

GEOTECHNICAL EVALUATION REPORT

DENNEHOTSO LOOP ROAD IMPROVEMENTS

Dennehotso, Arizona WT Reference No. 3127JS001

PREPARED FOR:

Dibble Engineering 7500 North Dreamy Draw Drive, Suite No. 200 Phoenix, Arizona 85020-4996

March 22, 2017



Roger K. Southworth, P.E. Managing Director

Arm ando de la Rocha, P.E. Senior Geotechnical Engineer Western 400 South Lorena Avenue Farmington, New Mexico 87401-5943 (505) 327-4966 • fax 327-5293

March 22, 2017

Dibble Engineering 7500 North Dreamy Draw Drive, Suite 200 Phoenix, Arizona 85020-4996

Attn: Mr. Drew Spear, P.E.

Re: Geotechnical Evaluation

Dennehotso Loop Road Improvements

Dennehotso, Arizona

Western Technologies Inc. has completed the geotechnical evaluation for the above-referenced project. The results of our evaluation, including the boring location diagram, boring logs, laboratory test results, and the geotechnical recommendations are attached.

Job No. 3127JS001

We have appreciated being of service to you in the geotechnical engineering phase of this project and are prepared to assist you during the construction phases as well. Please do not hesitate to contact us if the design conditions change or if you have any questions concerning this report. We look forward to working with you on future projects.

Sincerely,

WESTERN TECHNOLOGIES INC.

Roger K. Southworth, P.E.

Managing Director

TABLE OF CONTENTS

	ROJECT DESCRIPTION	
3.0 SC	.1 Field Exploration	_
	.1 Field Exploration	
	·	
	.2 Laboratory Testing	
	.3 Analyses and Report	
4.0 SIT	ITE CONDITIONS	3
	.1 Surface	
	.2 Subsurface	
5.0 PA	AVEMENT RECOMMENDATIONS	3
	.1 General	
	.2 Design Analysis	
	.3 Design Traffic Loading	
	.4 Design Parameters	
	.5 Recommended Pavement Sections	
6.0 EA	ARTHWORK	5
	.1 General	
	.2 Materials	
	.3 Corrosivity	
	.4 Compliance	
7.0 LIN	MITATIONS	6
8.0 CL	LOSURE	/
SITE LO	CATION DIAGRAM	Plate 1
	G LOCATION DIAGRAMPlate	
APPEND	DIX A	
r	Definition of Terminology	۸ 1
	<u> </u>	
	Method of Soil Classification	
	Boring Log Notes	
E	Boring Logs A-4	through A-33
APPEND	DIX B	



Laboratory Test Results B-1

TABLE OF CONTENTS (CONTINUED)

APPEN	DIX C	
	Traffic Data	C-1 through C-4
APPEN	DIX D	
	PaveXpress Output	D-1 through D-6

GEOTECHNICAL EVALUATION DENNEHOTSO LOOP ROAD IMPROVEMENTS DENNEHOTSO, ARIZONA

JOB NO. 3127JS001

1.0 PURPOSE

This report contains the results of our geotechnical evaluation for the reconstruction of the Dennehotso Loop Road. The purpose of these services is to provide information and recommendations for roadway reconstruction. The results of the field exploration and the field and laboratory testing programs are presented in the Appendices.

2.0 PROJECT DESCRIPTION

The project will consist of reconstructing the Dennehotso Loop Road. The reconstruction route includes N6460 and N6461, and two access roads that extend from US Highway 160 to N6460. The reconstruction will include approximately 6 miles of roadway and will follow the alignments of the current roadways. Grade changes of less than five feet will be required to develop the proposed finish site grades. The proposed reconstruction is shown on the attached *Site Location Diagram* (Plate 1).

3.0 SCOPE OF SERVICES

3.1 Field Exploration

Thirty borings were drilled for this project to a depth of 10 feet. The borings were drilled at the approximate locations indicated on the attached *Boring Location Diagram* (Plates 2 through 5). The approximate station number and ground surface elevation at each boring location is shown on the attached boring logs.

A WT engineer monitored the drilling operations and prepared a field log for each boring. These logs contain visual classifications of the materials encountered during drilling, as well as interpolation of the subsurface conditions between samples.

The final boring logs, included in Appendix A, represent our interpretation of the field logs and may include modifications based on laboratory observations of the recovered soil samples. The final logs describe the materials encountered, their thicknesses, and the depths at which samples were obtained.



Dibble Engineering Job No. 3127JS001

The Unified Soil Classification System was used to classify the soil. The soil classification symbols appear on the boring logs and are briefly described in Appendix A.

3.2 <u>Laboratory Testing</u>

Laboratory tests were performed on representative samples to aid in material classification and to estimate the pertinent engineering properties of the soil. Testing was performed in general accordance with applicable ASTM methods. The following tests were performed and the results are presented in Appendix B.

- Grain Size Distribution
- Water Content
- Liquid and Plastic Limits
- Sulfates

The laboratory test results were used to develop the recommendations contained in this report.

3.3 Analyses and Report

Analyses were performed and this report was prepared for the exclusive purpose of providing geotechnical engineering information and recommendations. The scope of services for this project does not include, either specifically or by implication, any environmental assessment of the site or identification of contaminated or hazardous materials or conditions. If the owner is concerned about the potential for such contamination, other studies should be undertaken. We are available to discuss the scope of such studies with you.

This geotechnical engineering report includes a description of the project, a discussion of the field and laboratory testing programs, a discussion of the subsurface conditions, and design recommendations as required to satisfy the purpose previously described.



4.0 SITE CONDITIONS

4.1 Surface

The existing roadways are typically unimproved with the road surface consisting of native silty sand and/or sandstone bedrock. The roads drain by sheet flow to ditches along the sides of the roads.

4.2 Subsurface

The borings typically encountered silty sand underlain by sandstone. The depth to sandstone varied from being exposed at the ground surface to greater than 10 feet, the maximum depth explored. Exceptions to this general stratigraphy were encountered in Borings B-14, B-15, and B-20, where a layer of lean clay was encountered at depths of about 2 to 5 feet. In addition, silty sand fill was encountered in Boring B-20 to a depth of about 2 feet. Groundwater was not encountered in the borings during drilling.

5.0 PAVEMENT RECOMMENDATIONS

5.1 General

The recommendations contained in this report are based on our understanding of the project criteria described in Section 2.0, **Project Description**, and the assumption that the soil and subsurface conditions are those disclosed by the borings. Others may change the plans, final elevations, or details of the project during design or construction. Substantially different subsurface conditions from those described herein may be encountered or become known. Any changes in the project criteria or subsurface conditions shall be brought to our attention in writing.

5.2 Design Analysis

The pavement design was performed in accordance with AASHTO design procedures using commercial computer software. A description of the pavement design methodology and recommended pavement sections are provided in the following sections.

5.3 Design Traffic Loading

The design traffic loading is based upon information contained in Average Daily Traffic (ADT) Reports dated April 2013. The reports included traffic counts at two locations; one located



0.7 miles northwest of US Highway 160 and N6460, and one located 0.2 miles east of the BIA School Access Road. The reports indicated average daily traffic (ADT) of 269 and 187 vehicles per day (VPD), respectively. The studies did not include determining the amount of truck traffic. The reports indicated a growth factor of 2 percent. Copies of the reports are attached in Appendix C.

