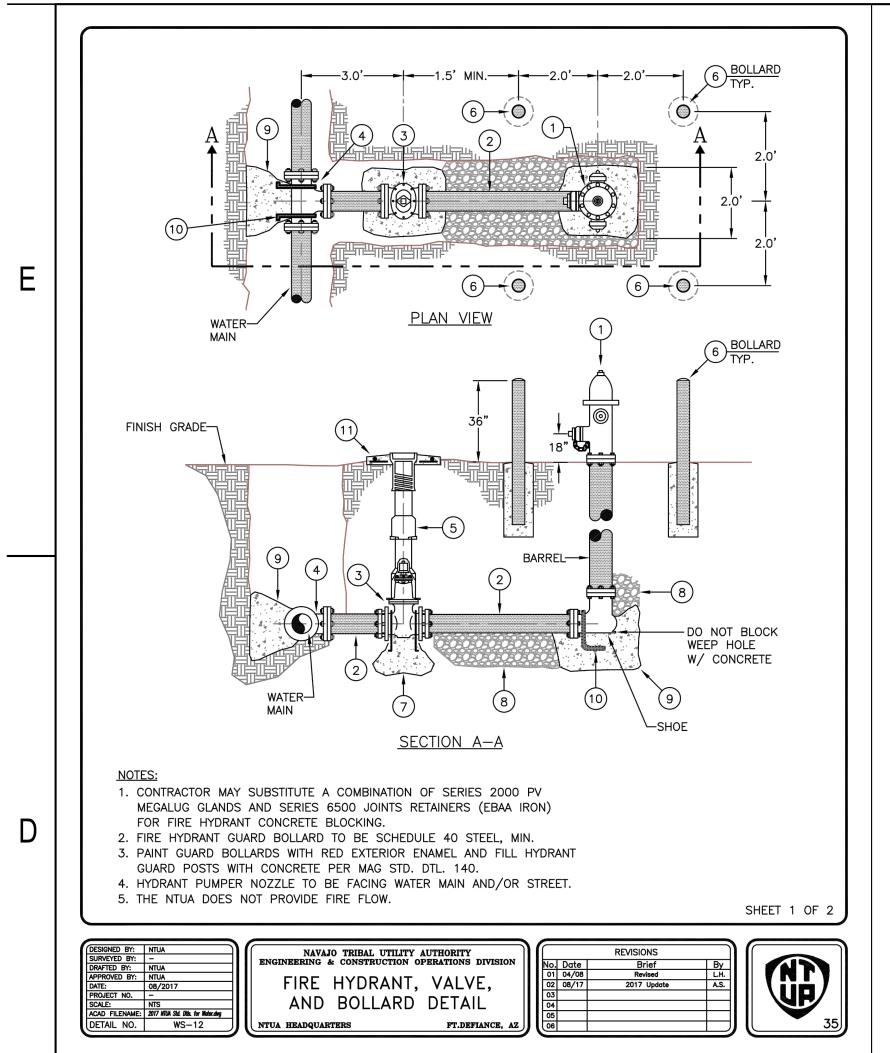


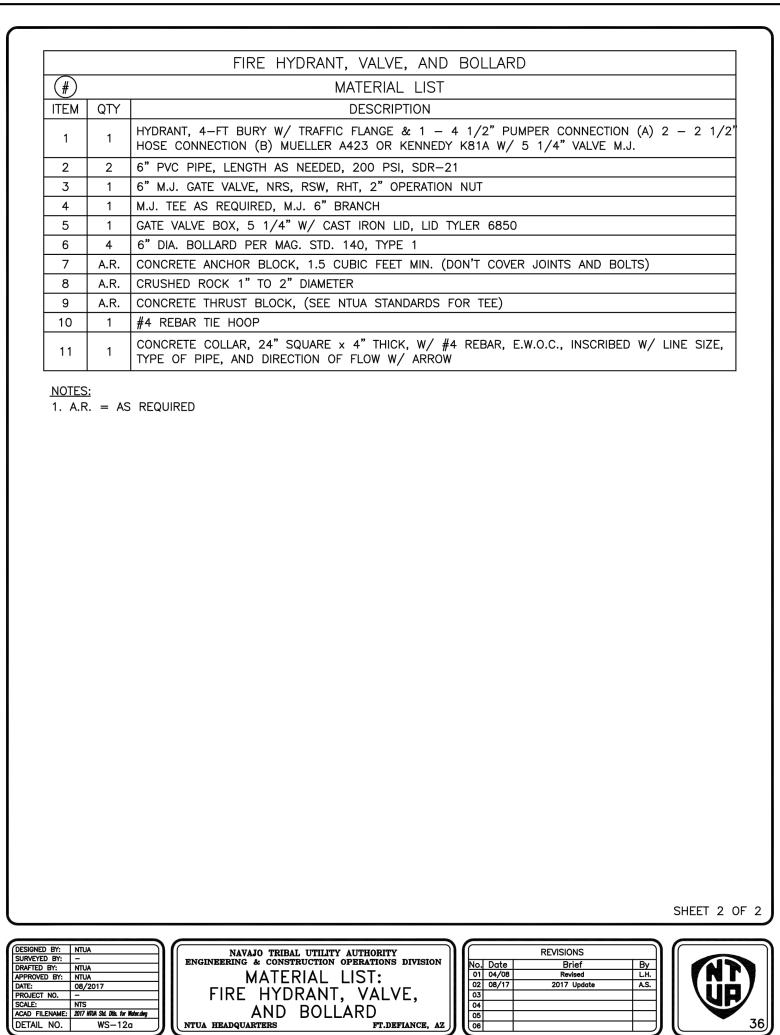


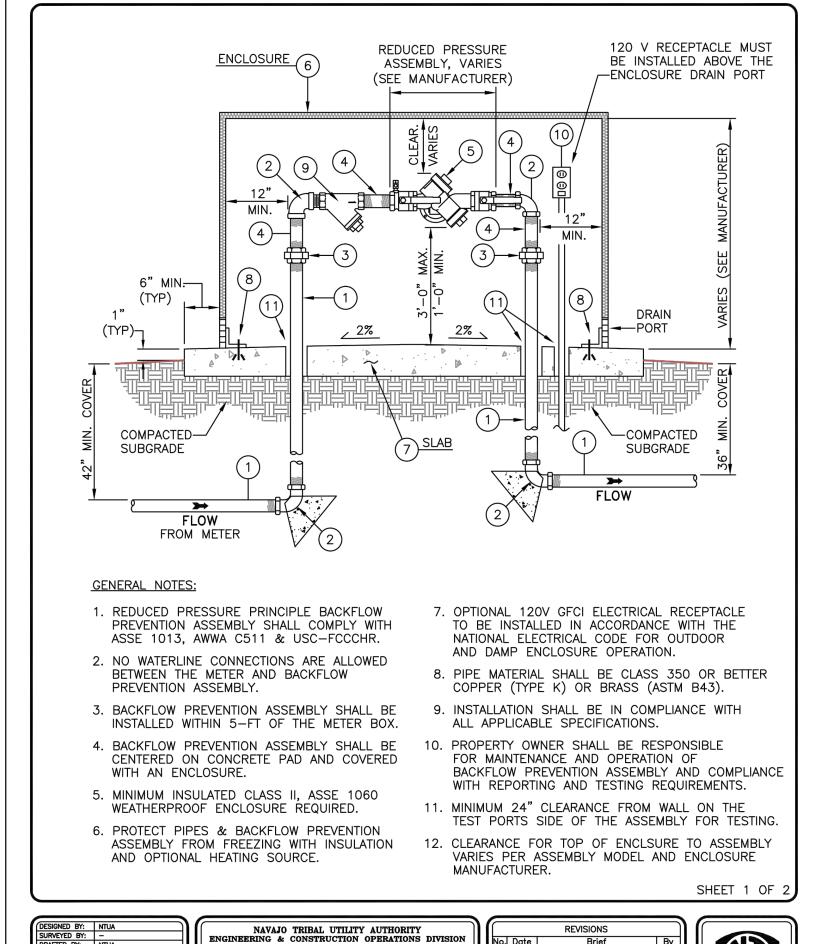








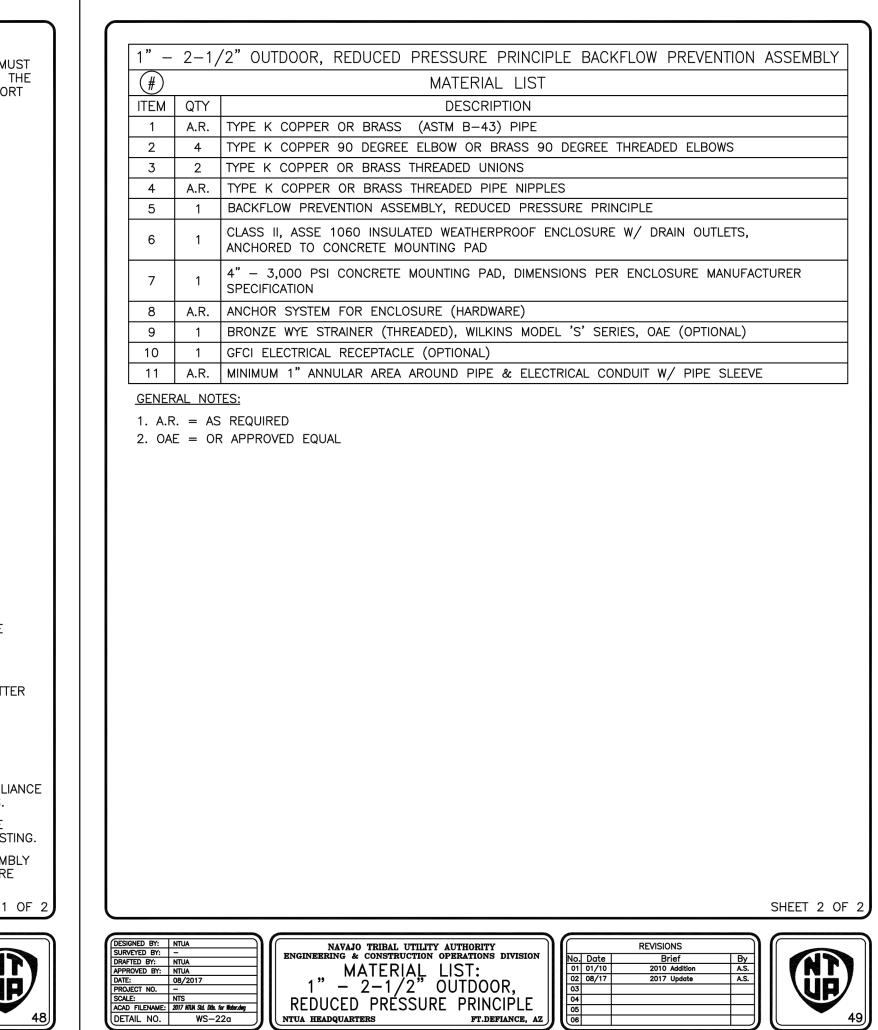




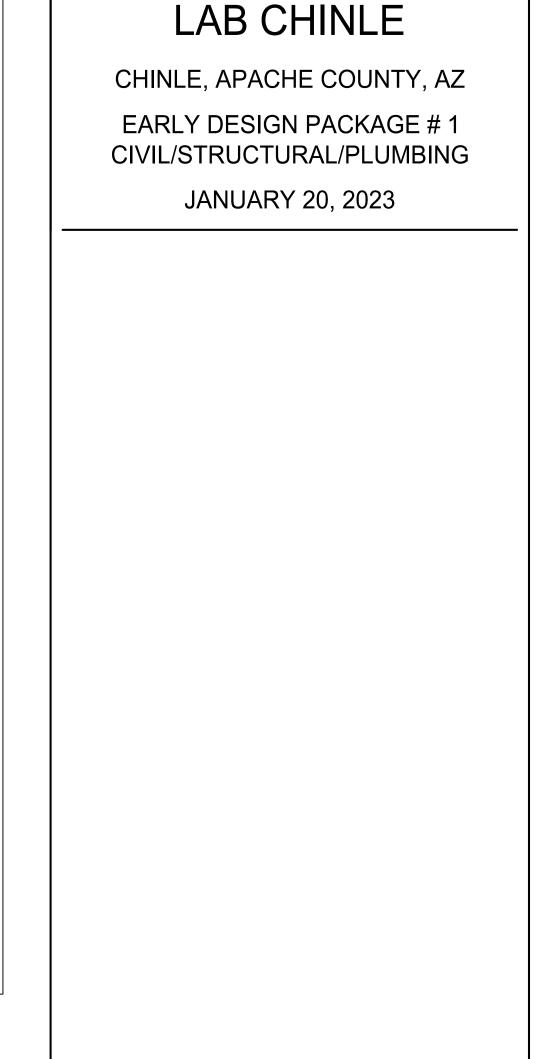
1" - 2-1/2" OUTDOOR,

REDUCED PRESSURE PRINCIPLE BACKFLOW PREVENTION ASSEMBLY NTUA HEADQUARTERS FT.DEFIANCE, AZ

ACAD FILENAME: 2017 NTUA Std. Dils. for Water.dwg
DETAIL NO. WS—22

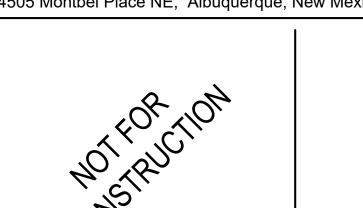