The design lane traffic loading was based on an average daily traffic (ADT) of 269 vehicles per day. A growth factor of 2 percent and a lane distribution factor of 1.0 was assumed for design. The traffic distribution was based upon 98% pickup trucks and 2% combination trucks. The design period for the pavement was 10 years. The pavement design was therefore based upon a total traffic loading of 26,663 18-kip equivalent single-axle loads (ESALs). Detailed ESAL calculations are presented in Appendix C.

5.4 <u>Design Parameters</u>

The following parameters were used in the pavement design:

TABLE 1 – PAVEMENT DESIGN PARAMETERS

Design Parameter	Design Value
18-kip Equivalent Single-Axle Loads, ESALs	26,663
Reliability, R	70%
Overall Deviation, S ₀	0.5
Initial Serviceability	4.5
Terminal Serviceability	2.5
Effective Soil Resilient Modulus, (M _R) _{eff}	20,000 psi
Base Course Structural Layer Coefficient	0.12
Asphalt Structural Coefficient	0.44

The design effective soil resilient modulus was based upon published correlations between the soil classification and the resilient modulus.



5.5 Recommended Pavement Sections

On the basis of the above design criteria, pavement sections were developed for three different scenarios; aggregate surfaced, chip-seal, and asphalt pavement. The recommended sections for each of these alternatives are presented in the following table.

TABLE 2 - RECOMMENDED PAVEMENT SECTIONS

Layer Component		Layer Thickness	
	Alternative No. 1 Aggregate Surface	Alternative No. 2 Asphalt Pavement	Alternative No. 3 Chip Seal
НМАС	-	2 inches	1 inch
Base Course	9 inches	4 inches	8 inches
Compacted Soil Subgrade	8 inches	8 inches	8 inches

The output sheets from the PaveXpress software used in the analysis and the pavement design calculations are presented in Appendix D.

The "design life" of a pavement is defined as the expected life at the end of which reconstruction of the pavement will need to occur. Pavement construction should be performed in accordance with the *Navajo Nation Road Standards and Engineering Specifications for Earth/Dirt and Gravel Roads*. The gradient of paved surfaces should ensure positive drainage. Water should not be allowed to pond in areas directly adjoining paved sections.

6.0 EARTHWORK

6.1 General

The conclusions contained in this report are contingent upon compliance with recommendations presented in this section. Any excavating, trenching, or disturbance that occurs after completion of the earthwork must be backfilled, compacted, and tested in accordance with the recommendations contained herein. It is not reasonable to rely upon our conclusions and recommendations if any future unobserved and untested trenching, earthwork activities, or backfilling occurs.



6.2 Materials

The on-site soil can be used as embankment fill and as fill in the planned pavement areas. Imported fill should have a minimum effective soil resilient modulus of 20,000 psi.

6.3 **Corrosivity**

Nine soil sulfate tests were performed in accordance with EPA Method 300.0. The test results indicated sulfate contents as high as 3,200 parts per million (ppm). The chemical test results indicate that the soils at the site classify as corrosive to concrete. We recommended that all concrete in contact with the site soil be made with Type V or equivalent cement, as set forth in ACI 318-4.3.

6.4 Compliance

Recommendations for pavement elements supported on compacted fill or prepared subgrade depend upon compliance with the **Earthwork** recommendations. To assess compliance, observation and testing should be performed under the direction of the project geotechnical engineer.

7.0 LIMITATIONS

This report has been prepared assuming the project criteria described in **Section 2.0**. If changes in the project criteria occur, or if different subsurface conditions are encountered or become known, the conclusions and recommendations presented herein shall become invalid. In any such event, WT should be contacted in order to assess the effect that such variations may have on our conclusions and recommendations.

The recommendations presented are based entirely upon data derived from a limited number of samples obtained from widely spaced borings. The attached logs are an indicator of subsurface conditions only at the specific locations and times noted. This report assumes the uniformity of the geology and soil structure, however variations can and often do exist. Whenever any deviation, difference or change is encountered or becomes known, WT should be contacted.

This report is for the exclusive benefit of our client alone. There are no intended third-party beneficiaries of our contract with the client or this report, and nothing contained in the contract or



Dibble Engineering Job No. 3127JS001

this report shall create any express or implied contractual or any other relationship with, or claim or cause of action for, any third party against WT.

This report is valid for the earlier of one year from the date of issuance, a change in circumstances, or discovered variations. After expiration, no person or entity shall rely on this report without the express written authorization of WT.

8.0 CLOSURE

We prepared this report as an aid to the designers of the proposed project. The comments, statements, recommendations and conclusions set forth in this report reflect the opinions of the authors. These opinions are based upon data obtained at the boring locations and from laboratory tests. Work on your project was performed in accordance with generally accepted standards and practices utilized by professionals providing similar services in this locality. No other warranty, express or implied, is made.







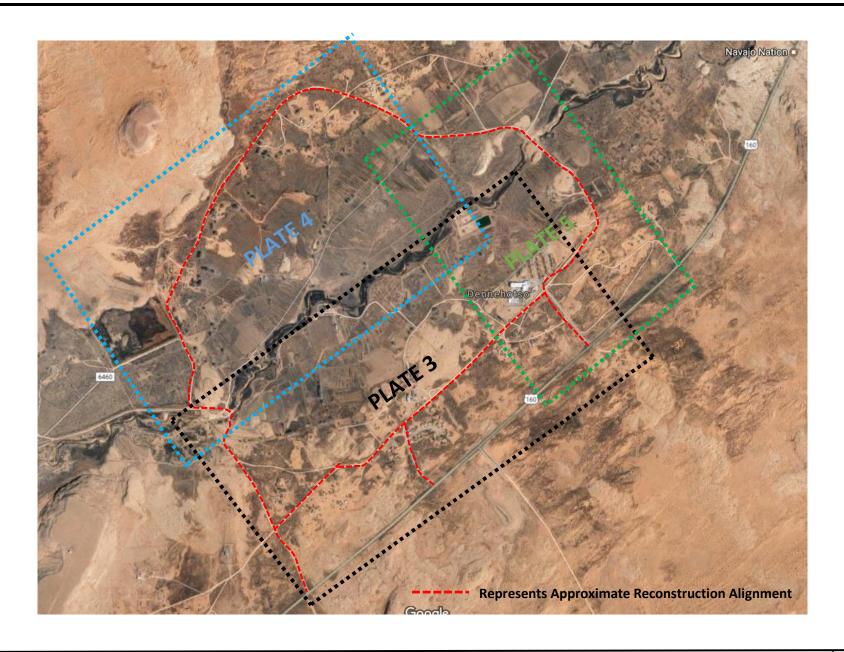


PROJECT: PROPOSED DENNEHOTSO LOOP ROAD

JOB NO.: 3127JS001

SITE LOCATION DIAGRAM







PROJECT: PROPOSED DENNEHOTSO LOOP ROAD

JOB NO.: 3127JS001

BORING LOCATION DIAGRAM





APPROXIMATE BORING LOCATION



PROJECT: PROPOSED DENNEHOTSO LOOP ROAD

JOB NO.: 3127JS001

BORING LOCATION DIAGRAM





APPROXIMATE BORING LOCATION

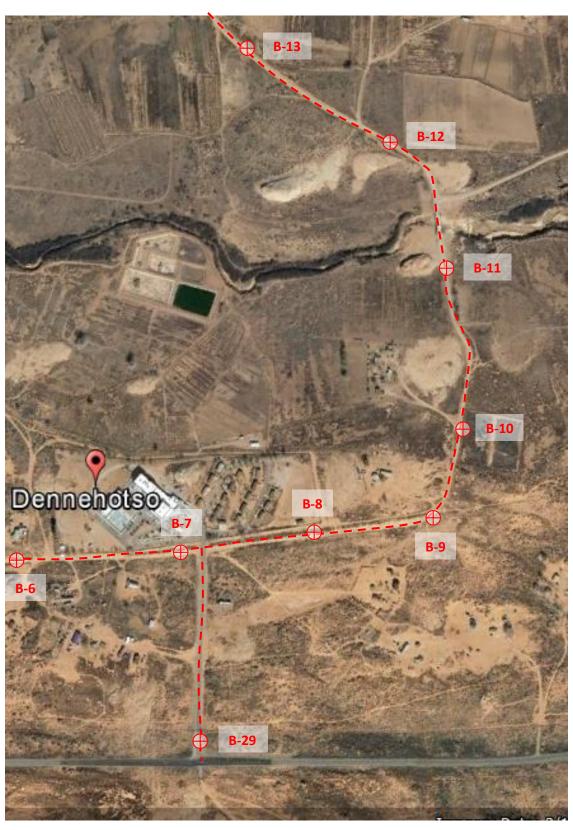


PROJECT: PROPOSED DENNEHOTSO LOOP ROAD

JOB NO.: 3127JS001

BORING LOCATION DIAGRAM





Approximate Boring Location

Geotechnical
Environmental
Inspections
Materials

Western
Technologies Inc.
The Quality People
Since 1955

wt-us.com

PROJECT: PROPOSED DENNEHOTSO LOOP ROAD

JOB NO.: 3127JS001

BORING LOCATION DIAGRAM

APPENDIX A

Allowable Soil Bearing Capacity The recommended maximum contact stress developed at the interface of the

foundation element and the supporting material.

Backfill A specified material placed and compacted in a confined area.

Base Course A layer of specified aggregate material placed on a subgrade or subbase.

Base Course Grade Top of base course.

Bench A horizontal surface in a sloped deposit.

Caisson/Drilled Shaft A concrete foundation element cast in a circular excavation which may have an

enlarged base (or belled caisson).

Concrete Slabs-On-Grade A concrete surface layer cast directly upon base course, subbase or subgrade.

Crushed Rock Base Course A base course composed of crushed rock of a specified gradation.

Differential Settlement Unequal settlement between or within foundation elements of a structure.

Engineered Fill Specified soil or aggregate material placed and compacted to specified density and/or

moisture conditions under observations of a representative of a soil engineer.

Existing Fill Materials deposited through the action of man prior to exploration of the site.

Existing Grade The ground surface at the time of field exploration.

Expansive Potential The potential of a soil to expand (increase in volume) due to absorption

of moisture.

Fill Materials deposited by the actions of man.

Finished Grade The final grade created as a part of the project.

Gravel Base CourseA base course composed of naturally occurring gravel with a specified gradation.

Heave Upward movement.

Native Grade The naturally occurring ground surface.

Native Soil Naturally occurring on-site soil.

Rock A natural aggregate of mineral grains connected by strong and permanent cohesive

forces. Usually requires drilling, wedging, blasting or other methods of extraordinary

force for excavation.

Sand and Gravel Base Course A base course of sand and gravel of a specified gradation.

Sand Base Course A base course composed primarily of sand of a specified gradation.

Scarify To mechanically loosen soil or break down existing soil structure.

Settlement Downward movement.

Soil Any unconsolidated material composed of discrete solid particles, derived from the

physical and/or chemical disintegration of vegetable or mineral matter, which can be

separated by gentle mechanical means such as agitation in water.

Strip To remove from present location.

Subbase A layer of specified material placed to form a layer between the subgrade and base

course.

Subbase Grade Top of subbase.

Subgrade Prepared native soil surface.



DEFINITION OF TERMINOLOGY

PLATE

A-1

COARSE-GRAINED SOILS

LESS THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
GW	WELL-GRADED GRAVEL OR WELL-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	GRAVELS
GP	POORLY-GRADED GRAVEL OR POORLY-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	MORE THAN HALF OF COARSE
GM	SILTY GRAVEL OR SILTY GRAVEL WITH SAND, MORE THAN 12% FINES	FRACTION IS LARGER THAN NO. 4
GC	CLAYEY GRAVEL OR CLAYEY GRAVEL WITH SAND, MORE THAN 12% FINES	SIEVE SIZE
sw	WELL-GRADED SAND OR WELL-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	SANDS
SP	POORLY-GRADED SAND OR POORLY-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	MORE THAN HALF OF COARSE
SM	SILTY SAND OR SILTY SAND WITH GRAVEL, MORE THAN 12% FINES	FRACTION IS SMALLER THAN NO. 4
sc	CLAYEY SAND OR CLAYEY SAND WITH GRAVEL, MORE THAN 12% FINES	SIEVE SIZE

NOTE: Coarse-grained soils receive dual symbols if they contain 5% to 12% fines (e.g., SW-SM, GP-GC).

SOIL SIZES

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. – 12 in.
GRAVEL Coarse Fine	No. 4 – 3 in. ¾ in. – 3 in. No. 4 – ¾ in.
SAND Coarse Medium Fine	No. 200 – No. 4 No. 10 – No. 4 No. 40 – No. 10 No. 200 – No. 40
Fines (Silt or Clay)	Below No. 200

NOTE: Only sizes smaller than three inches are used to classify soils

PLASTICITY OF FINE GRAINED SOILS

PLASTICITY INDEX	TERM
0	NON-PLASTIC
1 – 7 8 – 20	LOW MEDIUM
Over 20	HIGH

FINE-GRAINED SOILS

MORE THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
ML	SILT, SILT WITH SAND OR GRAVEL, SANDY SILT, OR GRAVELLY SILT	SILTS
CL	LEAN CLAY OF LOW TO MEDIUM PLASTICITY, SANDY CLAY, OR GRAVELLY CLAY	CLAYS LIQUID LIMIT
OL	ORGANIC SILT OR ORGANIC CLAY OF LOW TO MEDIUM PLASTICITY	LESS THAN 50
МН	ELASTIC SILT, SANDY ELASTIC SILT, OR GRAVELLY ELASTIC SILT	SILTS AND
СН	FAT CLAY OF HIGH PLASTICITY, SANDY FAT CLAY, OR GRAVELLY FAT CLAY	CLAYS LIQUID LIMIT
ОН	ORGANIC SILT OR ORGANIC CLAY OF HIGH PLASTICITY	MORE THAN 50
РТ	PEAT AND OTHER HIGHLY ORGANIC SOILS	HIGHLY ORGANIC SOILS

NOTE: Fine-grained soils may receive dual classification based upon plasticity characteristics (e.g. CL-ML).

CONSISTENCY

CLAYS & SILTS	BLOWS PER FOOT
VERY SOFT SOFT FIRM STIFF VERY STIFF HARD	0 - 2 3 - 4 5 - 8 9 - 15 16 - 30 OVER 30

RELATIVE DENSITY

SANDS & GRAVELS	BLOWS PER FOOT
VERY LOOSE	0 – 4
LOOSE	5 – 10
MEDIUM DENSE	11 – 30
DENSE	31 – 50
VERY DENSE	OVER 50

NOTE: Number of blows using 140-pound hammer falling 30 inches to drive a 2-inch-OD (1%-inch ID) split-barrel sampler (ASTM D1586).

DEFINITION OF WATER CONTENT

DRY	
SLIGHTLY DAMP	
DAMP	
MOIST	
WET	
SATURATED	

Geotechnical
Environmental
Inspections
Materials
Western
Technologies Inc.
The Quality People
Since 1955
wt-us.com

METHOD OF CLASSIFICATION

PLATE

A-2

The number shown in **"BORING NO."** refers to the approximate location of the same number indicated on the "Boring Location Diagram" as positioned in the field by pacing or measurement from property lines and/or existing features, or through the use of Global Positioning System (GPS) devices. The accuracy of GPS devices is somewhat variable.