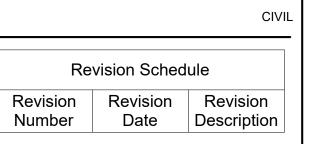
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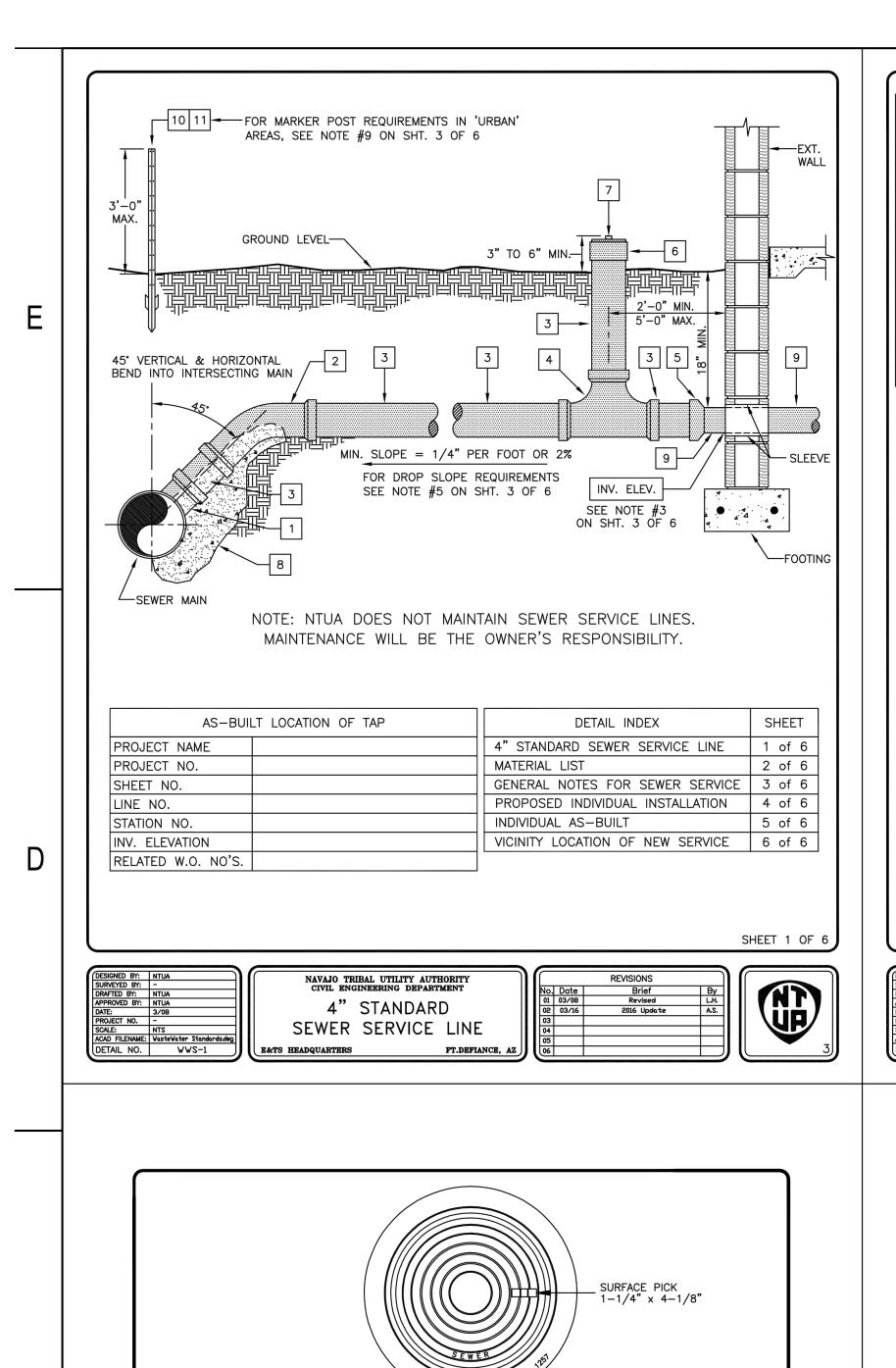
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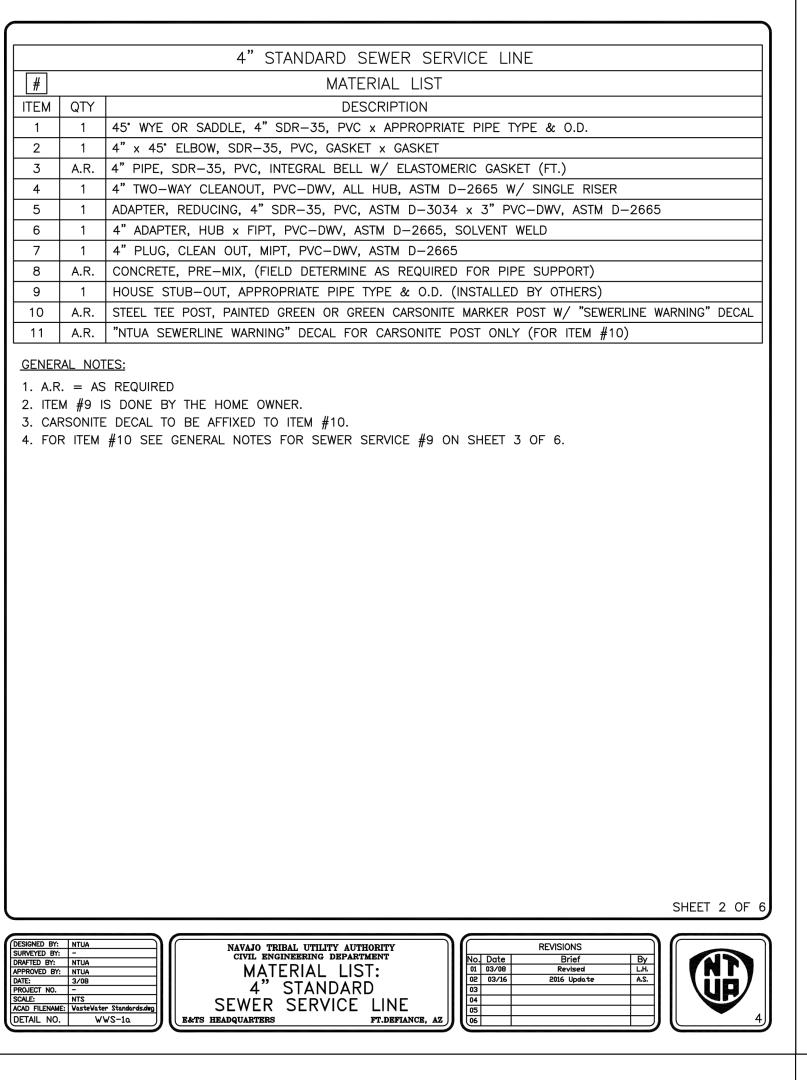


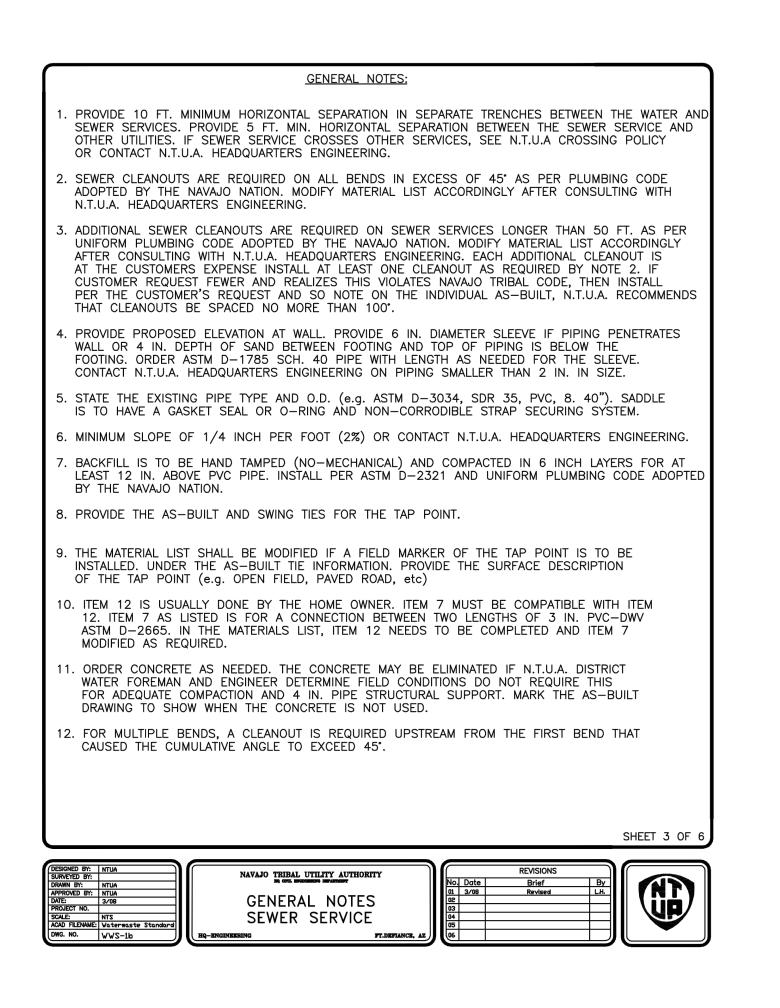


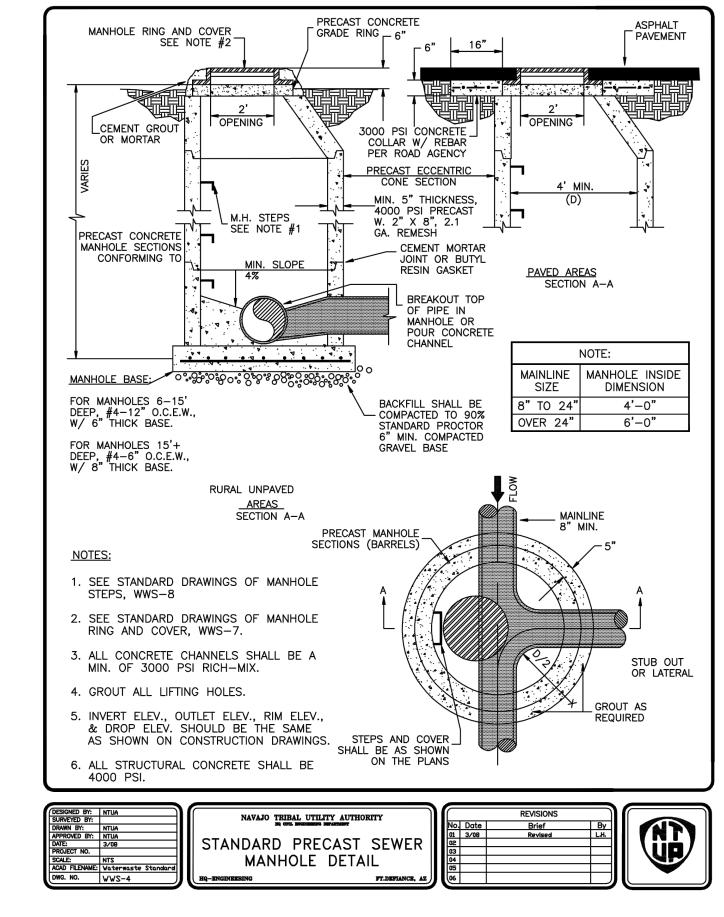


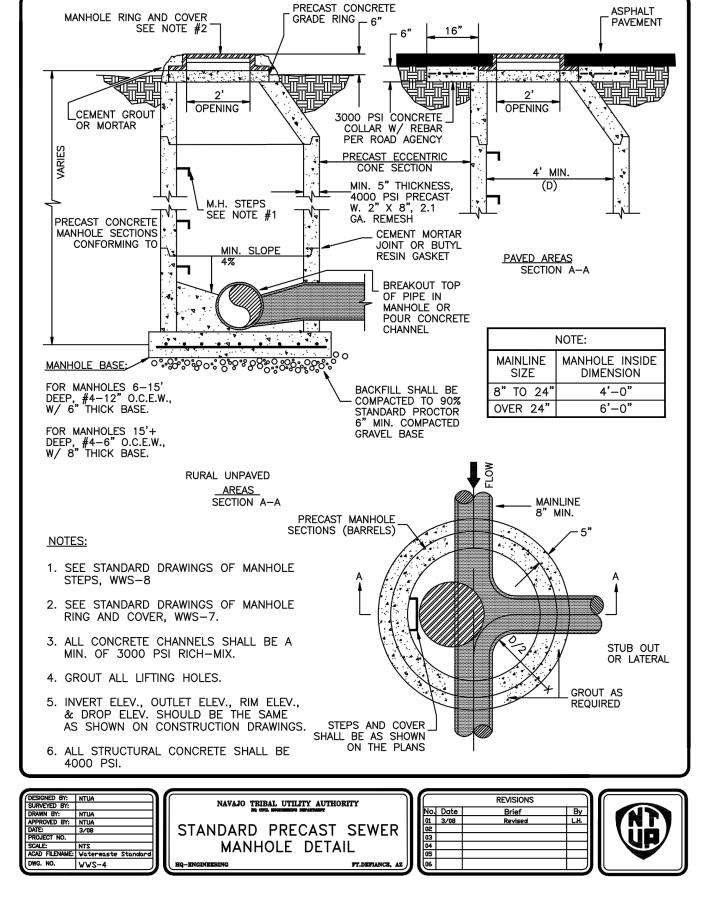
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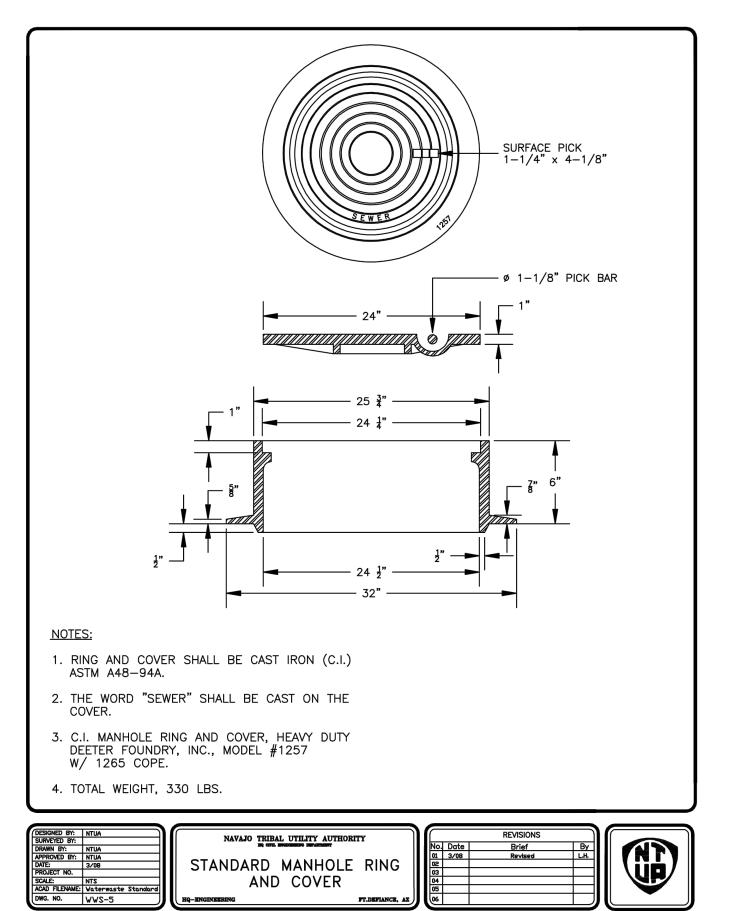