"DRILLING TYPE" refers to the exploratory equipment used in the boring wherein HSA = hollow stem auger, and the dimension presented is the outside diameter of the HSA used.

"N" in "BLOW COUNTS" refers to a 2-inch outside diameter split-barrel sampler driven into the ground with a 140 pound drop-hammer dropped 30 inches repeatedly until a penetration of 18 inches is achieved or until refusal. The number of blows, or "blow count", of the hammer is recorded for each of three 6-inch increments totaling 18 inches. The number of blows required for advancing the sampler for the last 12 inches (2nd and 3rd increments) is defined as the Standard Penetration Test (SPT) "N"-Value. Refusal to penetration is considered more than 50 blows per 6 inches. (Ref. ASTM D1586).

"R" in "BLOW COUNTS" refers to a 3-inch outside diameter ring-lined split barrel sampler driven into the ground with a 140 pound drop-hammer dropped 30 inches repeatedly until a penetration of 12 inch is achieved or until refusal. The number of blows required to advance the sampler 12 inches is defined as the "R" blow count. The "R" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows per foot. (Ref. ASTM D3550).

"CS" in "BLOWS/FT." refers to a 2½-in. outside diameter California style split-barrel sampler, lined with brass sleeves, driven into the ground with a 140-pound hammer dropped 30 inches repeatedly until a penetration of 18 inches is achieved or until refusal. The number of blows of the hammer is recorded for each of the three 6-inch increments totaling 18 inches. The number of blows required for advancing the sampler for the last 12 inches (2nd and 3rd increments) is defined as the "CS" blow count. The "CS" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows for a 6-inch increment. (Ref. ASTM D 3550)

"SAMPLE TYPE" refers to the form of sample recovery, in which N = Split-barrel sample, R = R Ring-lined sample, "CS" = California style split-barrel sample, R = R Grab sample, R = R Bucket sample, R = R Core sample (ex. diamond bit rock coring).

"DRY DENSITY (LBS/CU FT)" refers to the laboratory-determined dry density in pounds per cubic foot. The symbol "NR" indicates that no sample was recovered.

"WATER (MOISTURE) CONTENT" (% of Dry Wt.) refers to the laboratory-determined water content in percent using the standard test method ASTM D2216.

"USCS" refers to the "Unified Soil Classification System" Group Symbol for the soil type as defined by ASTM D2487 and D2488. The soils were classified visually in the field, and where appropriate, classifications were modified by visual examination of samples in the laboratory and/or by appropriate tests.

These notes and boring logs are intended for use in conjunction with the purposes of our services defined in the text. Boring log data should not be construed as part of the construction plans nor as defining construction conditions.

Boring logs depict our interpretations of subsurface conditions at the locations and on the date(s) noted. Variations in subsurface conditions and characteristics may occur between borings. Groundwater levels may fluctuate due to seasonal variations and other factors.

The stratification lines shown on the boring logs represent our interpretation of the approximate boundary between soil or rock types based upon visual field classification at the boring location. The transition between materials is approximate and may be more or less gradual than indicated.

Geotechnical
Environmental
Inspections
Materials

Western
Technologies Inc.
The Quality People
Since 1955

BORING LOG NOTES

PLATE

A-3

DATE DE LOCATION ELEVAT	ON: See ION: 50	Borin	-20-17 g Locatio	n Diagr	am	ВО	RING NO. B-1 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		G G		5	SM	S	Boring terminated at 10 feet	
R- NR- G- B-	RING SAM NO SAMP GRAB SA BUCKET :	MPLE PLE REC MPLE SAMPLE					NOTES: Groundwater not encountered during dri Boring Location: Station No. 11+00	illing
) O	GIES IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	ヲ"	E O I I	ERN IE	OUN()LU(GIES IN	BORING LOG	A -4

LOCAT	ORILLED: ION: See TION: 50	Borir	1-20 ig Lo		Diagr	am	E	EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
8.9		G G			5	SM		SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDAF RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COVE	ERY				NOTES: Groundwater not encountered during dr Boring Location: Station No. 19+00	illing
	<u> </u>	IEGT	FP	N TE	CHNC) O	GIES	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLA
7	ラ゛	IE31	⊏KI	14 IE(OHINC)LU	GIE∂	BORING LOG	A -

LOCATI	RILLED: ON: Sec TION: 50	Borir	1-20 ng L		n Diagı	ram	E	BORING NO. B-3 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
	L	G G			5-	SM		SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
R- NR- G- B-	STANDAR RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COV E	ERY				NOTES: Groundwater not encountered during dri Boring Location: Station No. 30+00	lling
	<u>,,</u>	/FQT	FP	N TE	CHN) (O	GIES	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	丿"	, E 3 1	Ľĸ	14 IE	OHN	JLU	SIES	BORING LOG	Α-(

LOCAT	ORILLED: TON: Se TION: 5 (e Boriı 036.4		0-17 ₋ocatioı	n Diagr	ram	E	BORING NO. B-4 EQUIPMENT TYPE: CME-7 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dum	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		G G			5-	-		SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDA RING SA NO SAMI GRAB SA BUCKET BLUNT N	MPLE PLE RE AMPLE SAMPL	COV .E	/ERY		-		NOTES: Groundwater not encountered duri Boring Location: Station No. 41-	
	y v	VEST	ER	kN TE	CHNO	OLO!	GIES		 PLATE A-7
_ `								BORING LOG	

		Borin	1-20 ig L		n Diagı	ram	В	ORING NO. B-5 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
	a.	S G			5-			SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
R- NR- G- B-	STANDAF RING SAM NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPL	COVI E	ERY				NOTES: Groundwater not encountered during dri Boring Location: Station No. 52+00	illing
	<u></u>	/FQT	FP	N TE	СНИ	OI (0)	GIES	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLA1
	ヺ゛	01	∟ ∩	14 IE	O1114(J_U	JILJ	BORING LOG	A-8

SANDSTONE; orange-brown, moderately hard G G SANDSTONE; orange-brown, moderately hard SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet Boring terminated at 10 feet NOTES: Groundwater not encountered during drilling Boring Location: Station No. 67+00 BUCKET SAMPLE BILLINT NOSE PENETROMETER PROJECT: PROPOSED BENNEHOTSO LOOP ROAD PLATER OF THE PROPOSED DENNEHOTSO LOOP ROAD PLATER OF THE PROPOSE	DATE DRILL LOCATION: ELEVATION	See Be	oring	:0-17 Locatio	n Diagr	am	В	ORING NO. B-6 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumit	
SANDSTONE; orange-brown, damp SANDSTONE; orange-brown, moderately hard SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet NOTES: Groundwater not encountered during drilling Boring Location: Station No. 67+00 BUILDIT SAMPLE BLUNT NOSE PENETROMETER PROJECT: PROPOSED DENNEHOTSO LOOP ROAD PLA REF. NO. 3127/35901	WATER CONTENT (%) POCKET	œ		BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC		
R- RING SAMPLE NR- NO SAMPLE RECOVERY G- GRAB SAMPLE B- BUCKET SAMPLE BN- BLUNT NOSE PENETROMETER PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001			<u>6</u> 6		5-	SM		SANDSTONE; orange-brown, moderately hard	
REF. NO.: 3127JS001	R- RINC NR- NO S G- GRA B- BUC	G SAMPL SAMPLE AB SAMP CKET SAM	LE RECO PLE MPLE	VERY					_
WESTERN TECHNOLOGIES INC.						DLO:	GIES	DEE NO : 2427 IS004	

DATE DR	ON: See	Boring	-20-17 J Locatio	n Diagr	am	В	ORING NO. B-7 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA	
ELEVATI	ON: 502	21.7	_	_	1		FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	DЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		S G G		5-	SM		Boring terminated at 10 feet	
R- F NR- N G- G	STANDAR RING SAM NO SAMPI GRAB SAM BUCKET S	IPLE LE REC MPLE		TEST			NOTES: Groundwater not encountered during dr Boring Location: Station No. 79+00	illing
	BLUNT NO	OSE PEN	NETROME [*]				PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
44	W	ESTE	RN TE	CHNO	DLO	GIES I	NC.	A-1

LOCATI	RILLED: ON: See TION: 50	e Borir)-17 ocatio	n Diagr	am	E	BORING NO. B-8 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION
6.9	d	/S G G			5-	SM		SILTY SAND; light orange-brown, damp SANDSTONE; orange-brown, moderately hard
R- NR- G-	STANDAI RING SAI NO SAMF GRAB SA	MPLE PLE RE MPLE	COV		10-			NOTES: Groundwater not encountered during drilling Boring Location: Station No. 98+00
	BUCKET BLUNT N	OSE PI	ENE ⁻		CHNO	OLO:	GIES	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001 A-*
	フ							BORING LOG

LOCAT	RILLED: ION: See TION: 50	Borir	1-20 ng L		n Diag	ram	E	BORING NO. B-9 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
	F. Control of the con	G G			5-	SM		SILTY SAND; light orange-brown, damp SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDAI RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COV E	ERY				NOTES: Groundwater not encountered during drill Boring Location: Station No. 106+00	ing
	<u> </u>	/EST	ER	N TF	CHN	OLO	GIFS	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	ヺ	0 1	-1\	16	J1114	J_0	JILU	BORING LOG	A-1

DATE DRII	N: See	Boring	-20-17 J Locatio	n Diagr	am	BOI	RING NO. B-10 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA	
WATER CONTENT (%)	PENETROMETER 30 (tsf)	щ	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	FIELD ENGINEER: C. Dumitru SOIL DESCRIPTION	
3.5		<u></u> 8G G G		5-	SP-SM		DORLY GRADED SAND; with silt, light brown, damp Boring terminated at 10 feet	
R- RII NR- NC G- GF B- BU	NG SAM D SAMPL RAB SAM JCKET S	PLE LE REC MPLE SAMPLE			<u> </u>	<u> </u>	NOTES: Groundwater not encountered during of Boring Location: Station No. 115+00	_
DIV- BL	<u> </u>) O	OIEO IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	y "	ESIE	KN IE	CHN	JLU	GIES IN	BORING LOG	⊢ A-1

LOCATI	ΓΙΟΝ: 50	Borin	-20-17 g Locatio	n Diagr	am	BOI	RING NO. B-11 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	USCS	GRAPHIC	SOIL DESCRIPTION	
		G G		5	SM		ANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
R- NR- G- B-	RING SAM NO SAMF GRAB SA BUCKET	MPLE PLE REC MPLE SAMPLE					NOTES: Groundwater not encountered during dri Boring Location: Station No. 122+00	illing
) O	GIES INC	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	ラ "	/E311	-KN IE	OUNC	LU	JIE9 IN(BORING LOG	A-1

LOCATI	RILLED: ION: Sec IION: 50	Borin	1-20- ig Lo		n Diagr	am	В	DRING NO. B-12 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	SOSO	GRAPHIC	SOIL DESCRIPTION	
5.2	a.	/s			5	SM		SANDSTONE; orange-brown, moderately hard Boring terminated at 10 feet	
	STANDAR RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPLI	COVE	ERY				NOTES: Groundwater not encountered during dri Boring Location: Station No. 153+00	lling
	À,	/FST	FRI	N TF	СНИС) ()	GIES I	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	フ ¨		-: \!		J. 1140	J_0	J.20 I	BORING LOG	A-1

DATE DRI	N: See I DN: 500 8	Boring	20-17 Locatio	n Diagra	am	BOF	RING NO. B-13 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKE I PENETROMETER (tsf)	SAMPLE TYPE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		G G G		5	SM	SII	Boring terminated at 10 feet	
R- RI NR- NC G- GF B- BL	NG SAMI D SAMPL RAB SAM JCKET SA	PLE E RECO IPLE AMPLE	TRATION VERY			,	NOTES: Groundwater not encountered during dri Boring Location: Station No. 161+00	illing
	<u> </u>)I O	SIES INC	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	y '''	_3 i E	1411	CITING	,_0(JILO INC	BORING LOG	A-1

LOCATI	RILLED: ION: Sec IION: 50	Borin	1-20 ig L		n Diagr	ram	ВО	NG NO. B-14 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru		
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	SOSU	GRAPHIC	SOIL DESCRIPTION		
		G G			5	CL		EAN CLAY; with interbedded layers of POORLY GRADED SAI brown to red-brown, moist Boring terminated at 10 feet	ND	
N- R- NR- G- B- BN-	STANDAF RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPL	COVI E	ERY				NOTES: Groundwater not encountered during drilling Boring Location: Station No. 169+00		
) O	GIES IN	REF. NO.: 3127JS001	_A7	
	ラ"	/E31	⊏K	N IE	CUN	JLU	GIES IN	BORING LOG	\-1	

LOCATI	RILLED: ION: Sec TION: 50	e Borir)-17 .ocatio	n Diagr	ram	В	ORING NO. B-15 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	DRILLING TYPE: 7" HSA		
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	DЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION			
7.8	<u> </u>	s G	X		_	SM		SILTY SAND; light orange-brown, damp			
18.4		G			_	CL		LEAN CLAY; with sand, brown to red-brown, moist			
9.2		G			5	SM		SILTY SAND; light orange-brown, damp			
					-	-		Boring terminated at 10 feet			
N- R- NR- G- B- BN-	STANDAI RING SAI NO SAME GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COV .E	ERY				NOTES: Groundwater not encountered during dri Boring Location: Station No. 177+00	lling		
	<u> </u>	/EST	.E.D	N TE	СПИ) O	GIES	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLA1		
	フ ゛	ve31	Ľĸ	41 V 1E	O IN	JLU	SIES	BORING LOG	A-1		

DATE DR LOCATIO ELEVATION	N: See ON: 502	Boring	-20-17 Locatio	n Diagr	am	BOR	PROPERTY TO SERVICE AND SERVIC		
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION		
		0		5	SM	SIL	Boring terminated at 10 feet		
R- R NR- N G- G B- B	RING SAM IO SAMPI BRAB SAM BUCKET S	IPLE LE RECO MPLE SAMPLE					NOTES: Groundwater not encountered during dri Boring Location: Station No. 185+00	illing	
	<u> </u>) O	GIES INC	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT	
	丿"	ESIE	.NN IE	UNIO		SIES INC	BORING LOG	A-19	

LOCATI	ΓΙΟΝ: 50	Borin	I-20-17 g Locat	tion Diagr	am	BOF	RING NO. B-17 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
5.2		S G		5—	SM	SI	Boring terminated at 10 feet	
	STANDAF RING SAM NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPLE	COVERY				NOTES: Groundwater not encountered during dr Boring Location: Station No. 193+00	illing
) O	GIES INC	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
7	ラ"	/E311	EKIN I	COUNC	JLU	GIES INC	BORING LOG	A-2