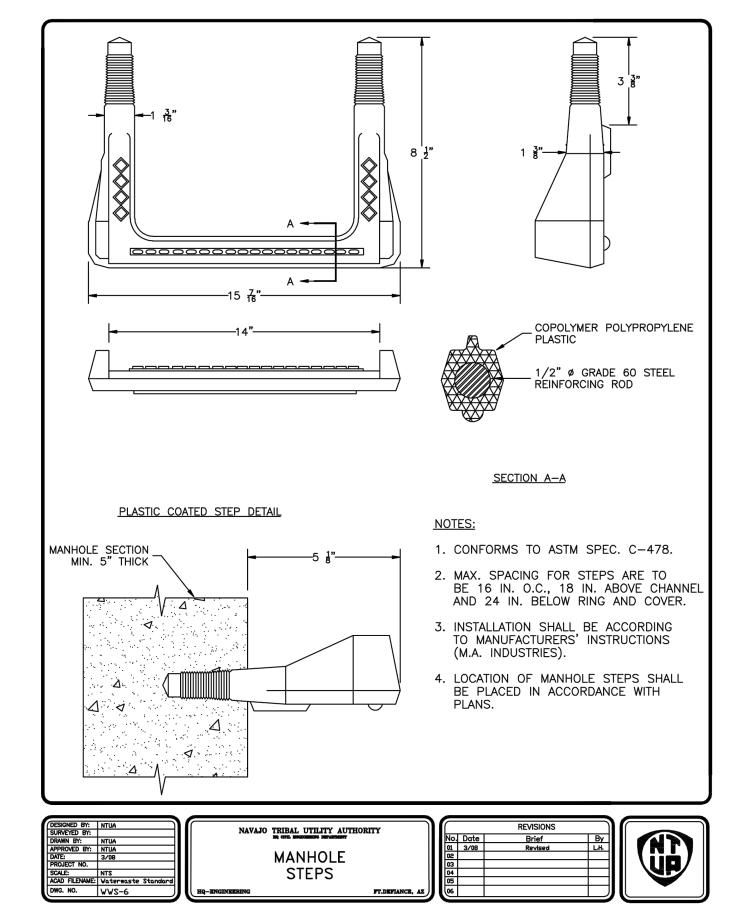


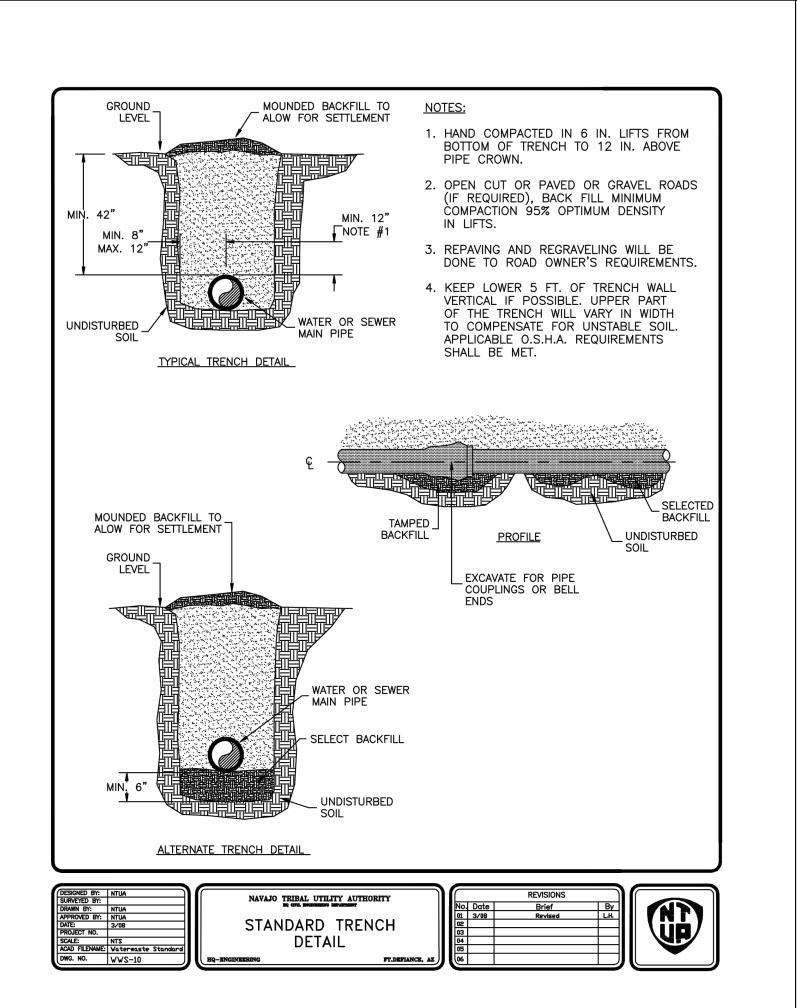


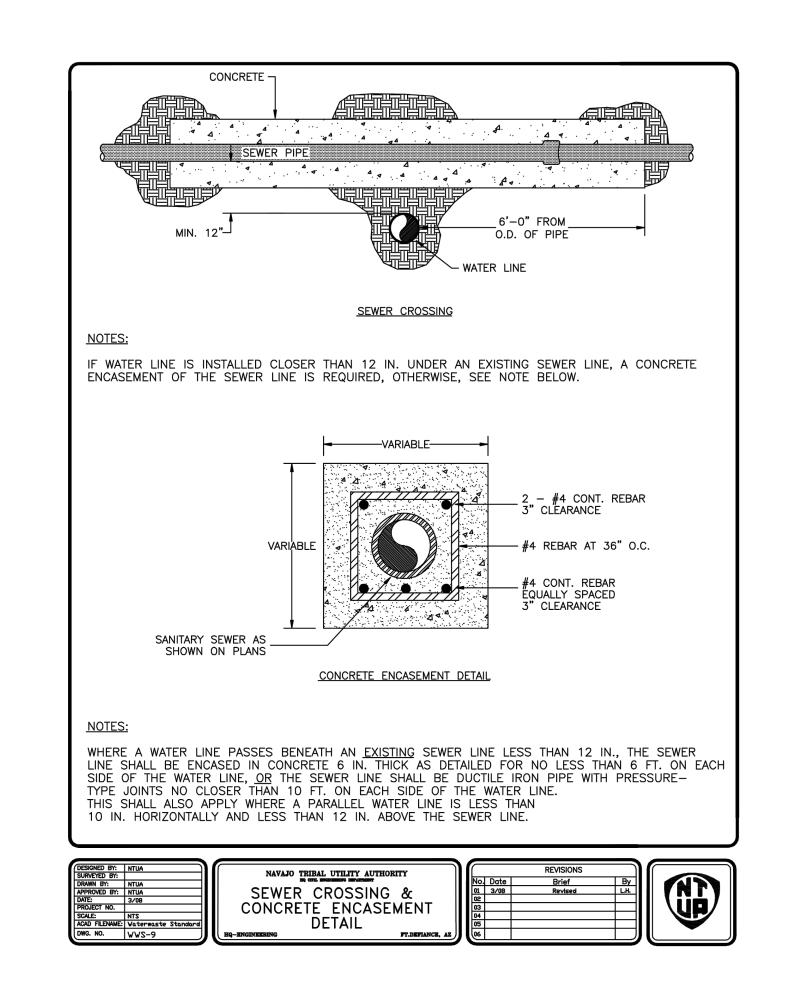


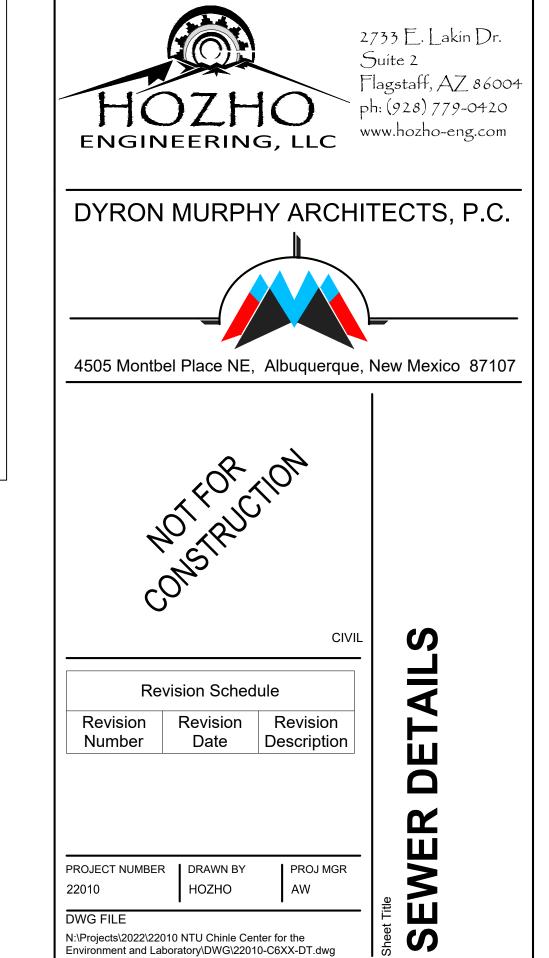












Sheet Number

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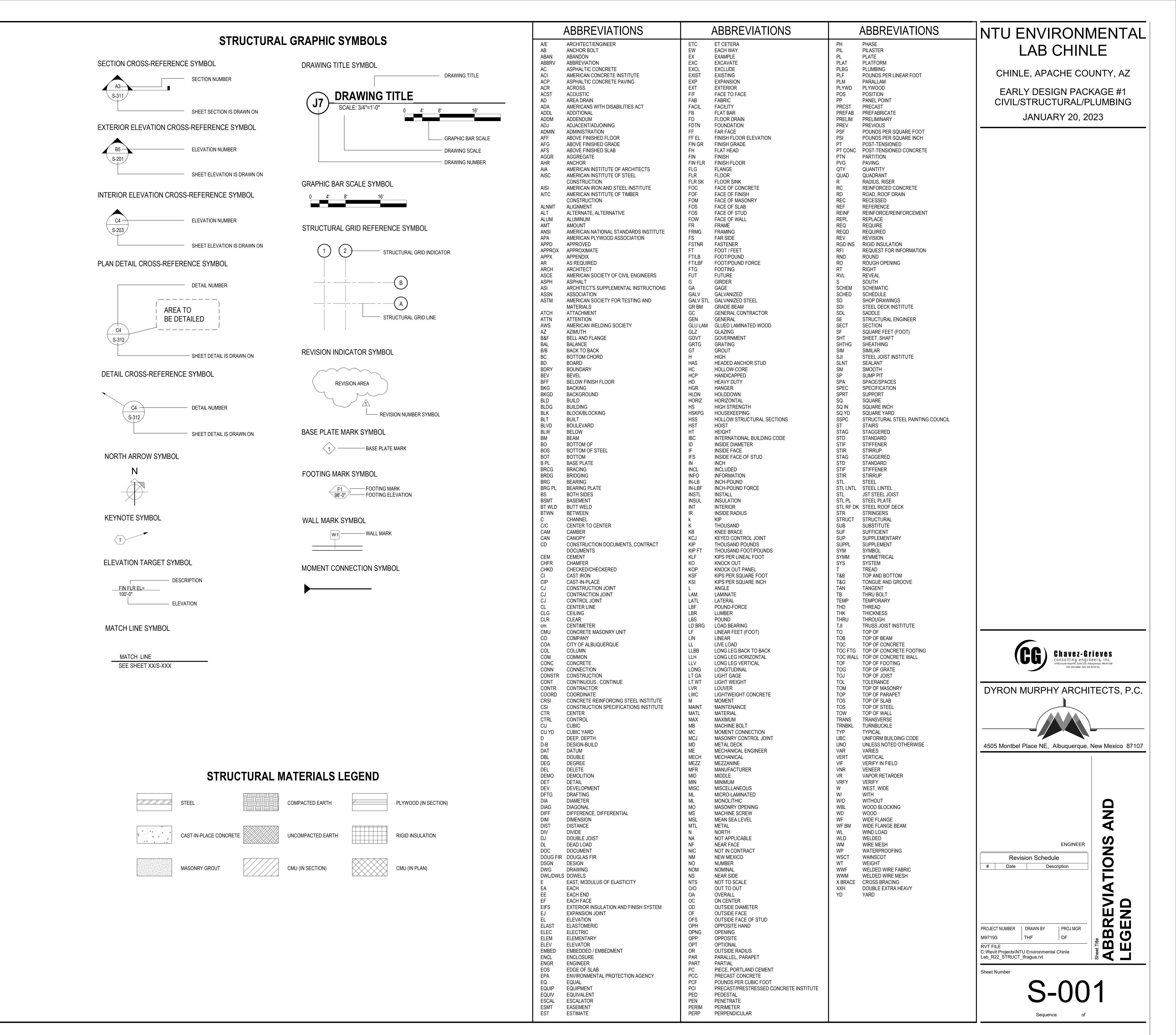
CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE # 1

CIVIL/STRUCTURAL/PLUMBING

JANUARY 20, 2023





GENERAL STRUCTURAL NOTES THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR. SECTION 1926. SUBPART R: A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE

NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED. REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STE ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE

MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORI INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS 1 LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

TYPICAL DETAIL SHEETS:

S-601 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE. THE S-500 AND S-700 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT.

THE TYPICAL DETAILS SHALL BE USED WHEN SPECIFIC DETAILS ARE NOT REFERENCED ON THE DRAWINGS. THE APPROPRIATE CONDITIONS FOR USE OF THE TYPICAL DETAILS ARE REFERENCED IN THEIR TITLES.

DRAWINGS:

DO NOT SCALE DRAWINGS.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

FAST-TRACK/PHASED CONSTRUCTION:

THE STRUCTURAL PORTION OF THIS PROJECT IS BEING DESIGNED, BID, PERMITTED, AND CONSTRUCTED PRIOR TO THE COMPLETION OF ARCHITECTURAL, ENGINEERING, AND OTHER DESIGN TEAM CONSTRUCTION DOCUMENTS. THE OWNER, ARCHITECT, AND CONTRACTOR SHALL BE AWARE THAT THIS ACCELERATED STRUCTURAL SCHEDULE CREATES INHERENT RISK OF FUTURE CHANGES DUE TO DESIGN COORDINATION WITH OTHER DISCIPLINES. WHILE EVERY EFFORT HAS BEEN MADE TO MINIMIZE THESE CHANGES, THE RISK OF ADDED COSTS DUE TO THESE CHANGES SHALL BE UNDERSTOOD AND ACCEPTED BY ALL PARTIES.