DATE DE LOCATION ELEVAT	ON: See ION: 50	Borin	-20-17 g Locati	on Diagr	am	ВО	RING NO. B-18 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	USCS	GRAPHIC	SOIL DESCRIPTION	
		G G		5	SM	S	Boring terminated at 10 feet	
R- NR- G- B-	STANDAF RING SAM NO SAMP GRAB SA BUCKET S	MPLE PLE REC MPLE SAMPLE	COVERY				NOTES: Groundwater not encountered during dr Boring Location: Station No. 235+00	illing
					ח סי	GIES IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	ヺ゚	11	-1714	_011140	,_0(OILO IIV	BORING LOG	A-2

LOCAT	RILLED: ION: Sec TION: 50	Borin	-20-17 g Locatio	on Diagr	am	ВО	RING NO. B-19 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		G	8.6	5-	SM	S	ILTY SAND; light brown, damp	
N- R- NR- G- B- BN-	STANDAF RING SAM NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPLE	OVERY				NOTES: Groundwater not encountered during dri Boring Location: 243+00	illing
					OLO:	GIES IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	フ ¨	0 1 1	-1314 16		J_0	J.E.J IIV	BORING LOG	A-2

LOCATI	RILLED: ION: See TION: 50	Borir)-17 ocatio	n Diagr	am	В	EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		S G			5	SM		(FILL) SILTY SAND; light brown, damp LEAN CLAY; dark gray, moist SILTY SAND; light orange-brown, damp Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDAR RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COV E	ERY				NOTES: Groundwater not encountered during drilli Boring Location: Station No. 251+00	ing
) (O	GIES	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLA ⁻
	ヺ ゛	0 1	-1\	I E	J11140	<i>-</i> _0	JILU	BORING LOG	A-2

DATE DR LOCATIO ELEVATION	N: See ON: 503	Boring	-20-17 Locatio	n Diagr	am	ВО	RING NO. B-21 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
	Δ.	<u>/s</u> G		5	SM	S	Boring terminated at 10 feet	
R- R NR- N G- G B- B	ING SAM O SAMPL RAB SAN UCKET S	PLE LE RECO MPLE SAMPLE					NOTES: Groundwater not encountered during dri Boring Location: Station No. 258+00	illing
	<u> </u>				טו ממ	GIES IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	丿"	_016		OI IIV	,_0(SILO IIV	BORING LOG	A-2

LOCAT	RILLED: ION: See TION: 50	Borin)-17 ocatio	n Diagr	am	В	EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
	_	s G	X		_	SP- SM		POORLY GRADED SAND; with silt, dark brown-gray, dar	np
3.8		G			5	SM		SILTY SAND; light orange-brown, damp Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDAR RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COV E	ERY				NOTES: Groundwater not encountered during d Boring Location: Station No. 289+00	
	 ,	/FST	FP	N TE	CHNO	טו הי	GIFS	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLA -
)	0 1	'\		J. 1140	<i>-</i>	J.LU	BORING LOG	A-2

DATE DRILLED: 1-20-1 LOCATION: See Boring Loc ELEVATION: 5035.4	BUIL	NG NO. B-23 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru
WATER CONTENT (%) POCKET PENETROMETER (tst) SAMPLE TYPE SAMPLE	BLOWS/FT. DEPTH (FEET) USCS GRAPHIC	SOIL DESCRIPTION
8 G G G	SM SILT	DSTONE; orange-brown, moderately hard, dry Boring terminated at 10 feet
N- STANDARD PENETRA' R- RING SAMPLE NR- NO SAMPLE RECOVER		NOTES: Groundwater not encountered during drilling
G- GRAB SAMPLE B- BUCKET SAMPLE BN- BLUNT NOSE PENETR		Boring Location: Station No. 297+00 PROJECT: PROPOSED DENNEHOTSO LOOP ROAD PLAT
		PROJECT: PROPOSED DENNEHOTSO LOOP ROAD PLAT

DATE DE LOCATION ELEVAT	ON: Sec ION: 50	Borin	1-20 g Lo		n Diagr	ram	ВС	RING NO. B-24 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
0	Δ.	<u>/\$</u>			5-	SM		ANDSTONE; orange-brown, dry Boring terminated at 10 feet	
R- NR- G- B-	STANDAF RING SAM NO SAMF GRAB SA BUCKET: BLUNT N	MPLE PLE REC MPLE SAMPL	COVE	ERY				NOTES: Groundwater not encountered during dr Boring Location: Station No. 318+00	illing
) ()	GIES IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	ヺ゛		- 1\					BORING LOG	A-2

LOCAT	RILLED: ION: Sec TION: 50	Borir)-17 ocatio	n Diagı	ram	В	EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
8.8		S G G			5-	SM		SILTY SAND; light orange-brown, damp SANDSTONE; orange-brown, moderately hard, dry Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDAR RING SAI NO SAME GRAB SA BUCKET BLUNT N	MPLE PLE REG MPLE SAMPL	COV E	ERY				NOTES: Groundwater not encountered during drilling Boring Location: Station No. 326+00	
	<u> </u>	/EST	ER	N TF	CHN	OLO	GIFS	REF. NO.: 3127JS001	LAT
	フ "	0 1			J. 114		JU	BORING LOG	۷-2

DATE DI LOCATIO ELEVAT	ON: Sec ION: 50	Borir)-17 .ocatio	n Diagr	ram	В	EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	NSCS	GRAPHIC	SOIL DESCRIPTION
6.3	ā	<u>^</u> G G			5-	SM		SILTY SAND; light orange-brown, damp SANDSTONE; orange-brown, moderately hard, dry
R- NR-	STANDAF RING SAI NO SAMF GRAB SA	MPLE PLE RE			TEST	-		NOTES: Groundwater not encountered during drilling Boring Location: Station No. 334+00
B-	BUCKET BLUNT N	SAMPL OSE PE	ENE					PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001
4	"	/EST	ER	N TE	CHN	OLO	GIES	BORING LOG

LOCATI	RILLED: ION: Sec IION: 50	Borin	1-20- g Lo		Diagr	am	В	DRING NO. B-27 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
O .	Δ.	8 <mark>8</mark> 0 0			5— — —————————————————————————————————	SM		SANDSTONE; orange-brown, moderately hard, dry Boring terminated at 10 feet	
R- NR- G- B-	STANDAF RING SAI NO SAMF GRAB SA BUCKET	MPLE PLE REC MPLE SAMPLI	COVE	RY				NOTES: Groundwater not encountered during dri Boring Location: Station No. 340+00	lling
BN-	BLUNT N) O	GIES I	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	ラ"	IEO II	⊏KI\	4 IEC	ZTINC	JLU	GIES I	BORING LOG	A-3

DATE DI LOCATIO ELEVAT	ON: See	Borir	1-20 ng L		n Diagı	ram	ВО	(IIIO IIO. D-20	EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE	BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC		ESCRIPTION	
2.6	d.	<u>/s</u> G			5-	SM		ANDSTONE; orange-brown, n		
R- NR- G- B-	STANDAF RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REI MPLE SAMPL	COV .E	ERY					not encountered during dr tion: Station No. +00	illing
	N	/EST	ER	N TE	CHN	OLO	GIES IN	PROJECT: PROPOSED DENNE REF. NO.: 3127JS001	EHOTSO LOOP ROAD	PLA
	フ							BORIN	IG I OG	~~`

LOCATI	RILLED: On: See TON: 50	Boring	-20-17 g Locatio	n Diagra	_m B	ORING NO. B-29 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7" HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	Й	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	USCS	SOIL DESCRIPTION	
8.8		S G G			SM	SILTY SAND; light brown, damp SANDSTONE; orange-brown, moderately hard, dry Boring terminated at 10 feet	
R- NR- G- B-	RING SAI NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPLE OSE PEN	NETROME	TER	LOGIES	NOTES: Groundwater not encountered during d Boring Location: +00 PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	rilling

LOCATI	TION: 50	Borin	I-20-17 g Locat	tion Diagr	am	BOI	RING NO. B-30 EQUIPMENT TYPE: CME-75 DRILLING TYPE: 7"HSA FIELD ENGINEER: C. Dumitru	
WATER CONTENT (%)	POCKET PENETROMETER (tsf)	SAMPLE TYPE	SAMPLE BLOWS/FT.	ОЕРТН (FEET)	nscs	GRAPHIC	SOIL DESCRIPTION	
		/ <mark>s</mark>		5	SM	SI	Boring terminated at 10 feet	
N- R- NR- G- B- BN-	STANDAF RING SAM NO SAMF GRAB SA BUCKET BLUNT N	MPLE PLE REC MPLE SAMPLI	COVERY			1 1	NOTES: Groundwater not encountered during dr Boring Location: Station No.	illing
					טו מי	GIES IN	PROJECT: PROPOSED DENNEHOTSO LOOP ROAD REF. NO.: 3127JS001	PLAT
	フ ¨			_3.1140		J. 10 114	BORING LOG	A-3

APPENDIX B

No	Depth	USCS	Water Content				Distribut by Weigl			Atte Lin	nits	Sulfates (ppm)	Remarks
No.	(ft)	Class.	(%)	3"	3/4"	#4	#10	#40	#200	ш	PI	Sunates (ppin)	Kemarks
B-2	0 - 2	SM	8.9				100	99	20.3				
B-2	2 - 3	SM	14.5				100	99	27.0				
B-2	2 - 5	SM										< 11.2	
B-5	0 - 2	SM										190	
B-6	2 - 3	SM	5.8				100	87	14.1				
B-8	2 - 5	SM	6.9				100	98	37.6		NP		
B-10	0 - 2	SM										493	
B-10	2 - 5	SM	3.5				100	99	26.1				
B-12	2 - 5	SM	5.2				100	97	34.9			3,200	
B-15	0 - 2	SM	7.8				100	96	22.4				
B-15	2 - 5	CL	18.4				100	80	78.1				
B-15	5 - 10	SM	9.2				100	99	25.2		NP		
B-17	0 - 2	SM	5.2				100	99	8.2			14.2	
B-18	0 - 2	SM										21.7	
B-19	2 - 5	SM	8.6				100	95	48.6				
B-20	0 - 2	SM										< 10.6	
B-22	2 - 5	SM	3.8				100	95	21.4		NP	1,910	
B-25	0 - 1	SM	8.8				100	91	17.8				
B-26	0 - 2	SM	6.3				100	94	18.3				
B-28	2 - 4	SM	2.6				100	98	8.7				
B-29	0 - 2	SM										< 10.7	
B-29	2 - 5	SM	8.8					100	33.7		NP		

NOTE: NP = Non-plastic



PROJECT: PROPOSED DENNEHOTSO LOOP ROAD

JOB NO.: 3127JS001

LABORATORY TEST RESULTS

PLATE

B-1

APPENDIX C

PEAK CT YEAR

33

 $\frac{\omega}{2}$

0

29

PEAK HR

13:00

16:00

#N/A

17:00

17:00

19:00

20:00

2033 23

18

2028 44

4997

Battery Volt:

400

362

2023

328

2018

FADT (02%)

297

Counter Type: Gamma

PEAK CT

22

16

0

33

24

22

20

PEAK HR

10:00

10:00

#N/A

11:00

12:00

11:00

2000年1000年100日

% TRKS

TOTALS

18:00 19:00 20:00 21:00 22:00

Daily Factors

0.9161 294

.1222

0.0000

0.8470

1.1917

1.0688 252

226

1,616

240

Seven-day Total

*

*

**

**

**

*

*



TRIBAL TRANSPORTATION PLANNING PROGRAM AVERAGE DAILY TRAFFIC (ADT) REPORT NAVAJO DIVISION OF TRANSPORTATION

Location: 0.	Data i lic.	Data File: 'n	occioii. 30	Continu		Rollto:		Reservation.	nyelicy.	- 1	
.7 mile northwes	10-0-011100+0-1.101	Data File: 'n6460mn 0 7 rdf	30	30	0700	6460	100	780	CONI	NISS	
Location: 0.7 mile northwest of Jct. US160 & N6460. (north of Laguna Wash)			Community: Dennehotso		State:	2011	county:	00000	Class:	2	
6460. (north of La			Dennehotso		04	٠.	100	004	4		
duna (Wash)					Roadway Width (ft.):		Surface Type:		Mile Post:		
					23.0	a	_		22.20		
COMMENTS:	End Time:	בווט במנס.	End Date:	Court Illie.	Start Time:	Owit Date.	Start Date:		ADT Mon/Yr		The state of the s
COMMENTS:	24:00	4100/100/17	A/30/2013	00.00	00.00	TICUICUIU	4/25/2013	101-10	Anr-13		
		0.04-10-801	100 E1 40 C	30-30-21.1	36 EO 34 4	(Sprion-minute-seconds)	(Downson Minute Constant)	AIRCOORDINAIES	ATE COORDINAT		
		×	101	Z		(N OF W)	141	EO	7		NAVAJO D.O.T.

- 1.) Daily Factor = 6 day avg. / daily total.

END TIME LOAY

30 Tue

Wed 1

Sun

Daily

0 0

01:00 02:00 03:00 04:00 05:00 05:00 06:00 07:00 07:00 09:00 11:00 12:00 14:00 14:00 14:00

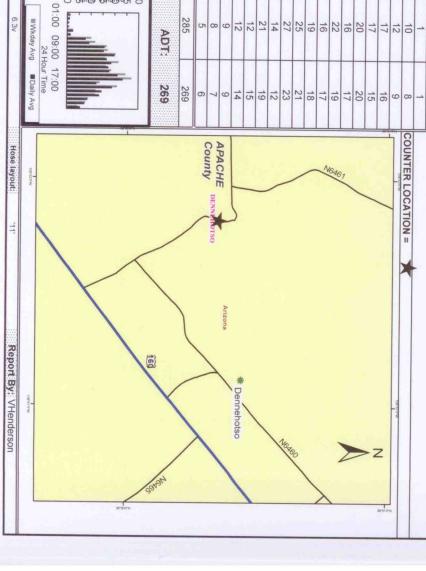
14 23 23 23 23 23 23 23 23

14 15

4 8

17 13 16 22 19 0 2 2 0 0 2

- % TRKS = Percent Trucks (** No Truck Study Performed).
 Counter location is drawn utilizing the Map from either TOPO or ArcView program.
- 4.) The daily totals per lane includes the trucks.
 5.) Set near proposed bridge site



PEAK CT

22

16

17

PEAK HR

17:00

14:00

20:00

19:00

17:00

21:00

20:00

2033 17

2028

3667

Battery Volt:

278

251

27

21

2023

228

YEAR

2018

FADT (02%)

206

Counter Type: Gamma

AM PEAK CT

PEAK HR

11:00

11:00

08:00

12:00

09:00

12:00

Traffic Valuma

14

14

13

15

15

18

16

% TRKS

TOTALS

09:00 10:00 11:00 11:00 12:00 13:00 13:00 14:00 16:00 16:00 17:00 18:00 19:00 19:00 20:00 22:00