DRAWINGS THAT DO NOT HAVE AN ENGINEERING SEAL BY THE STRUCTURAL ENGINEER OF RECORD OR NOT LABELED AS CONSTRUCTION DRAWINGS ARE PRELIMINARY AND SUBJECT TO CHANGE. IF THESE DOCUMENTS AR BEING USED FOR PRICING. BIDDING. STEEL MILL ORDER. OR PREPARATION OF SHOP DRAWINGS. THE CONTRACTOR SHALL ANTICIPATE FUTURE DRAWING REVISIONS THAT MAY AFFECT THIS WORK OR INCREASE CONSTRUCTION COSTS. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY CHANGE ORDER COSTS INCURRED DUE TO THESE DRAWING REVISIONS, AND THE CONTRACTOR SHALL CONSIDER THESE ANTICIPATED COSTS IN ANY BIDS OR PRICE GUARANTEES TO THE OWNER.

USE THE MOST CURRENT SET OF DRAWINGS IN PREPARATION OF ALL SUBMITTALS. ALL SUBMITTALS SHALL LIST THE DATE OF THE DRAWINGS USED TO PREPARE THE SUBMITTAL. SUBMITTALS PREPARED FROM OUTDATED DRAWINGS MAY BE REJECTED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING THE LATEST SET OF CONSTRUCTION DRAWINGS AND DISTRIBUTING TO THE APPROPRIATE PARTIES.

CAST-IN-PLACE CONCRETE:

ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.

ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.

NORMALWEIGHT CONCRETE:

،. F'C = 4500 PSI @ 28 DAYS – ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC... EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE

B. F'C = 3000 PSI @ 28 DAYS - ALL INTERIOR CONCRETE (I.E. FOOTINGS, PEDESTALS, TIE BEAMS, GRADE

BEAMS. INTERIOR RETAINING WALLS, ETC.). C. F'C = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.

D. F'C = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE. E. F'C = 4000 PSI @ 28 DAYS – ALL CAST-IN-PLACE CONCRETE COLUMNS AND ELEVATED BEAMS.

F'C = 5000 PSI @ 28 DAYS - ALL ELEVATED CAST-IN-PLACE SLABS.

G. F'C = 4000 PSI @ 28 DAYS - ALL SLABS ON GRADE AND ELEVATED SLABS TO RECEIVE POLISHED CONCRETE

CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 19.3.2.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 19.3.1.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE. AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING

COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1.

HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY-MIX CONCRETE, LOWER THE TEMPERATURE OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RE-TEMPERING WITH WATER IS NOT ALLOWED. COMPLY WITH ACI 305R.

SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.

ITHE CONTRACTOR SHALL NOT CAST FOUNDATIONS AGAINST EXCAVATED VERTICAL SIDE SURFACES.1

ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION AS DICTATED BY THE [THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO: RESPECTIVE PRODUCT'S ICC-ES EVALUATION SERVICE REPORT

A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SLOPE WITHOUT SOIL

SLOUGHAGE. . THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE

SPECIFIED FOOTING WIDTH. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVI

INCHES VERTICAL . IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION.

THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT | OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO COMMENCEMENT OF

EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:

12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT 18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT

20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE II OR TYPE V.

SEE SHEET S-701 FOR TYPICAL CONCRETE DETAILS.

GENERAL STRUCTURAL NOTES

PRIOR TO PLACEMENT OF THE CONCRETE SLABS TO RECEIVE A POLISHED FINISH, THE CONTRACTOR, ARCHITECT, ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION AND ENGINEER SHALL MEET TO DISCUSS THE PROCESS AND CLARIFY THE EXPECTATIONS FOR THE POLISHED FOR STRUCTURAL STEEL BUILDINGS".

STRUCTURAL AND MISCELLANEOUS STEEL:

ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.

ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM

TO ASTM A36 UNLESS NOTED OTHERWISE. ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE C, FY = 50 KSI.

ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B,

ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.

BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS. AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS. THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT

ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.

ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.

ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL

ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.

HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET [S-521] FOR TYPICAL CONNECTION DETAIL. SEE SHEET [S-741] FOR TYPICAL STEEL DETAILS.

STEEL DECK:

WELDING CODE.

FY = 50 KSI.

GENERAL STRUCTURAL NOTES

EACH CONCRETE MIX INGREDIENT OF THE POLISHED FLOORS SHALL BE FROM THE SAME SOURCE, FROM THE

INCLUSION OF ADMIXTURES, PLASTICIZERS, SLAG, FLY ASH, OR OTHER PRODUCTS REPLACING PORTIONS OF THE

PORTLAND CEMENT IN THE CONCRETE MIX SHALL NOT BE USED UNLESS APPROVED BY THE ENGINEER OF

RECORD. ANY APPROVED ADMIXTURES SHALL NOT BE CALCIUM CHLORIDE BASED, AND THEIR VOLUME SHALL

POLISHED CONCRETE FLOORS SHALL BE CURED WITH A MOISTURE RETAINING COVER IN ACCORDANCE WITH

SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR THE FLOOR SHINE AND AGGREGATE EXPOSURE

ACI308R-01. THE SLAB SHALL REMAIN CONTINUOUSLY COVERED DURING THIS TIME TO MAINTAIN THE MOISTURE

FLOOR FLATNESS (FF NUMBER) OF POLISHED CONCRETE FLOORS SHALL HAVE A MINIMUM OVERALL VALUE OF 50.

FLOOR LEVELNESS (FL NUMBER) OF POLISHED CONCRETE FLOORS SHALL HAVE A MINIMUM OVERALL VALUE OF

CONTRACTOR SHALL CREATE A MOCKUP OF THE POLISHED FLOOR PRIOR TO POLISHING THE BUILDING SLAB. THE

ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED

ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS

TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPLICE SCHEDULE ON SHEET

ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR

HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR

A. CONCRETE FOR FOUNDATIONS CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"

FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1

INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES, RECESSES ARE TO BE CAREFULLY

REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE

BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99.

CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF

THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE

ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED

BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS

MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR

SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE

ALL ADHESIVE (EPOXY OR ACRYLIC) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO CONCRETE SHALL BE

HILTI HIT-RE 500 V3 OR HIT-HY 200 EPOXY ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE

ALL ADHESIVE (EPOXY OR ACRYLIC) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED

MASONRY SHALL BE HILTI HIT HY 270 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE

ALL ADHESIVE (EPOXY OR ACRYLIC) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY

AND/OR BRICK SHALL BE HILTI HIT HY 270 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE

ALL POST INSTALLED MECHANICAL ANCHORS INTO GROUT FILLED MASONRY SHALL BE HILTI KWIK BOLT TZ

ALL POST INSTALLED MECHANICAL SCREW ANCHORS INTO CONCRETE SHALL BE HILTI KWIK HUS EZ (KH-EZ)

(KH-EZ) SCREW ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION

SCREW ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT

SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED

THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE

INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE

INSTALLATION TRAINING, UNLESS ALL PERSONNEL INSTALLING ANCHORS ARE CERTIFIED IN ACCORDANCE WITH

ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT APPROVED BY THE ENGINEER

CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE

BY THE BUILDING OFFICIAL. INSTALLATION SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS

CERTIFIED THROUGH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. PROOF

INSTALLATION, AND INSPECTION REPORTS SHALL BE PROVIDED TO THE ENGINEER OF RECORD AND THE BUILDING

LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED

ALL POST INSTALLED MECHANICAL SCREW ANCHORS INTO GROUT FILLED MASONRY SHALL BE HILTI KWIK HUS-EZ

EXPANSION ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION

INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK BOLT TZ EXPANSION ANCHOR.

DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:

C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:

E. STRUCTURAL CONCRETE SLABS ON METAL DECK: 1" FROM TOP OF SLAB

2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER): 3/4"

REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE

SAME RESPECTIVE BATCH, AND EACH DELIVERED TO THE CONCRETE PRODUCER IN ONE DELIVERY.

CONCRETE FLOORS. THE CONTRACTOR IS RESPONSIBLE FOR SCHEDULING THIS MEETING.

IN THE SLAB. CURING COMPOUNDS ARE NOT ALLOWED IN POLISHED CONCRETE FLOORS.

AND A MINIMUM LOCAL VALUE OF 35 WHEN TESTED IN ACCORDANCE WITH ASTM E1155

30, AND A MINIMUM LOCAL VALUE OF 20 WHEN TESTED IN ACCORDANCE WITH ASTM E1155.

AGGREGATE IN POLISHED CONCRETE SLABS SHALL BE NON-POROUS.

NOT EXCEED 20% OF THE PORTLAND CEMENT VOLUME.

MOCKUP SHALL BE AT LEAST 4 FEET SQUARE.

FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.

BARS LARGER THAN NO. 5: 2"

2. BARS NO. 5 OR SMALLER: 1 1/2"

1. COLUMNS, GIRDERS AND BEAMS: 1 1/2"

D. CONCRETE SLAB-ON-GRADE: 1 1/2" FROM TOP OF SLAB

REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.

MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

POLISHED CONCRETE FLOORS:

REQUIREMENTS.

REINFORCING STEEL:

REINFORCEMENT (ACI 315-99).

DIAMETERS (18" MINIMUM).

FILLED AND POINTED WITH MORTAR.

CONCRETE IN CONTACT WITH GRADE.

POST INSTALLED ANCHORS:

ALTERNATE ANCHORING SYSTEM.

INSTRUCTIONS.

CONNECTION.

IN THESE NOTES.

OF RECORD.

STRUCTURAL PLANS.

ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.

SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.

PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.

ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.

ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.

COLD-FORMED METAL FRAMING (43 MILS OR HEAVIER):

ALL COLD-FORMED METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF AISI STANDARD S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

WALLS SHALL BE PROVIDED WITH MANUFACTURER'S STANDARD BRIDGING: (EITHER WELDED 2 1/2" x 43 MILS STUD OR CLIPPED COLD-ROLLED CHANNEL 1 1/2" x 54 MILS). PROVIDE BRIDGING AT 4'-0" ON CENTER MAXIMUM FOR LOAD BEARING WALLS AND EXTERIOR WALLS.

PROVIDE ALL MISCELLANEOUS ACCESSORIES AND FOLLOW ERECTION PROCEDURES AS PER MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS UNLESS NOTED OTHERWISE.

COLD-FORMED METAL FRAMING SHALL MEET THE MINIMUM PROPERTIES AS SHOWN IN THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) SPECIFICATIONS.

SPACED AT 4'-0" ON CENTER UNLESS SHOWN OTHERWISE ON PLANS.

ALL TRACK SHALL BE ANCHORED TO CONCRETE WITH 1/2" DIAMETER x 3 1/2" EMBED EXPANSION ANCHORS

SECURE STUDS TO TOP AND BOTTOM TRACKS BY WELDING AT BOTH INSIDE AND OUTSIDE FLANGES OR WITH A MINIMUM OF 1-#10 SELF-DRILLING SCREW PER LOCATION UNLESS NOTED OTHERWISE.

ALL COMPONENTS OF BUILT-UP STUD SECTIONS, INCLUDING COLUMNS, JAMBS, HEADERS, ETC. SHALL BE WELDED TOGETHER UTILIZING 1/8" FILLET WELDS, 1" LONG AT 12" OC OR MECHANICALLY FASTENED WITH #10 SELF-

DRILLING SCREWS AT 12" OC ALONG THE FULL LENGTH OF EACH FLANGE TO FLANGE CONNECTION. $\,$ FASTEN WELD CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA TO PROVIDE AN ALLOWABLE LOAD OF [700# MINIMUM] IN THE HORIZONTAL DIRECTION AND [700# MINIMUM] IN THE

VERTICAL DIRECTION. SEE SHEET [S-731] FOR TYPICAL WELD CLIP DETAIL. FASTEN SLIDE CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA TO PROVIDE AN ALLOWABLE LOAD OF [700# MINIMUM] IN THE HORIZONTAL DIRECTION. SEE SHEET [S-731] FOR TYPICAL SLIDE CLIP DETAIL.

SEE SHEET S-731 FOR TYPICAL COLD-FORMED DETAILS.

NTU ENVIRONMENTAL LAB CHINLE

CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING **JANUARY 20, 2023**





4505 Montbel Place NE. Albuquerque, New Mexico 87107

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Revision Schedule Description

Date

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CELLS CONTAINING REBAR SHALL BE GROUTED SOLID FROM THE BOTTOM TO THE TOP OF THE WALL IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.

ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID UP TO GRADE.

CELLS CONTAINING EXPANSION ANCHORS SHALL BE GROUTED SOLID.

ALL VERTICAL REBAR SHALL BE IN PLACE AND SECURED WITH REBAR POSITIONERS PRIOR TO GROUTING.

COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

MASONRY FACE NOT EXPOSED TO EARTH OR WEATHER: 1 1/2" . MASONRY FACE EXPOSED TO EARTH OR WEATHER: 1. BARS LARGER THAN NO. 5: 2"

BARS NO. 5 OR SMALLER:

UNLESS OTHERWISE NOTED MASONRY CELLS SHALL BE GROUTED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (MAXIMUM 5 FOOT GROUT LIFTS).

LAP REBAR 48 BAR DIAMETERS (12" MINIMUM) UNLESS NOTED OTHERWISE.

WHERE REBAR LAP SPLICES EXCEED 5 FT GROUT LIFTS. 8 FT GROUT LIFTS MAY BE USED WITH CLEANOUTS PROVIDED AT THE BOTTOM OF EACH VERTICALLY REINFORCED CELL. SOLID GROUTED WALLS SHALL HAVE CLEANOUTS AT 32" ON CENTER MAXIMUM.

ALL HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND A LAP AS NOTED ABOVE. VERTICAL STEEL SHALL CONTINUE THROUGH BOND BEAMS.

PROVIDE STANDARD LADDER TYPE JOINT REINFORCING AT 16" ON CENTER (ALTERNATE COURSES) UNLESS NOTED OTHERWISE IN THE PROJECT DOCUMENTS. USE PREFABRICATED CORNERS AND TEES AT ALL WALL CORNERS AND INTERSECTIONS RESPECTIVELY.

PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED THERWISE. SEE SHEET [S-521] FOR TYPICAL CONNECTION DETAIL

SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR INFORMATION REGARDING MASONRY COLORS, FINISHES, BOND, COURSING, ETC. AT ALL EXPOSED MASONRY WALLS.

ALL MASONRY WALL CONFIGURATIONS INCLUDING WALL OPENINGS SHALL BE COORDINATED WITH CIVIL,

MECHANICAL, PLUMBING, ELECTRICAL AND DRAWINGS FROM ALL OTHER DISCIPLINES.

EXPOSED MASONRY SITE WALLS AND RETAINING WALLS GREATER THAN 16 FEET IN LENGTH SHALL HAVE MASONRY CONTROL JOINTS INSTALLED AT THE FOLLOWING MINIMUM SPACING:

12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT 18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT

20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

SEE SHEET [S-721] FOR TYPICAL MASONRY DETAILS

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16") WIRE OR APPROVED EQUAL. PROVIDE HOHMANN BARNARD 265 LADDER ADJUSTABLE INTEGRAL ANCHOR TIES OR APPROVED EQUAL.

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A WO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16") WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16") WIRE. PROVIDE HOHMANN BARNARD HB-213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT SELF-DRILLING SCREWS.

SEE TYPICAL DETAILS ON SHEET [S-711, S-721, AND S-731] FOR VENEER TIE SPACING.

PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.

COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.

SEISMIC BRACING OF NON-STRUCTURAL COMPONENTS:

SEISMIC BRACING AND RESTRAINTS FOR MECHANICAL/ELECTRICAL EQUIPMENT AND SYSTEMS SHALL BE PROVIDED BY THE CONTRACTOR PER THE 2018 INTERNATIONAL BUILDING CODE (2018 IBC) AND THE ASCE 7-16, MINIMUM DESIGN LOADS FOR BUILDINGS AND STRUCTURES.

SEE THESE GENERAL STRUCTURAL NOTES FOR THE SITE-SPECIFIC SEISMIC DESIGN CRITERIA.

HE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE SEISMIC BRACING AND RESTRAINTS. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY BRACING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS

FEMPORARY SHORING OF EXCAVATIONS:

THE TEMPORARY SHORING OF EXCAVATIONS SHALL BE [AN/A] [H-PILE/LAGGING SYSTEM INSTALLED IN PRE-DRILLED CONCRETE PIERS] [SOIL NAIL/SHOTCRETE SYSTEM] [SHEET PILING] OR APPROVED EQUAL.

THE SHORING SHALL NOT BE DRIVEN OR INSTALLED IN ANY MANNER THAT COULD POTENTIALLY DAMAGE EXISTING STRUCTURES OR CAUSE HUMAN DISCOMFORT.

THE CONTRACTOR SHALL LOCATE ALL EXISTING UNDERGROUND UTILITIES PRIOR TO INSTALLING SHORING. PROVISIONS SHALL BE MADE TO AVOID EXISTING UTILITIES.

THE SHORING AS SHOWN ON THE PLANS IS FOR GRAPHICAL REPRESENTATION ONLY. THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND CONFIGURATION OF THE SHORING.

ANY SHORING THAT REMAINS IN PLACE SHALL NOT HAMPER FUTURE CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE TEMPORARY SHORING. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY SHORING

HE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS

SPECIAL INSPECTION:

HE OWNER SHALL PROVIDE FOR SERVICES OF A CERTIFIED INSPECTOR (APPROVED BY THE BUILDING OFFICIAL OR THE ENGINEER OF RECORD) IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE FOR THE SPECIAL INSPECTION ITEMS NOTED ON SHEET S-003.

DEFERRED SUBMITTALS:

THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION FOR PERMIT AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE

- **CURTAINWALL SYSTEMS** INTERIOR COLD FORMED METAL FRAMING
- INTERIOR AND EXTERIOR SIGNAGE
- MECHANICAL EQUIPMENT CURBS AND ATTACHMENT TO STRUCTURE

GENERAL FOUNDATION NOTES

FOUNDATION NOTES GENERAL:

THE FOUNDATION NOTES INCLUDED HEREIN ARE A SUMMARY OF THE REQUIREMENTS OF THE PROJECT GEOTECHNICAL REPORT. THESE NOTES ARE PROVIDED FOR THE CONVENIENCE OF THE CONTRACTOR ONLY.

F THERE ARE DISCREPANCIES BETWEEN THE FOUNDATION NOTES AND THE PROJECT GEOTECHNICAL REPORT, T PROJECT GEOTECHNICAL REPORT SHALL GOVERN.

A SUBSURFACE GEOTECHNICAL INVESTIGATION HAS BEEN MADE BY GEOMAT, PROJECT NO. 222-4248 DATED JAN. 12. MOISTURE CONTENT: 2023. THE CONTRACTOR SHALLCOMPLY WITH ALL RECOMMENDATIONS CONTAINED IN THAT REPORT.

THE FOUNDATION SYSTEM FOR THIS PROJECT IS SPREAD FOOTINGS OVER ENGINEERED FILL.

THE SLAB-ON-GRADE FOR THIS PROJECT WAS NOT DESIGNED AS A STRUCTURAL DIAPHRAGM.

ADDITIONAL INFORMATION CONCERNING THE SPECIFIC SITE SOIL CONDITIONS TO BE ENCOUNTERED ARE IN THE PROJECT GEOTECHNICAL REPORT AND SHALL BE REVIEWED AND FULLY UNDERSTOOD BY THE CONTRACTOR.

F THERE ARE ANY QUESTIONS REGARDING THE REQUIREMENTS OF THE PROJECT GEOTECHNICAL REPORT, THE CONTRACTOR SHALL ISSUE RFI'S (REQUESTS FOR INFORMATION) TO THE ARCHITECT FOR CLARIFICATION. EARTHWORK SHALL NOT PROCEED UNTIL THE REQUIREMENTS OF THE PROJECT GEOTECHNICAL REPORT ARE FULLY UNDERSTOOD BY THE CONTRACTOR.

FIELD OBSERVATION AND TESTS

THE OWNER SHALL EMPLOY THE SERVICES OF A REGISTERED, LICENSED GEOTECHNICAL ENGINEER TO OBSERVE ALL CONTROLLED EARTHWORK. THE GEOTECHNICAL ENGINEER SHALL PROVIDE CONTINUOUS ON-SITE OBSERVATION BY EXPERIENCED PERSONNEL DURING CONSTRUCTION OF CONTROLLED EARTHWORK. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER AT LEAST TWO WORKING DAYS IN ADVANCE OF ANY FIELD OPERATIONS OF THE CONTROLLED EARTHWORK.

TESTS OF MATERIALS SHALL BE MADE AT THE FOLLOWING MINIMUM RATES. THE ON-SITE GEOTECHNICAL ENGINEER SHALL DETERMINE THE ACTUAL TESTING RATES:

ONE FIELD DENSITY TEST PER 2500 SQUARE FEET] OF COMPACTED SUBGRADE, PRIOR TO PLACING STRUCTURAL FILL OR SLAB-ON-GRADE, WITH A MINIMUM OF 3 TESTS.

ONE FIELD DENSITY TEST PER 2500 SQUARE FEET OF STRUCTURAL FILL PLACED OR PER EACH HORIZONTAL LAYER OF STRUCTURAL FILL, WHICHEVER IS THE GREATER NUMBER OF TESTS.

ONE MOISTURE-DENSITY CURVE FOR EACH TYPE OF MATERIAL USED, AS INDICATED BY THE SIEVE ANALYSIS AND THE PLASTICITY INDEX.

THE GEOTECHNICAL ENGINEER SHALL SUBMIT THE RESULTS OF ALL REQUIRED TESTS.

CLEARING AND GRUBBING:

REMOVE ALL BRUSH, RUBBISH, GRASS, AND GRASS ROOTS FROM THE CONSTRUCTION AREA.

REMOVE STUMPS, MATTED ROOTS AND ROCKS LARGER THAN 2 INCHES IN DIAMETER WITHIN 6 INCHES OF THE SURFACE OF AREAS ON WHICH FILL AND/OR FOOTINGS ARE TO BE CONSTRUCTED.

STRIP AND REMOVE ALL EXISTING PAVEMENT, FILL, DEBRIS AND OTHER DELETERIOUS MATERIALS FROM THE PROPOSED CONSTRUCTION AREA. ANY EXISTING STRUCTURES SHOULD BE COMPLETELY REMOVED FROM BELOW AND BUILDING, INCLUDING FOUNDATION ELEMENTS AND ANY ASSOCIATED DEVELOPMENT SUCH AS UNDERGROUND UTILITIES, SEPTIC TANKS, ETC. ALL EXPOSED SURFACES BELOW FOOTINGS AND SLABS SHOULD BE FREE OF MOUNDS AND DEPRESSIONS WHICH COULD PREVENT UNIFORM COMPACTION.

THE GEOTECHNICAL ENGINEER SHALL BE CONTACTED IF UNEXPECTED FILLS OR UNDERGROUND FACILITIES ARE ENCOUNTERED DURING SITE CLEARING.

STRIPPED MATERIALS CONSISTING OF VEGETATION AND ORGANIC MATERIALS SHOULD BE REMOVED FROM THE SITE, OR USED TO RE-VEGETATE EXPOSED SLOPES AFTER COMPLETION OF GRADING OPERATIONS. IF IT IS NECESSARY TO DISPOSE OF ORGANIC MATERIALS ON-SITE, THEY SHOULD BE PLACED IN NON-STRUCTURAL AREAS, AND IN FILL SECTIONS NOT EXCEEDING 5 FEET IN HEIGHT.

SLOPING AREAS STEEPER THAN 5:1 (HORIZONTAL:VERTICAL) SHOULD BE BENCHED TO REDUCE THE POTENTIAL FOR SLIPPAGE BETWEEN EXISTING SLOPES AND FILLS. BENCHES SHOULD BE LEVEL AND WIDE ENOUGH TO ACCOMMODATE COMPACTION AND EARTH MOVING EQUIPMENT.

ALL EXPOSED AREAS WHICH WILL RECEIVE FILL. ONCE PROPERLY CLEARED AND BENCHED WHERE NECESSARY. SHOULD BE SCARIFIED TO A MINIMUM DEPTH OF EIGHT INCHES, CONDITIONED TO NEAR OPTIMUM MOISTURE CONTENT, AND COMPACTED TO AT LEAST 95% OF MODIFIED PROCTOR (ASTM D1557).

EXCAVATION:

ADDITIONAL INFORMATION REGARDING EXCAVATION CONDITIONS SHALL BE EVALUATED BY CONTRACTORS FROM TEST EXCAVATIONS USING EQUIPMENT TO BE USED DURING CONSTRUCTION.

ON-SITE SOILS MAY PUMP OR BECOME UNSTABLE AT HIGH WATER CONTENTS, ESPECIALLY FOR EXCAVATIONS NEAR THE WATER TABLE. OVER-EXCAVATIONS OF WET ZONES AND REPLACEMENT WITH GRANULAR MATERIALS, DEWATERING, SCARIFYING, AND DRYING SHALL BE USED TO IMPROVE EXCAVATION CONDITIONS.

STANDARD EXCAVATION EQUIPMENT CAN BE USED ON-SITE. LIGHTWEIGHT EXCAVATION EQUIPMENT MAY REDUCE SUBGRADE PUMPING.

SITE, SUBFLOOR AND BEARING SURFACE PREPARATION:

EQUAL TO HALF THE WIDTH OF THE FOUNDATION OR 2 FEET.

A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER SHALL BE PRESENT TO CONFIRM COMPLETE EXCAVATION OF ANY UNCONTROLLED FILL.

ALL LOOSE AND/OR DISTURBED SOILS SHALL BE REMOVED OR COMPACTED FROM THE BOTTOMS OF FOOTING EXCAVATIONS PRIOR TO PLACEMENT OF REINFORCING STEEL AND/OR CONCRETE.

OVEREXCAVATE ALL SOILS UNDERLYING COLUMN FOOTINGS AND SITE WALL FOOTINGS AND ALL UNCONTROLLED FILL TO A MINIMUM DEPTH EQUAL TO THE WIDTH OF THE FOUNDATION OR 4 FEET.

OVEREXCAVATE ALL SOILS ADJACENT TO COLUMN FOOTINGS AND SITE WALL FOOTINGS TO A MINIMUM DISTANCE

OVEREXCAVATE ALL SOILS UNDERLYING FLOOR SLABS TO A MINIMUM DEPTH OF 2 FEET.

FLOOR SLABS SHALL BE PLACED ON ENGINEERING FILL TO A MINIMUM DEPTH OF 2 FEET, INCLDUING A 4 INCH AGGREGATE BASE COURSE.

PLACE ALL STRUCTURAL FILL IN APPROXIMATELY HORIZONTAL LAYERS NOT GREATER THAN 10 INCHES IN LOOSE THICKNESS, MOISTEN TO OPTIMUM MOISTURE CONTENT (+/- 2%) AND COMPACT TO DENSITY SPECIFIED HEREINAFTER.

ALL EARTHWORK FOR THE BUILDING PAD SHALL EXTEND A MINIMUM OF 5 FEET BEYOND THE PERIMETER FOOTINGS.

STRUCTURAL FILL REQUIREMENTS:

MAXIMUM EXPANSIVE POTENTIAL

GRADATION (ASTM C136):

SIEVE SIZE PERCENT PASSING BY WEIGHT

NO. 4 50-100 20-50

MATERIAL LARGER THAN 6 INCHES SHALL NOT BE PLACED IN THE STRUCTURAL FILL, AND MATERIAL LARGER THAN 4 INCHES SHALL NOT BE PLACED WITHIN TWELVE INCHES OF THE BEARING SURFACES OF SLABS OR FOUNDATIONS.

ON-SITE OR IMPORTED SOILS WITH LOW EXPANSIVE POTENTIALS SHALL BE USED IN STRUCTURAL FILL. PERIODIC TESTING SHALL BE PERFORMED DURING CONSTRUCTION TO CONFIRM THE SUITABILITY OF ON-SITE SOILS.

NO BRUSH, SOD, FROZEN MATERIAL OR OTHER UNSUITABLE MATERIAL SHALL BE PLACED IN THE STRUCTURAL FILL. MATERIAL SHALL BE PLACED IN SUCH A MANNER AS TO RESULT IN A UNIFORMLY COMPACTED FILL.

BASED ON THE REQUIREMENTS FOR THE STRUCTURAL FILL AND THE DESCRIPTION OF THE EXISTING SITE SOILS IN THE PROJECT GEOTECHNICAL REPORT. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE MOST APPROPRIATE METHOD FOR PROVIDING THE REQUIRED STRUCTURAL FILL. DEPENDING ON THE SITE CONDITIONS, APPROPRIATE METHODS COULD INCLUDE REBLENDING OF THE EXISTING SITE SOILS, MIXING THE EXISTING SITE SOILS WITH IMPORTED FILL, OR REMOVING THE EXISTING SITE SOILS ENTIRELY AND REPLACING WITH IMPORTED

GENERAL FOUNDATION NOTES GRANULAR BASE COURSE REQUIREMENTS:

AGGREGATE BASE SHOULD CONFORM TO CLASS I AGGREGATE BASE AS SPECIFIED IN SECTION 303 OF THE 2008 ARIZONA DEPARTMENT OF TRANSPORTATION (ADOT) "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE

COMPACTION REQUIREMENTS:

CONSTRUCTION".