14 14 9 22 21 8

7 10 12 13 14

12

10 15

Daily Factors

0.9485

1.0100

0.8773 213

1.0678

0.9115

1.0739 174

1.1752 **

ADT:

159

195

205

197

185

Seven-day Total

*

** 1,308

*

*

**

*

20



TRIBAL TRANSPORTATION PLANNING PROGRAM AVERAGE DAILY TRAFFIC (ADT) REPORT NAVAJO DIVISION OF TRANSPORTATION

		COMMENTS:					
	24:00	End Time:				Julipo-Z.IOI	water inc. HOTE
109-50-30.3	4/30/2013	Elid Date:					Data File: 'n6461mn0 2 rdf
	4/20/2012	End Date:			Community: Dennehotso		Section: 10
36-50-51.8	00:00	Start Time:	20.0	Roadway Width (ft.):	State: U4		
(N or W)	4/24/2013	Orait Date.			- 1		Route: 6461
	4/2//2013	Start Date:	_	Surface Type:	County: 001	/8U Cc	reservation:
ATR COORDINATES	Apr-13	ADT Mon/Yr:	0.20	Mile Post:	Class.		
					Noon:	N33	Agency.

Daily Avg

1.) Daily Factor = 7 day avg. / daily total.

DATE | DAY

Mon.

Tue

Wed Wed

Thu

27 Sat

Sun 2

9 3 0 0 0 2 0 7 26

06:00 07:00 05:00

9

103

6 15

0 4 0

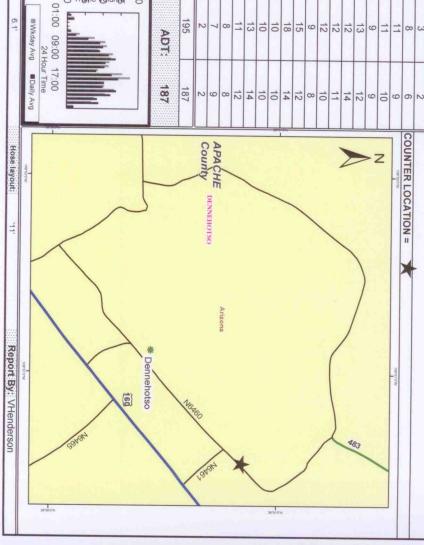
8 3 6 3 3 5

10 15

8 3 5 2 1

12 12 13

- 2.) % TRKS = Percent Trucks (** No Truck Study Performed).
 3.) Counter location is drawn utilizing the Map from either TOPO or ArcView program.
 4.) The daily totals per lane includes the trucks.



ADT INTIAL 269
GROWTH RATE (%) 2.00
COORESPONDING GROWTH FACTOR 10.95

VEHICLE TYPE	TOTAL VEHICLES	PERCENT	ESAL FACTOR	DESIGN ESAL	
Automobiles Trucks	269 269	98 2	0.0008 1.2	0.21 6.46	
		_	TOTAL ADT	6.67	ESAL/DAY

FIRST YEAR TRAFFIC VOLUME: 365 days x 6.67 ESAL/Day = 2,435 ESALs

TEN YEAR DESIGN: 10.95 x 2,435 ESALs = 26,663 ESALs

AASHTO Traffic Growth Factors

Analysis	Annual Growth Rate, Percent (g)												
Period Years (n)	No Growth	2	4	5	6	7	8	10					
1	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0					
2	2.0	2.02	2.04	2.05	2.06	2.07	2.08	2.10					
2 3 4 5 6 7	3.0	3.06	3.12	3.15	3.18	3.21	3.25	3.31					
4	4.0	4.12	4.25	4.31	4.37	4.44	4.51	4.64					
5	5.0	5.20	5.42	5.53	5.64	5.75	5.87	6.11					
6	6.0	6.31	6.63	6.80	6.98	7.15	7.34	7.72					
7	7.0	7.43	7.90	8.14	8.39	8.65	8.92	9.49					
8	8.0	8.58	9.21	9.55	9.90	10.26	10.64	11.44					
8	9.0	9.75	10.58	11.03	11.49	11.98	12.49	13.58					
10	10.0 -	→ 10.95	12.01	12.58	13.18	13.82	14.49	15.94					
11	11.0	12.17	13.49	14.21	14.97	15.78	16.65	18.53					
12	12.0	13.41	15.03	15.92	16.87	17.89	18.98	21.38					
13	13.0	14.68	16.63	17.71	18.88	20.14	21.50	24.52					
14	14.0	15.97	18.29	19.16	21.01	22.55	24.21	27.97					
15	15.0	17.29	20.02	21.58	23.28	25.13	27.15	31.77					
16	16.0	18.64	21.82	23.66	25.67	27.89	30.32	35.95					
17	17.0	20.01	23.70	25.84	28.21	30.84	33.75	40.55					
18	18.0	21.41	25.65	28.13	30.91	34.00	37.45	45.60					
19	19.0	22.84	27.67	30.54	33.76	37.38	41.45	51.16					
20	20.0	24.30	29.78	33.06	36.79	41.00	45.76	57.28					
25	25.0	32.03	41.65	47.73	54.86	63.25	73.11	98.35					
30	30.0	40.57	56.08	66.44	79.06	94.46	113.28	164.49					
35	35.0	49.99	73.65	90.32	111.43	138.24	172.32	271.02					

^{*}Factor = $\frac{(1+g)^n - 1}{g}$, where $g = \frac{rate}{100}$ and is not zero. If annual growth rate is zero, the growth factor is equal to the analysis period.

Note: The above growth factors multiplied by the first year traffic estimate will give the total volume of traffic expected during the analysis period.

APPENDIX D

PaveXpress

Project Information

Project Name Dennehotso Loop Road

Project Description

Estimated Completion 2018

Year

State Arizona

Roadway Classification Residential/Collector

Design Parameters

Design Period (Years) 10 years

Reliability Level (R) 70 Z_R=-0.524

Combined Standard 0.5 Error (S0)

Initial Serviceability 4.5

Index (pi)

Terminal Serviceability 2.5

Index (pt)
Change in 2.00

Serviceability (APSI)

Traffic Data

Completion Year Traffic N/A

Load Equivalency N/A Factor

Completion Year N/A

ESALs
Design Period N/A

Future Traffic Growth N/A Rate (%)

ESAL Growth Rate (%) N/A

Total Design ESALs 27,000 (W18)

Pavement Structure

Surface Lifts

None

Base Layers

Type Layer Coef Drainage Thickness

Resilient Modulus (MR)

20000 psi

Design Guidance

Required minimum design SN: 1.10

Layer Thicknesses (in)

Surface: 9.50

Surface

Total SN: 1.14

Subgrade

Design Notes

PAVEMENT DESIGN CALCULATIONS

Alternative No. 1 - Aggrergate Surface

Material

Thickness Coefficient Drainage Value

Base Course 9 0.12 1 1.08

Total SN 1.08 Required SN 1.10

Alternative No. 2 - Asphalt Pavememt

Material

Thickness Coefficient Drainage Value

Asphalt 2 0.44 0.88 Base Course 4 0.12 1 0.48

Total SN 1.36 Required SN 1.10

Alternative No. 3 - Chip Seal

Material

Thickness Coefficient Drainage Value

 Chip Seal
 2
 0.12
 0.24

 Base Course
 7
 0.12
 1
 0.84

Total SN 1.08 Required SN 1.10