IN ACCORDANCE WITH ASTM D1557 (MODIFIED PROCTOR), SUBGRADE SOILS AND STRUCTURAL FILL MATERIALS SHALL BE COMPACTED TO THE FOLLOWING PERCENTAGES OF THE MAXIMUM DRY DENSITY AT +/- 2% OPTIMUM

| MATERIAL | PERCENT COMPACTION |
|--------------------------------------|--------------------|
| STRUCTURAL FILL IN THE BUILDING AREA | 95 |
| SUBBASE FOR SLAB SUPPORT | 95 |
| SUBGRADE BELOW STRUCTURAL FILL | 95 |
| MISCELLANEOUS BACKFILL | 90 |
| SITE RETAINING WALL DESIGN CRITERIA: | |

LOADING CONDITION EQUIVALENT FLUID PRESSURE

| ACTIVE EARTH PRESSURE PASSIVE EARTH PRESSURE | 35 PCF |
|--|--------------------|
| SHALLOW FOUNDATION WALLS SHALLOW COLUMN FOOTINGS | 250 PCF 350 PCF |
| EARTH PRESSURE AT REST | 50 PCF |
| SOIL FRICTION FACTOR SOIL BEARING CAPACITY | 0.4 2500 PSF |

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

- 1. SPECIAL INSPECTIONS / TESTING "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED. BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.
- 2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.
- 3. REPORTING FOR SPECIAL INSPECTION SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.
- 4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.
- 5. SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.
- 6. IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110

7. DEFINITIONS:

* SPECIAL INSPECTION: INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).

* CONTINUOUS SPECIAL INSPECTION: FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. * PERIODIC SPECIAL INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

| ITEM | DESCRIPTION OF REQUIREMENTS | REQUIRED (YES/NO) |
|---|--|----------------------|
| SPECIAL INSPECTION OF STRUCTURAL STEEL | TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10 | YES |
| SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2 | YES |
| SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3 | YES |
| SPECIAL INSPECTIONS AND VERIFICATIONS FOR MASONRY CONSTRUCTION | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.4 AND REFERENCED STANDARDS | NO |
| SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5 | NO |
| SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES | YES |
| SPECIAL INSPECTIONS AND VERIFICATIONS FOR DEEP FOUNDATIONS (DRIVEN PILES, CAST-IN-PLACE, OR HELICAL PILES AS APPLICABLE) | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTIONS 1705.7-1705.9 AS APPLICABLE, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE CONSTRUCTION DOCUMENTS | NO |
| SPECIAL INSPECTIONS FOR WIND RESISTANCE (REQUIRED ONLY FOR Vult= 155MPH OR GREATER IN EXPOSURE CATEGORY B, OR Vult=142MPH OR GREATER IN EXPOSURE CATEGORY C OR D) | TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.11 | NO |
| SPECIAL INSPECTIONS AND VERIFICATIONS FOR SEISMIC RESISTANCE (REQUIRED FOR STRUCTURES ASSIGNED TO CATEGORIES C, D, E, OR F) | TO BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE PORTIONS OF IBC SECTIONS 1705.12 AND 1705.13 | NO |

NTU ENVIRONMENTAL LAB CHINLE

CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING

JANUARY 20, 2023



DYRON MURPHY ARCHITECTS, P.C.

4505 Montbel Place NE. Albuquerque, New Mexico 87107

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Revision Schedule

Description

Sheet Number

Date

NTU ENVIRONMENTAL LAB CHINLE

CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023

GENERAL SHEET NOTES

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- 3. NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR BRACED FRAME GUSSET PLATE CLEARANCE.
- 4. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- PROVIDE BLOCKOUTS AT ALL COLUMNS UNLESS NOTED OTHERWISE.
- 6. DIMENSIONS ARE TO THE FACE OF CONCRETE, STUD, OR GRID LINES, UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 8. PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2.1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2-#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- 9. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
- SEE SHEET S-501 FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- 11. SEE SHEETS S-701 THRU S-741 FOR TYPICAL DETAILS.
- 12. SEE SHEET S-601 FOR SCHEDULES.

○ SHEET KEYNOTES

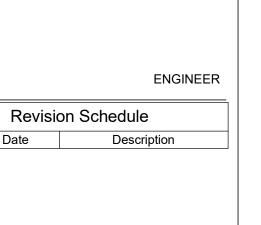
 FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.



DYRON MURPHY ARCHITECTS, P.C.



4505 Montbel Place NE, Albuquerque, New Mexico 87107

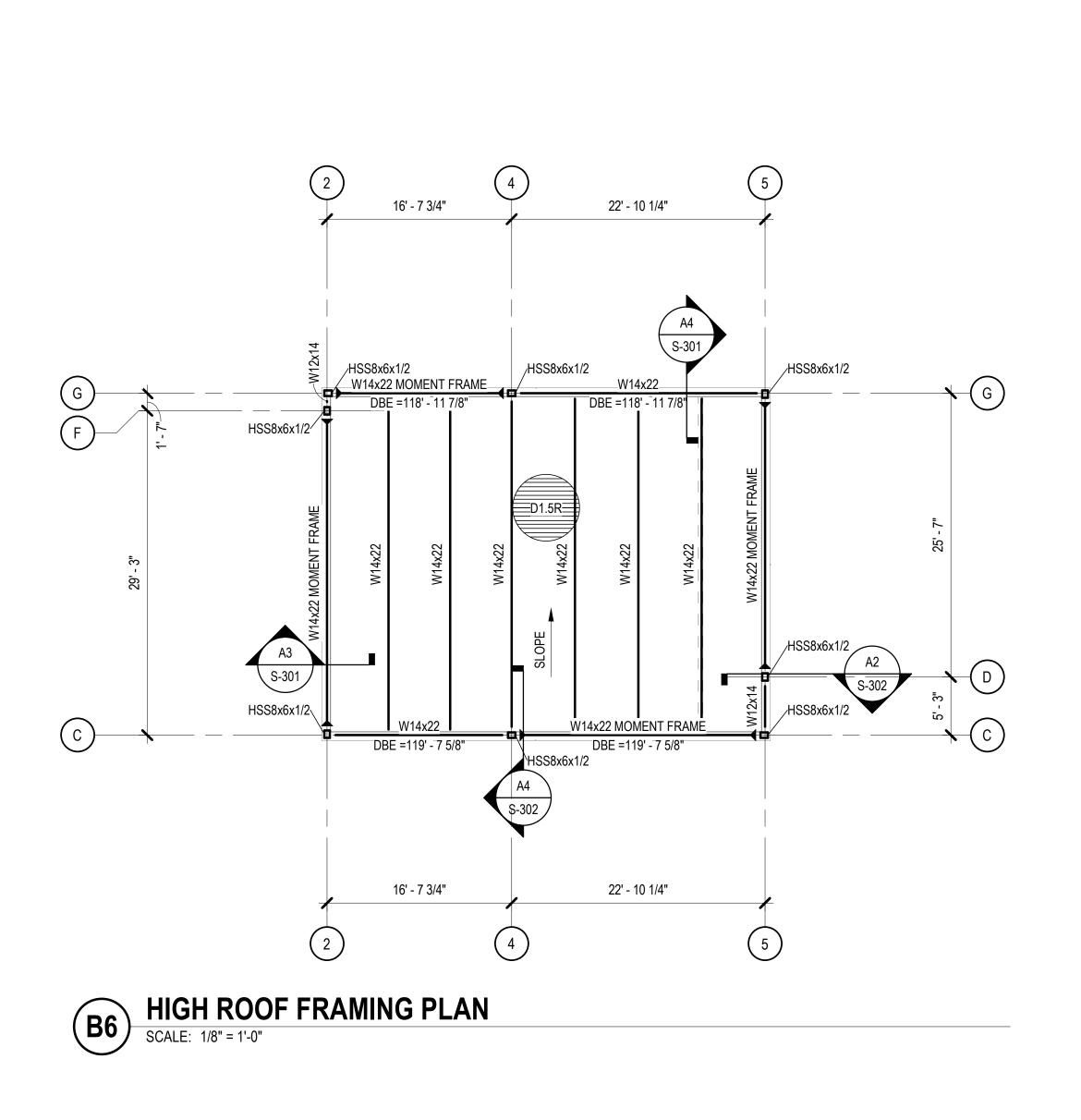


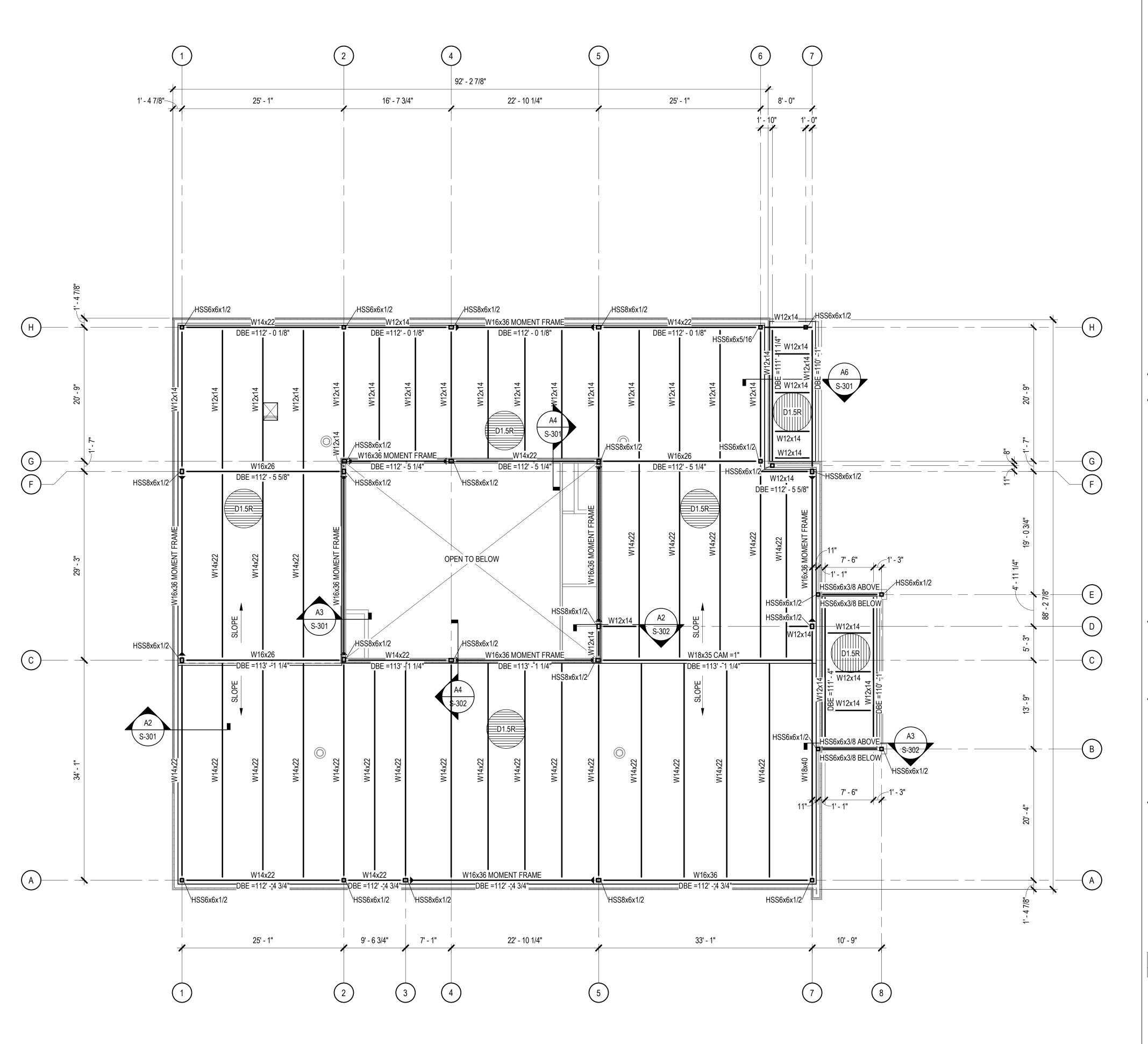
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S-101





NTU ENVIRONMENTAL LAB CHINLE

CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023

GENERAL SHEET NOTES

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- 3. DIMENSIONS ARE TO THE FACE OF STUD OR GRID LINES, UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 5. BEAMS ARE EQUALLY SPACED BETWEEN COLUMNS, UNLESS NOTED OTHERWISE.
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE [600S162-43] AT [16"] ON CENTER UNLESS NOTED OTHERWISE.
- 7. SEE SHEET S-501 FOR TYPICAL ROOF FRAMING SECTIONS.
- 8. SEE SHEET S-701 THRU S-742 FOR TYPICAL DETAILS.
- 9. SEE SHEET S-601 FOR SCHEDULES.

○ SHEET KEYNOTES

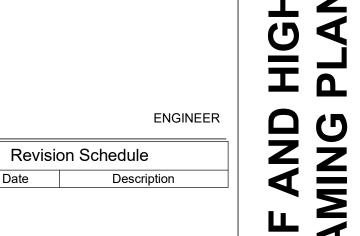
1. MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.



DYRON MURPHY ARCHITECTS, P.C.



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PROJECT NUMBER | DRAWN BY | PROJ
M97193 | THF | DF

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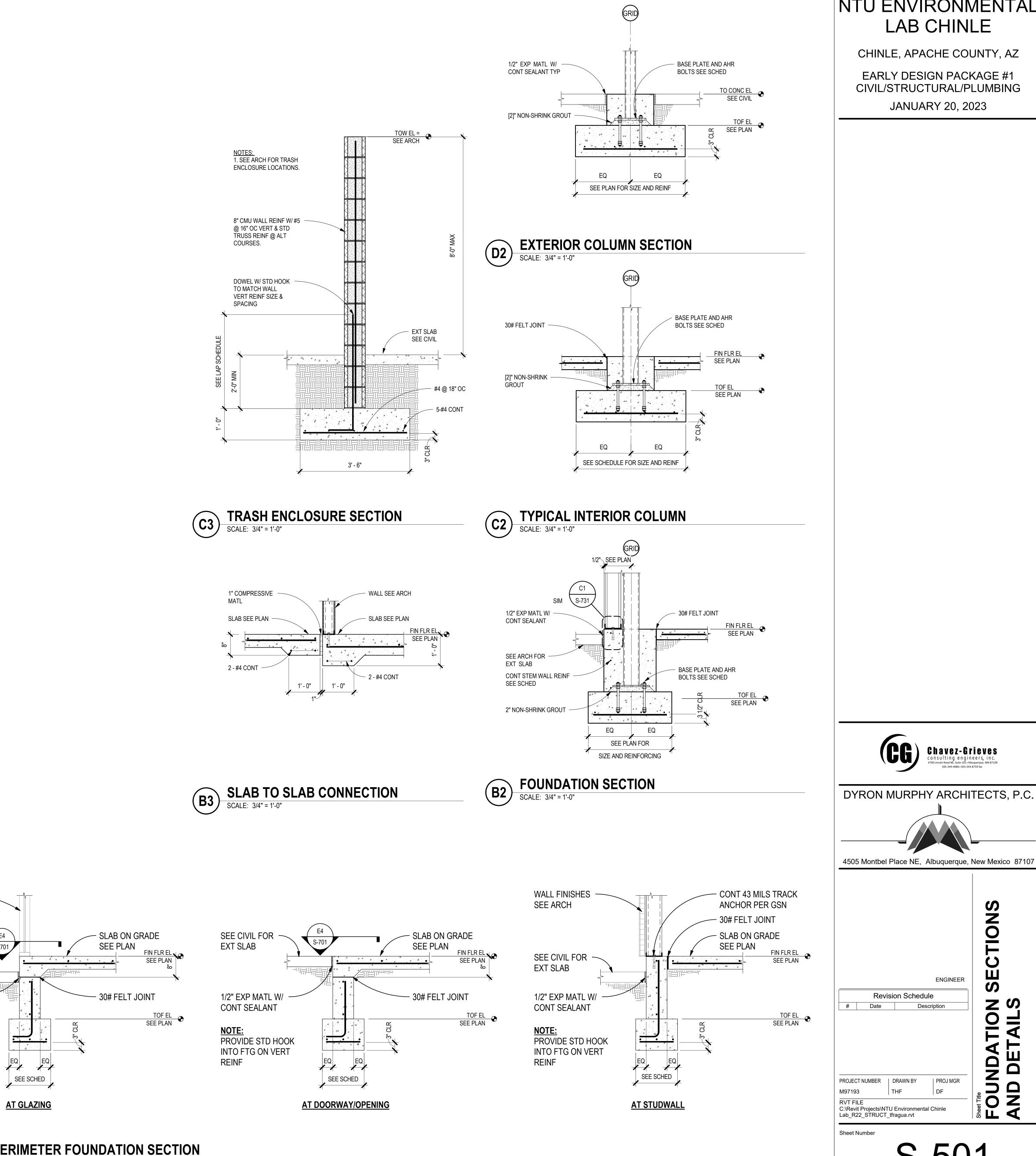
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S-121

ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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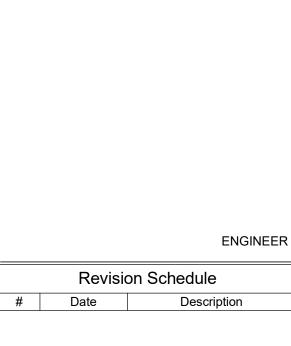


NTU ENVIRONMENTAL LAB CHINLE

CHINLE, APACHE COUNTY, AZ EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023



DYRON MURPHY ARCHITECTS, P.C.



FOUNDATION AND DETAILS

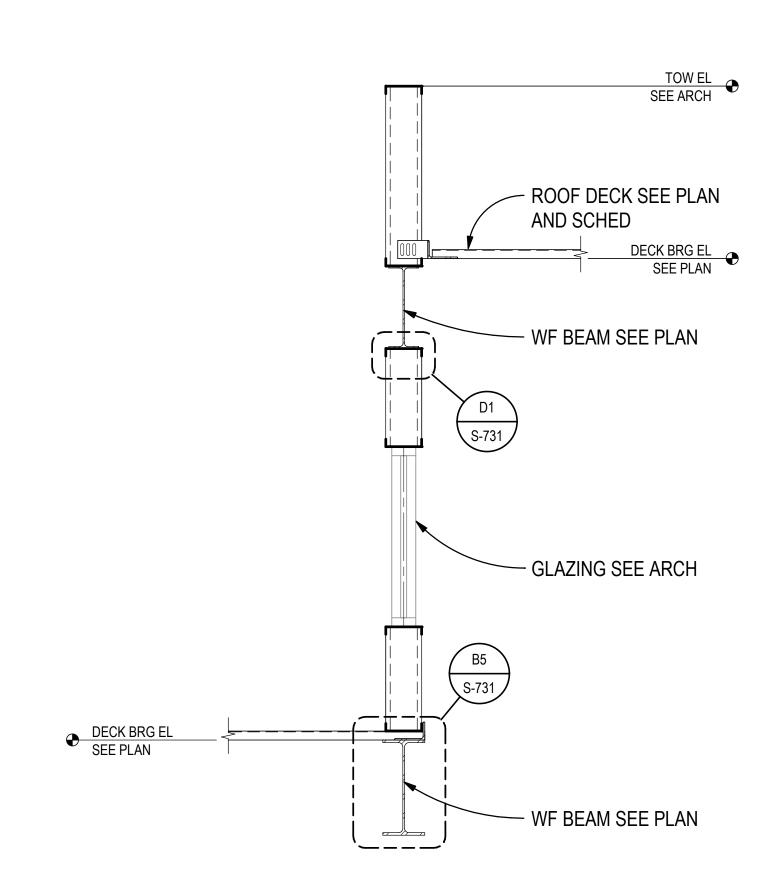
GLAZING SEE

SEE CIVIL FOR

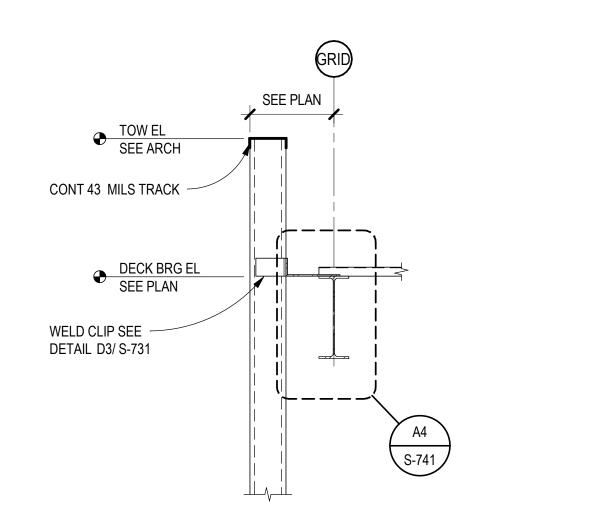
1/2" EXP MATL W/ CONT SEALANT

NOTE:
PROVIDE STD HOOK
INTO FTG ON VERT
REINF

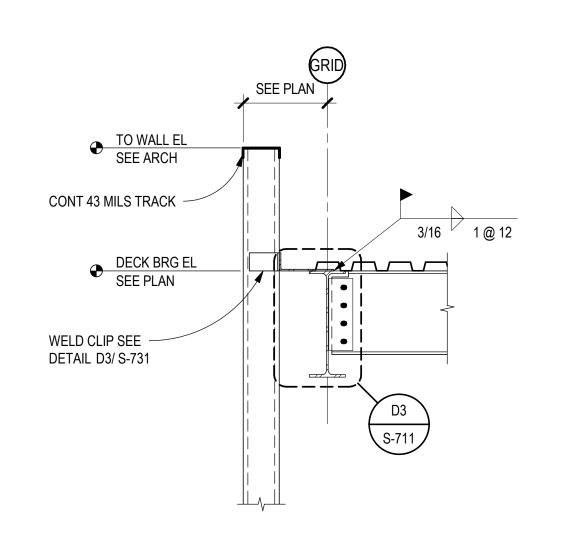
EXT SLAB



FRAMING SECTION @ ROOF STEP SCALE: 3/4" = 1'-0"











CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023



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4505 Montbel Place NE, Albuquerque, New Mexico 87107

ENGINEER Revision Schedule

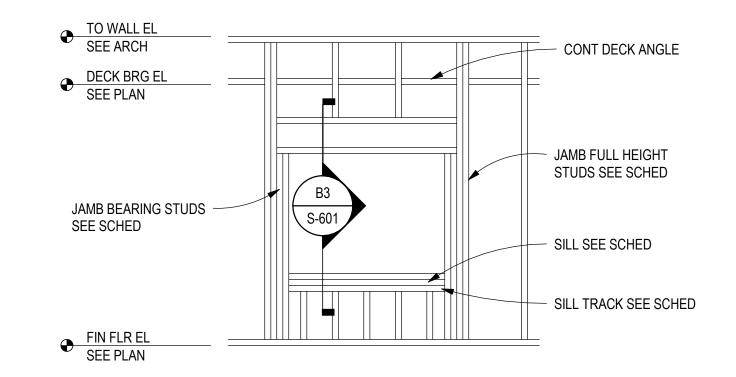
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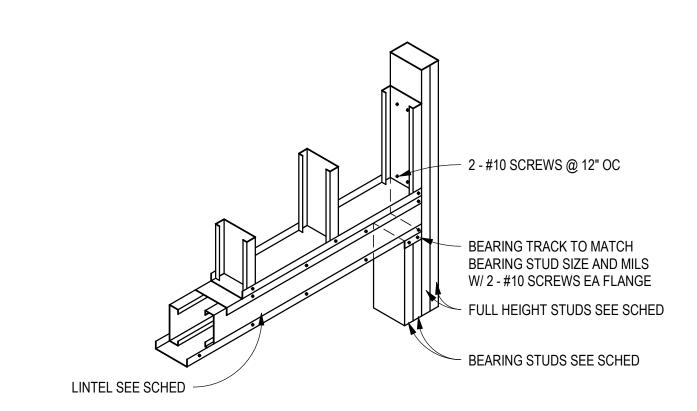
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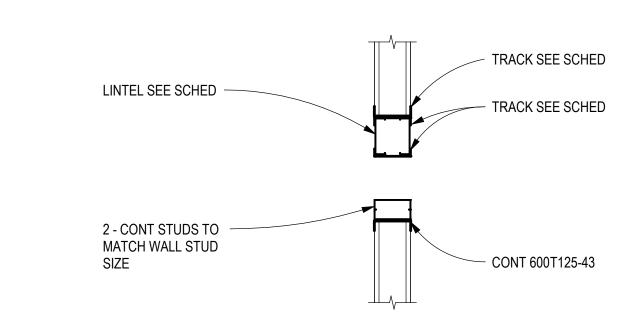
CHINLE, APACHE COUNTY, AZ

EARLY DESIGN PACKAGE #1 CIVIL/STRUCTURAL/PLUMBING JANUARY 20, 2023

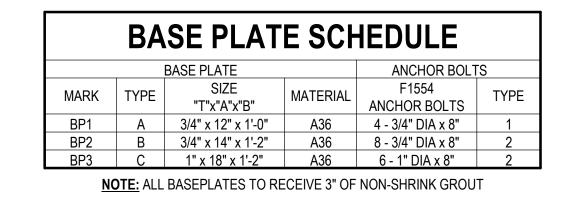
COLD-FORMED LINTEL SCHEDULE SPAN SIZE STUDS AT BEARING FULL HEIGHT STUDS TRACK 0'-0" - 3'-0" 2-600S162-43 1-600S162-43 CONT 600T125-43 3'-1" - 6'-0" 2-600S162-43 2-600S162-43 CONT 600T125-43 6'-1" - 9'-0" 2-600S162-54 2-600S162-43 2-600S162-43 CONT 600T125-54 9'-1" - 11'-0" 2-600S162-54 2-600S162-43 2-600S162-43 CONT 600T125-54

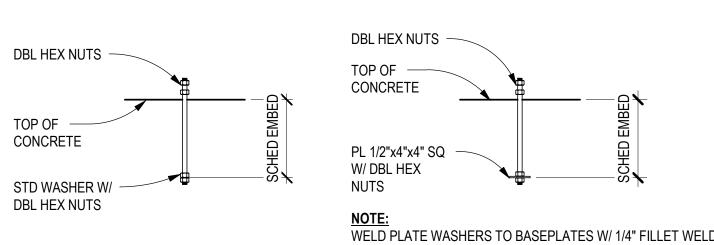






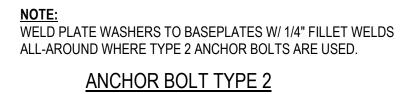




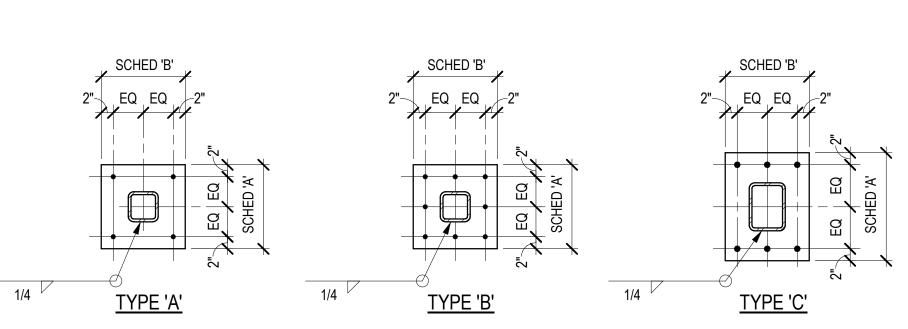


ANCHOR BOLT TYPE 1

MATL



COMMENTS



| (CG) | Chavez-Grieves consulting engineers, inc. 4700 Lincoln Road NE, Suite 102 - Albuquerque, NM 87109 505-344-4080 • 505-343-8759 fax |
|------|---|
| | 505-344-4080 • 505-343-8759 fax |

DYRON MURPHY ARCHITECTS, P.C.

4505 Montbel Place NE, Albuquerque, New Mexico 87107

Revision Schedule

Date

| | | V | ALL SCH | EDULE | |
|------|---------|-------------|-------------|----------|--|
| | | REINFO | ORCING | | |
| MARK | WALL | VERTICAL | HORIZONTAL | COMMENTS | |
| WC8 | 8" CONC | #4 @ 12" OC | #4 @ 12" OC | | |

NORMAL WEIGHT #4 @ 12" OC EACH WAY TOP CONC & BOT 15 MIL VAPOR RETARDER OVER SUBGRADE PER GEN STRUCT NOTES

SLAB-ON-GRADE SCHEDULE

BEARING STRATA

15 MIL VAPOR RETARDER OVER SUBGRADE PER GEN STRUCT NOTES

REINFORCING

NORMAL WEIGHT #4 @ 18" OC EACH WAY

| | SPOT FOOTING SCHEDULE | | | | | | | |
|------|-----------------------|---------|---------|-------------------------|------------------------------|--|--|--|
| | | SIZE | | | | | | |
| MARK | WIDTH | LENGTH | DEPTH | REINFORCING | COMMENTS | | | |
| F1 | 3' - 0" | 3' - 0" | 1' - 0" | 5 - #5 EA WAY TOP & BOT | STD HOOK AT ENDS OF ALL BARS | | | |
| F2 | 5' - 0" | 5' - 0" | 1' - 0" | 5 - #5 EA WAY TOP & BOT | STD HOOK AT ENDS OF ALL BARS | | | |
| F3 | 6' - 0" | 6' - 0" | 1' - 0" | 6 - #5 EA WAY TOP & BOT | STD HOOK AT ENDS OF ALL BARS | | | |
| F4 | 7' - 0" | 7' - 0" | 1' - 0" | 6 - #5 EA WAY TOP & BOT | STD HOOK AT ENDS OF ALL BARS | | | |

| | CONTINUOUS FOOTING SCHEDULE | | | | | | |
|------|-----------------------------|---------|-------------|-------------|----------|--|--|
| | SIZ | ZE | REINFORCING | | | | |
| MARK | WIDTH | DEPTH | CONTINUOUS | TRANSVERSE | COMMENTS | | |
| CF16 | 1' - 4" | 1' - 0" | 3 - #4 BOT | #5 @ 12" OC | | | |

| | DECK SCHEDULE | | | | | | | | |
|-------|---------------|------|----------|------------|--|--------------------------------|---------------------|--------------|----------|
| | | MI | ETAL DEC | < | DEC | K ATTACHMENTS | | TOTAL SLAB / | 1 |
| | | | | | | | | DECK | 1 |
| MARK | DECK | TYPE | GAGE | FINISH | ATTACH PERP TO RIBS | ATTACH PARALLEL TO RIBS | ATTACH SIDELAPS | THICKNESS | COMMENTS |
| D1.5R | 1 1/2" | В | 20 | G60 | 4-5/8" DIA PUDDLE WELDS PER 36" WIDE SHEET | 5/8" DIA PUDDLE WELDS @ 12" OC | #10 SCREWS @ 12" OC | 1 1/2" | |
| | | | | GALVANIZED | | | | | _ |

| REINFORCEMENT TYPE | #6 AND SMALLE | R (NUMBER OF B | AR DIAMETERS) | #7 AND LARGE | R (NUMBER OF B | AR DIAMETERS) | MINIMUM | COMMENTS |
|---|---------------|----------------|---------------|--------------|----------------|---------------|-------------|---|
| REINFORGEWENT TIPE | 3000 PSI | 4000 PSI | 5000 PSI | 3000 PSI | 4000 PSI | 5000 PSI | LENGTH (IN) | COMMENTS |
| CONTINUOUS WALL FOOTINGS AND HORIZONTAL REINFORCEMENT IN SITE WALLS AND STEMWALLS | 30 | 30 | 30 | 30 | 30 | 30 | 18 | |
| CONCRETE WALLS: ALL VERTICAL REINFORCEMENT | 57 | 50 | 45 | 72 | 62 | 56 | 12 | |
| CONCRETE WALLS: ALL HORIZONTAL REINFORCEMENT, EXCLUDING SITE WALLS AND STEMWALLS | 75 | 65 | 58 | 93 | 81 | 72 | 12 | |
| CONCRETE COLUMNS | 57 | 50 | 45 | 72 | 62 | 56 | 12 | |
| TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED FOOTING COLUMNS | 75 | 65 | 58 | 93 | 81 | 72 | 12 | |
| BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS | 57 | 50 | 45 | 72 | 62 | 56 | 12 | |
| MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE | 22 | 19 | 17 | 22 | 19 | 17 | 6 | ALLOWED FOR BARS LARGER THAN #11 |
| SLABS-ON-GRADE | | 30 | | | 30 | | 12 | |
| SLABS OVER METAL DECK | | 30 | | | 30 | | 6 | WELDED WIRE FABRIC MINIMUM LAP LENGTH = 6 INCHES |
| ALL CMU LAPS UNLESS NOTED OTHERWISE | | 48 | | | 48 | | 18 | |

NOTES:

1. LAP SPLICES SHALL NOT BE PERMITTED FOR BARS LARGER THAN #11 IN CONCRETE OR #9 IN MASONRY. SUCH SPLICES SHALL USE APPROVED MECHANICAL CONNECTIONS

2. LAP SPLICES FOR BUNDLED BARS SHALL BE IN ACCORDANCE WITH ACI 318

3. LAP LENGTHS FOR LIGHTWEIGHT CONCRETE SHALL BE INCREASED BY 33%4. LAP LENGTHS FOR EPOXY COATED BARS SHALL BE INCREASED BY 50%

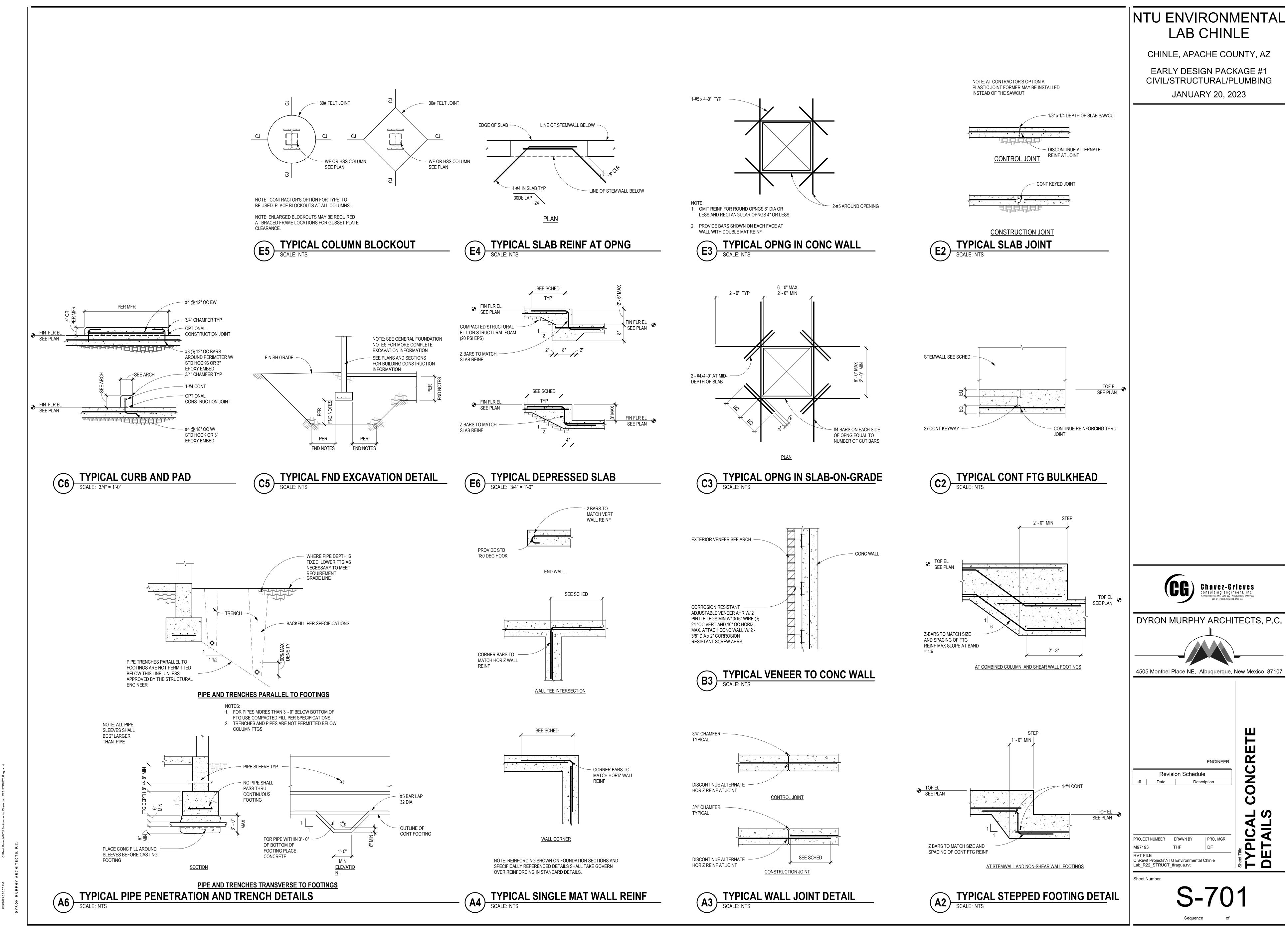
5. FOR INTERMEDIATE OR LARGER VALUES OF F'c, USE THE CLOSEST LOWER VALUE IN THE TABLE. DO NOT INTERPOLATE

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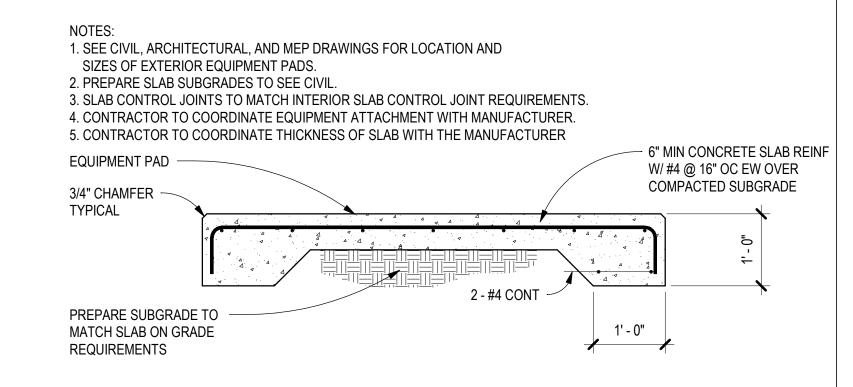
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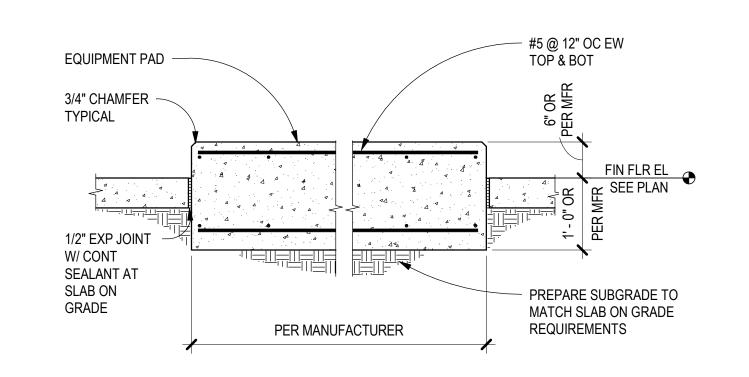


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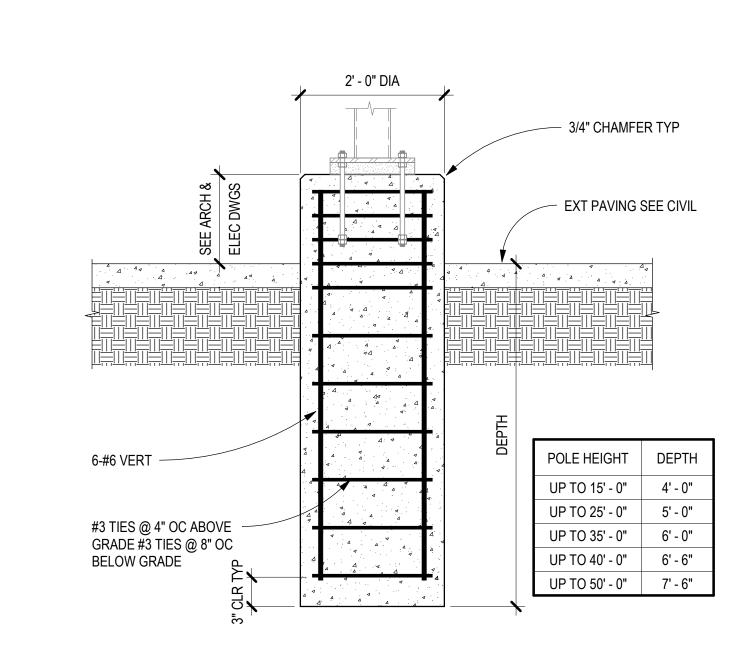
TYPICAL TRENCH SECTION SCALE: 3/4" = 1'-0"



C2 ISOLATED EQUIPMENT PAD SCALE: 3/4" = 1'-0"



B2 INT ISOLATED EQUIPMENT PAD SCALE: 3/4" = 1'-0"



LIGHT/ FLAG POLE BASE DETAIL SCALE: 3/4" = 1'-0"

KEY KEY KEY
DEPTH WIDTH REINFORCING

NONE

NONE

NONE NONE

4 - #4 CONT TOP & BOT & #4 @ 18" OC NONE NONE TRANS TOP & BOT

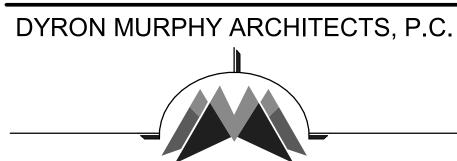


CHINLE, APACHE COUNTY, AZ

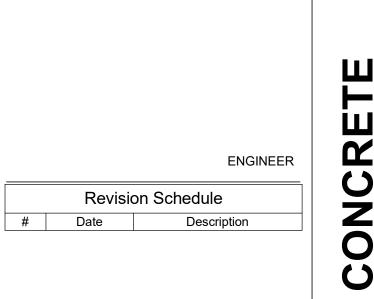
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JANUARY 20, 2023









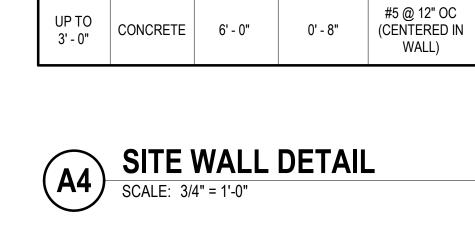
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Sheet Number

S-702



RETAINED WALL HEIGHT WALL HEIGHT ABOVE SOIL THICKNESS

MASONRY 6' - 0" 0' - 8"

AT MASONRY — WALLS, PROVIDE

NON - RETAINED

DOWELS W/ STD HOOK TO MATCH WALL VERT REINF HOOK INWARD AS SHOWN

VERTICAL VERTICAL NON-RETAIN SIDE RETAIN SIDE REINFORCING REINFORCING

NONE

NONE

KEY WIDTH

2 - #5 @ 48" OC 3' - 6"

#4 @ 12" OC

FOOTING WIDTH

1' - 6"

1' - 6"

1' - 5"

FOOTING THICKNESS

REINFORCING

4 - #4 CONT TOP &

BOT & #4 @ 18" OC TRANS TOP & BOT

HORIZONTAL FOOTING FOOTING REINFORCEMENT WIDTH THICKNESS

FOOTING REINF

KEY REINF

#4 @ 16" OC (CENTERED IN WALL) **RETAIN SIDE REINF**

2 - #5 CONT IN TOP COURSE

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